





# Midwest Pooled Fund Program

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# EVALUATION OF FLARED APPROACH GUARDRAIL TRANSITIONS: TEST NO. FLAGT-2



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#### 16. Abstract

Approach guardrail transitions (AGTs) are typically installed tangent to the roadway. However, there was a desire to install AGTs flared away from the roadway to satisfy geometry constraints or to shorten system lengths. Previously, LS-DYNA simulations were used to explore the performance of AGTs flared away from the roadway, and a 15:1 flare rate was identified as the critical maximum flare rate. A crash test on a flared AGT was conducted in accordance with the Test Level 3 (TL-3) criteria of the American Association of State and Highway and Transportation Officials' (AASHTO) *Manual for Assessing Safety Hardware* (MASH). Test no. FLAGT-1, a MASH test designation no. 3-21 test, was conducted with the 2270P pickup impacting an AGT flared away from the roadway at a 15:1 rate. The selected AGT configuration had previously passed MASH TL-3 in a tangent layout. However, the 15:1 flared AGT test resulted in vehicle snag at the upstream end of the concrete buttress, which caused excessive occupant compartment deformations and excessive longitudinal ORAs. Thus, the test failed to meet MASH standards.

As documented herein, the research project continued by redesigning the flared AGT to strengthen the barrier, reduce system deflections, and mitigate vehicle snag at the end of the buttress. The redesigned system continued the use of a 15:1 flare, but the 6.5-ft long W6x9 posts on the downstream end of the AGT were replaced with larger 7.5-ft long W6x15 posts. MASH test designation no. 3-21 was then re-run on the downstream end of the redesigned system.

In test no. FLAGT-2, the 5,000-lb pickup truck impacted the system at 62.6 mph and angle of 25.4 degrees relative to the roadway (effectively 29.2 degrees relative to the flared system). The vehicle was successfully contained and redirected. Vehicle decelerations, ORAs, and OVIs all fell within the MASH limits. However, deformations to the toe pan area were 9.9 in., which exceeded MASH limits. Therefore, test no. FLAGT-2 failed to satisfy the safety criteria for MASH test designation no. 3-21.

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#### DISCLAIMER STATEMENT

This material is based upon work supported by the Federal Highway Administration, U.S. Department of Transportation and the Midwest Pooled Fund Program under TPF-5(430) Supplement #3. The contents of this report reflect the views and opinions of the authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the University of Nebraska-Lincoln, state highway departments participating in the Midwest Pooled Fund Program nor the Federal Highway Administration, U.S. Department of Transportation. This report does not constitute a standard, specification, or regulation. Trade or manufacturers' names, which may appear in this report, are cited only because they are considered essential to the objectives of the report. The United States (U.S.) government and the State of Nebraska do not endorse products or manufacturers.

#### UNCERTAINTY OF MEASUREMENT STATEMENT

The Midwest Roadside Safety Facility (MwRSF) has determined the uncertainty of measurements for several parameters involved in standard full-scale crash testing and non-standard testing of roadside safety features. Information regarding the uncertainty of measurements for critical parameters is available upon request by the sponsor and the Federal Highway Administration.

#### INDEPENDENT APPROVING AUTHORITY

The Independent Approving Authority for the data contained herein was Dr. Andrew Loken, Research Assistant Professor.

## **A2LA ACCREDITATION**

The test reported herein is within the scope of MwRSF's A2LA Accreditation. MwRSF's accreditation documentation can be found in Appendix A.

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		IMATE CONVERSION	S TO SI UNITS	
Symbol	When You Know	Multiply By	To Find	Symbol
		LENGTH		
1.	inches	25.4	millimeters	mm
t	feet	0.305	meters	m
d	yards	0.914	meters	m
ni	miles	1.61	kilometers	km
		AREA		
$n^2$	square inches	645.2	square millimeters	$mm^2$
$\mathfrak{f}\mathfrak{t}^2$	square feet	0.093	square meters	$m^2$
$d^2$	square yard	0.836	square meters	$m^2$
ıc	acres	0.405	hectares	ha
ni <sup>2</sup>	square miles	2.59	square kilometers	km <sup>2</sup>
		VOLUME		
fl oz	fluid ounces	29.57	milliliters	mL
gal	gallons	3.785	liters	L
$t^3$	cubic feet	0.028	cubic meters	$m^3$
$vd^3$	cubic yards	0.765	cubic meters	$m^3$
		E: volumes greater than 1,000 L shall		
		MASS		
oz	ounces	28.35	grams	g
b	pounds	0.454	kilograms	kg
Γ	short ton (2,000 lb)	0.907	megagrams (or "metric ton")	Mg (or "t")
	(2,000 20)	TEMPERATURE (exact d		-11g (or t )
		5(F-32)/9		
°F	Fahrenheit	or (F-32)/9	Celsius	°C
		` /		
P-	£4 11	ILLUMINATION	1	1
ĈC	foot-candles	10.76	lux	lx
1	foot-Lamberts	3.426	candela per square meter	cd/m <sup>2</sup>
		FORCE & PRESSURE or S		
lbf	poundforce	4.45	newtons	N
lbf/in <sup>2</sup>	poundforce per square inch	6.89	kilopascals	kPa
	APPROXI	MATE CONVERSIONS	FROM SI UNITS	
Symbol	When You Know	Multiply By	To Find	Symbol
		LENGTH		
nm	millimeters	0.039	inches	in.
n	meters	3.28	feet	ft
n	meters	1.09	yards	yd
cm	kilometers	0.621	miles	mi
		AREA		
$nm^2$	square millimeters	0.0016	square inches	$in^2$
n <sup>2</sup>	square meters	10.764	square feet	ft <sup>2</sup>
$n^2$	square meters	1.195	square yard	yd <sup>2</sup>
1a	hectares	2.47	acres	ac
cm <sup>2</sup>	square kilometers	0.386	square miles	mi <sup>2</sup>
	•	VOLUME	•	
nL	milliliter	0.034	fluid ounces	fl oz
IIL	liters	0.264	gallons	gal
$n^3$	cubic meters	35.314	cubic feet	ft <sup>3</sup>
n <sup>3</sup>	cubic meters	1.307	cubic yards	yd <sup>3</sup>
	Cuoto motors	MASS	Cuoto Jurus	Ju
5	grams	0.035	ounces	OZ
g 4 ( "")	kilograms	2.202	pounds	lb T
Ag (or "t")	megagrams (or "metric ton")	1.103	short ton (2,000 lb)	T
~		TEMPERATURE (exact d		277
C	Celsius	1.8C+32	Fahrenheit	°F
		ILLUMINATION		
X	lux	0.0929	foot-candles	fc
d/m <sup>2</sup>	candela per square meter	0.2919	foot-Lamberts	fl
		FORCE & PRESSURE or S	STRESS	
		ONCE & I NESSUNE OF A	TILLOS	
1	newtons	0.225	poundforce	lbf

<sup>\*</sup>SI is the symbol for the International System of Units. Appropriate rounding should be made to comply with Section 4 of ASTM E380.

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#### 1 INTRODUCTION

# 1.1 Background

Approach guardrail transitions (AGTs) are commonly used to safeguard rigid hazards, including bridge railings and concrete parapets. A typical AGT is shown in Figure 1 [1]. AGT installations provide a safe transition in lateral stiffness between semi-rigid guardrail and rigid bridge rail installations. Often, intersecting roadways or other roadside obstacles create space constraints that limit the ability to install an AGT within the desired area. Thus, a need exists to minimize the length of AGTs tangent to the roadway.



Figure 1. AGT Installation [1]

Installing an AGT with a flare away from the roadway would reduce the system length along the primary roadway, as the guardrail would intercept the vehicle runout path closer to the hazard, when compared to a tangent installation. Additionally, the flared AGT configuration would result in a greater lateral offset between the guardrail and the traveled roadway. Thus, the flared AGT configuration would move the hazard posed by impacts with the guardrail farther away from the traveled road, and increase the area for the driver to regain control of the vehicle. As a result, flared AGT installations would reduce both accident frequency and the overall installation maintenance and material costs.

Previously, guidance for flaring the Midwest Guardrail System (MGS) away from the roadway [2] had been established in accordance with NCHRP Report 350 Test Level 3 (TL-3) criteria [3]. Due to the need to reduce guardrail length adjacent to the rigid parapet, initiating the flare in the transition region rather than the upstream MGS is more desirable as it would provide a greater reduction in barrier length along the primary road than flaring the W-beam section of guardrail at the upstream end of the transition. Unfortunately, minimal research and full-scale crash testing has been conducted on flared AGTs.

Several concerns for flaring AGTs have arisen due to previous flare rate studies. Flaring a guardrail system away from the roadway increases the vehicle impact angle with the barrier installation, which increases the chance for pocketing and wheel snag. The increased impact angle also results in larger loads imparted to the barrier system, which could lead to component failure or rail rupture. Thus, a need exists to evaluate and establish guidance for flaring AGT installations under the American Association of State and Highway and Transportation Officials' (AASHTO) *Manual for Assessing Safety Hardware* (MASH) safety performance criteria [4].

The Midwest Pooled Fund Program sponsored the Phase I research effort to identify the critical flare rate for a thrie-beam AGT, which would provide the greatest reduction in length of

need (LON) while maintaining acceptable barrier safety performance [5]. The computer simulation study identified a flare rate of 15:1 (3.81 degrees from roadway) as the maximum critical flare rate to pass MASH TL-3 safety performance criteria. LS-DYNA simulations were also conducted to identify downstream critical impact points (CIPs) for both the 2270P and 1100C test vehicles with the 15:1 flared AGT.

The subsequent phases of the research effort included crash testing and evaluation of flared AGT installations to MASH TL-3 conditions. Test no. FLAGT-1 was conducted in accordance with MASH test designation no. 3-21 on an AGT with a 15:1 flare rate [6]. The AGT consisted of nested, 12-gauge thrie beam supported by W6x9 posts spaced at 18¾-in. intervals. The downstream end of the AGT was connected to a concrete parapet with the geometry of the Standardized Transition Buttress [1] oriented tangent to the roadway. In test no. FLAGT-1, the pickup truck impacted the AGT 90 in. upstream of the buttress at speed of 63.2 mph and at an angle of 25.4 degrees (29.2 degrees relative to the guardrail).

The vehicle was contained and redirected by the flared AGT. However, increased impact severity resulting from the flare caused excessive guardrail deflections and led to a large kink in the nested thrie beam at the upstream end of the buttress. The 2270P vehicle snagged on this kink, which resulted in excessive occupant compartment deformations to the toe pan and wheel well, as well as an excessive longitudinal acceleration of -24.2 g's. Thus, test no. FLAGT-1 failed to satisfy the safety performance criteria of MASH test designation no. 3-11 [6]. Subsequently, the flared AGT needed to be modified to limit system deflections and create a crashworthy barrier.

# 1.2 Objective

The objective of the research study was to identify the critical flare rate for flaring AGTs away from the primary roadway. Research focused on determining the maximum allowable flare rate that could safely be applied to 31-in. tall thrie-beam AGTs without curbs below the guardrail. Additionally, the standardized buttress was targeted for use at the downstream end of the AGT because it included chamfers intended to mitigate tire snag.

The objective of Phases II through IV was to evaluate the safety performance of flared AGTs in accordance with MASH TL-3 criteria. Test no. FLAGT-2, documented herein, was conducted with the 2270P pickup truck impacting the downstream end of the modified, flared AGT to evaluate the potential for vehicle snag on the rigid buttress.

## 1.3 Scope

After the failure observed in test no. FLAGT-1 on an AGT with a 15:1 flare rate, the project was rescoped to include identifying and implementing system modifications that would increase the safety performance of the flared AGT. The redesigned AGT was then retested to the same impact conditions as the failed test (FLAGT-1) to evaluate the barrier system modifications. Note, the complete evaluation of the flared AGT required MASH test designation nos. 3-20 and 3-21 at both the downstream end of the AGT to evaluate the snag on the buttress and the upstream end of the AGT to evaluate pocketing, snag, and capture at the W-to-thrie transition section.

#### 2 FLARED AGT DESIGN MODIFICATIONS

The previous crash test on a flared AGT, test no. FLAGT-1, failed to satisfy MASH safety performance criteria for two reasons: (1) the longitudinal ORA of –24.23 g's exceeded the MASH limit of ±20.49 g's, and (2) the 12.0-in. intrusion into the toe pan area of the occupant compartment exceeded the MASH limit of 9 in. After examining all available data for this test, it became evident that both the excessive decelerations and excessive occupant compartment crush were the result of the vehicle snagging on the upstream end of the buttress and the large kink in the guardrail that formed adjacent to the buttress. The large kink in the nested thrie beam was a damage characteristic not observed in previous MASH testing of AGTs. Additionally, a large soil crack formed along the front flange of the AGT posts, as shown in Figure 2, which was not typical of previous AGT crash testing.



Figure 2. System Damage, Test No. FLAGT-1

These unusual damage characteristics were the result of excessive guardrail deflections within the downstream region of the AGT. System deflections were expected to be higher than previous MASH AGTs due to the 30 percent increase in impact severity associated with the 15:1 flare. However, the lateral deflections of the flared AGT in test no. FLAGT-1 were significantly higher than predicted by the Phase I numerical simulations. To prevent vehicle snag, the system deflections needed to be reduced.

It was noted that the AGT posts did not plastically bend during the impact. Instead, the soil behind the posts shifted, opening the large soil crack along the front of the posts and allowing the

posts to rotate back. Installing larger and deeper embedded posts would result in more soil resistance, increased lateral stiffness to the AGT, and reduced deflections during impact. Therefore, the 6.5-ft long W6x9 posts in the nested thrie beam region of the AGT were replaced with 7.5-ft long W6x15 posts, as shown in Figure 3. Note, all other barrier components were identical to the system configuration evaluated in test no. FLAGT-1, including the 15:1 flare rate.

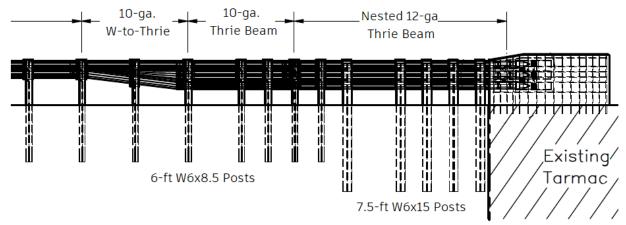


Figure 3. Modified AGT Posts, Test No. FLAGT-2

#### **3 DESIGN DETAILS**

The flared AGT test installation was approximately 81 ft – 6 in. long and consisted of five main components: (1) a concrete buttress, (2) a thrie-beam AGT, (3) standard MGS, (4) a guardrail anchorage system, and (5) a flared AGT connector plate. Design details for test no. FLAGT-2 are shown in Figures 4 through 31. Photographs of the test installation are shown in Figures 32 through 34. Material specifications, mill certifications, and certificates of conformity for the system materials are shown in Appendix A. Note, the test installation was identical to the one evaluated previously in test no. FLAGT-1 except the posts supporting the nested thrie beam (post nos. 16 through 20).

The downstream end of the installation consisted of a concrete parapet with dimensions matching those of the standardized concrete buttress [1]. The buttress was 84 in. long and 36 in. tall. To prevent vehicle snag on the buttress above the thrie-beam rail, the upstream end of the buttress was 32 in. tall and incorporated a 24-in. long slope to bring the barrier height up to 36 in. The upstream end of the standardized buttress utilized a dual tapered design, or dual chamfer, as shown in Figure 13. The lower tire chamfer was 18 in. long by 4½ in. wide by 14 in. tall and was designed to reduce the propensity for wheel snag on the buttress. The upper chamfer measured 4 in. long by 3 in. wide and extended vertically 18 in. along the remaining height of the buttress. The upper chamfer was designed to limit vehicle snag on the buttress, prevent the guardrail from bending around a rigid corner, and to limit the unsupported span length of the rail upstream from the buttress. The buttress was reinforced with Grade 60 rebar, as detailed in Figures 14 through 17, and the vertical steel in the buttress was anchored to the tarmac using an epoxy adhesive, as detailed in Figure 14.

The AGT consisted of a 12½-ft section of nested 12-gauge thrie-beam, a 6¼-ft section of 12-gauge thrie-beam, a 6¼-ft long 10-gauge W-to-thrie transition section, and a 12½-ft section of nested 12-gauge W-beam guardrail. Upstream from the AGT was 37½ ft of 12-gauge W-beam guardrail, which included the MGS and a guardrail anchor. All guardrail sections were mounted with a height of 31 in. to the top of the guardrail. Post nos. 3 through 15 and were 6-ft long W6x8.5 steel posts embedded 40 in. into the soil, while post nos. 16 through 20 were 7.5-ft long W6x15 steel posts embedded 61 in. in the soil. Spacing between posts varied, as show in Figure 5.

The upstream stiffness transition, or the W-beam to nested thrie beam transition, was previously full-scale crash tested to MASH TL-3 [7]. Nested W-beam was placed adjacent to the W-to-thrie transition segment to strengthen the AGT and prevent rupture of the W-beam, as was previously done to strength the upstream stiffness transition installed behind a curb [8].

The upstream end of the guardrail installation, post nos. 1 and 2, were configured with a trailing-end anchorage system. The guardrail anchorage system was utilized to simulate the strength of other crashworthy end terminals. The anchorage system consisted of timber posts, foundation tubes, anchor cables, bearing plates, rail brackets, and channel struts, which closely resembled the hardware used in the Modified Breakaway Cable Terminal (BCT) system and is now part of a crashworthy, downstream trailing end terminal [9-12].

Finally, guardrail segments were installed with a 15:1 flare rate relative to the face of the concrete buttress. An angled connector plate assembly was placed between the thrie-beam terminal connector and the buttress to connect the guardrail at a 15:1 flare rate, as shown in Figure 7 and

detailed in Figures 24 through 26. The angled connector plate assembly was constructed with a  $^{3}/_{16}$ -in. thick steel face plate and  $^{1}/_{4}$ -in. thick steel gussets. The plate extended 2 in. laterally from the traffic-side face of the buttress and the downstream end of the plate tapered flush with the face of the buttress via a 5:1 slope to mitigate snag in the reverse direction. As shown in Figure 28, the 10-gauge thrie-beam terminal connector had  $1^{1}/_{4}$ -in. tall by 1-in. wide splice slots oriented in the vertical direction to improve constructability of the nested thrie-beam splice connection. Five  $7^{1}/_{4}$ -in. diameter ASTM F3125, Grade 120 heavy hex head bolts were used to connect the guardrail and connector assembly to the buttress.

7

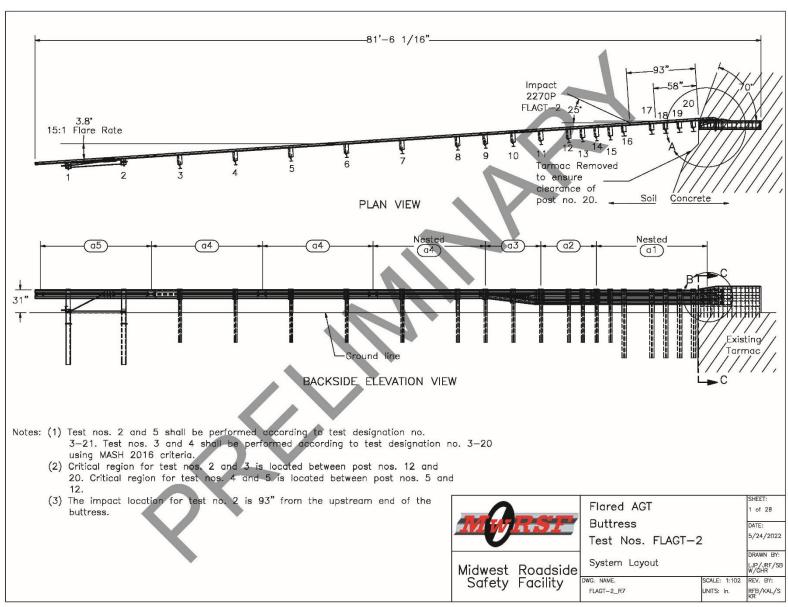


Figure 4. Test Installation Layout, Test No. FLAGT-2

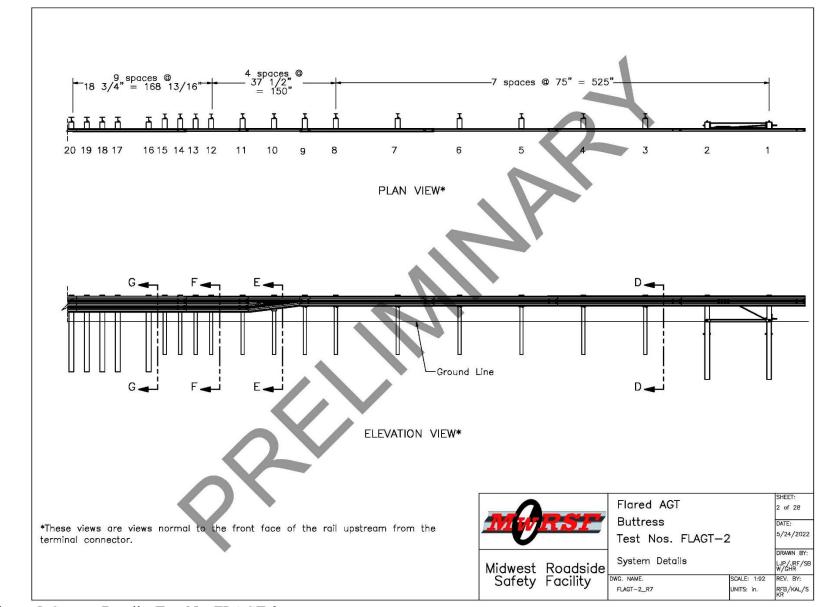


Figure 5. System Details, Test No. FLAGT-2

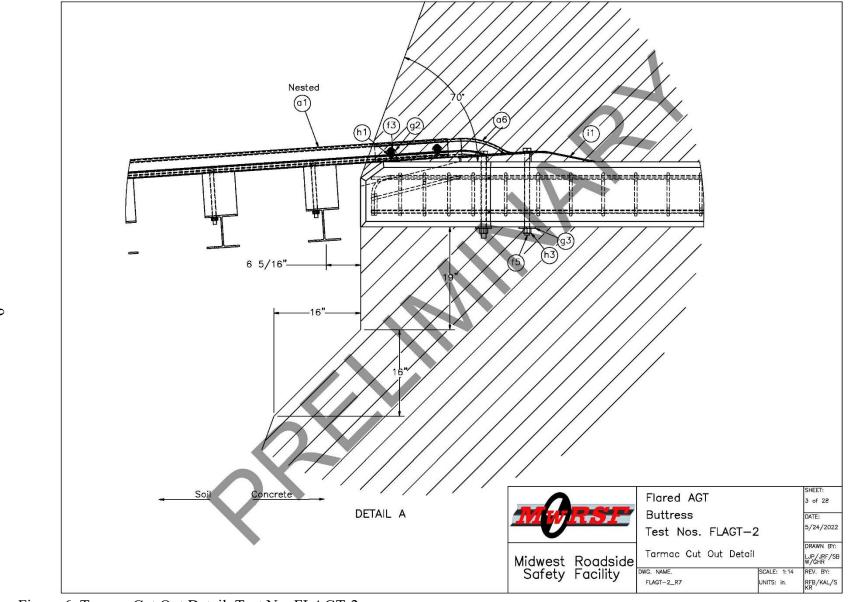


Figure 6. Tarmac Cut Out Detail, Test No. FLAGT-2

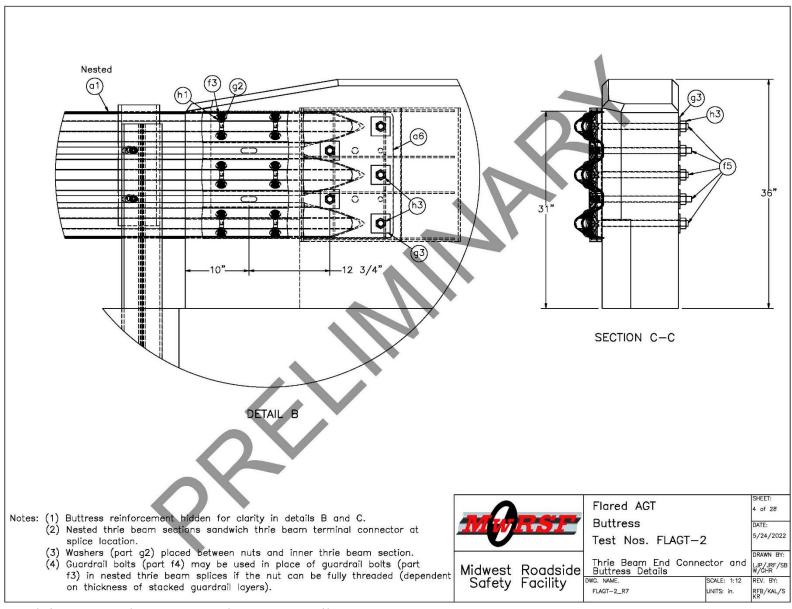


Figure 7. Thrie-Beam End Connector and Buttress Details, Test No. FLAGT-2

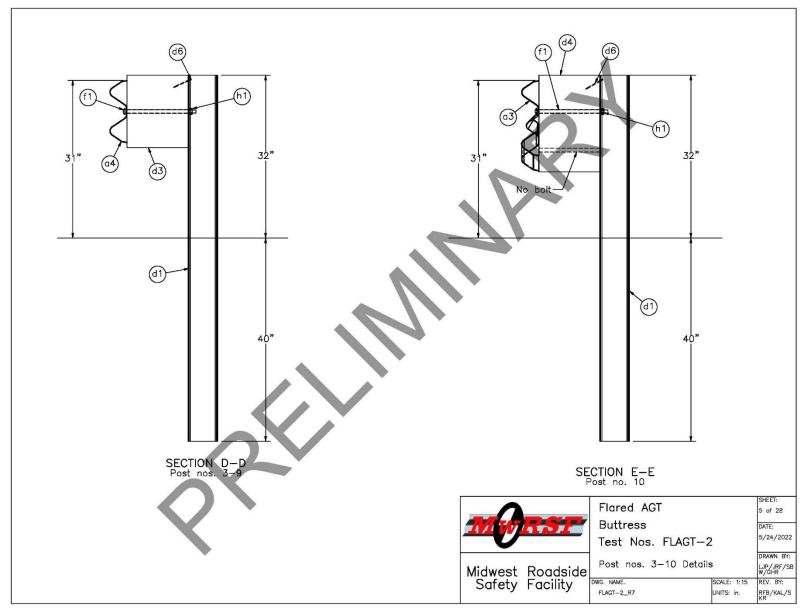


Figure 8. Post Nos. 3 through 10 Details, Test No. FLAGT-2

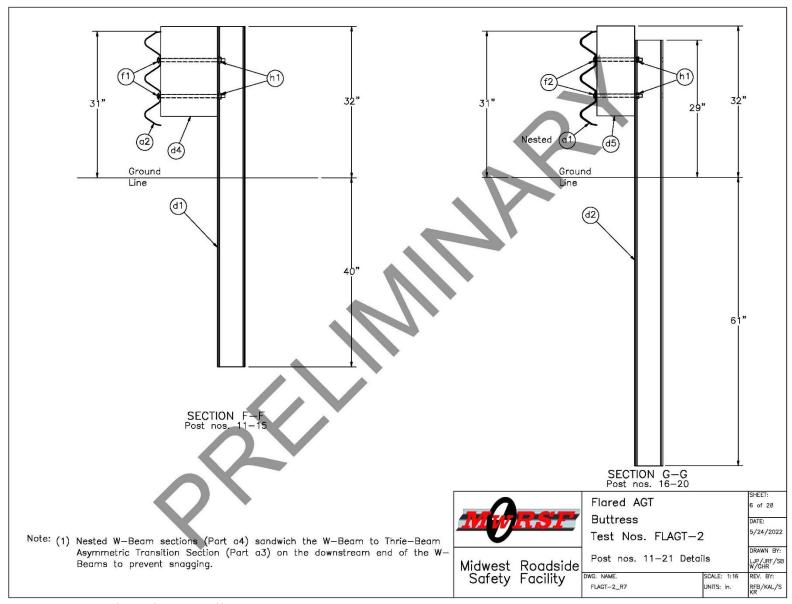


Figure 9. Post Nos. 11 through 20 Details, Test No. FLAGT-2

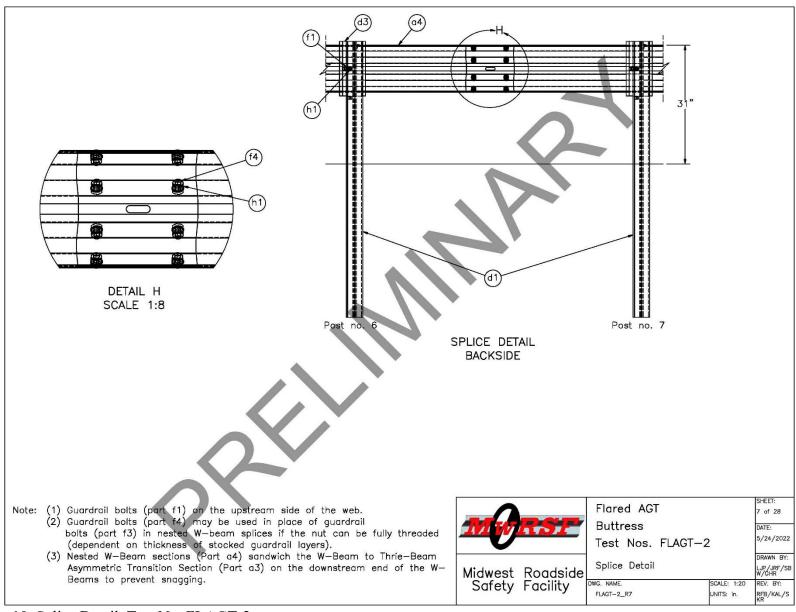


Figure 10. Splice Detail, Test No. FLAGT-2

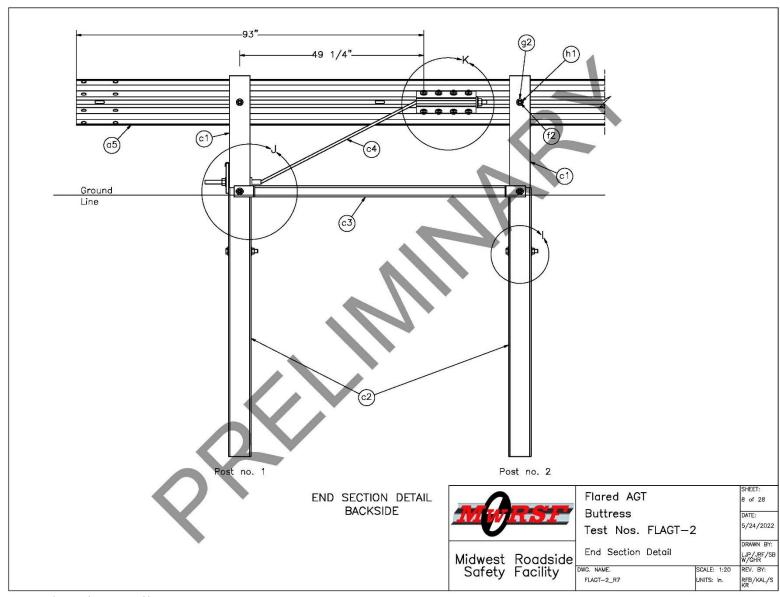


Figure 11. End Section Detail, Test No. FLAGT-2

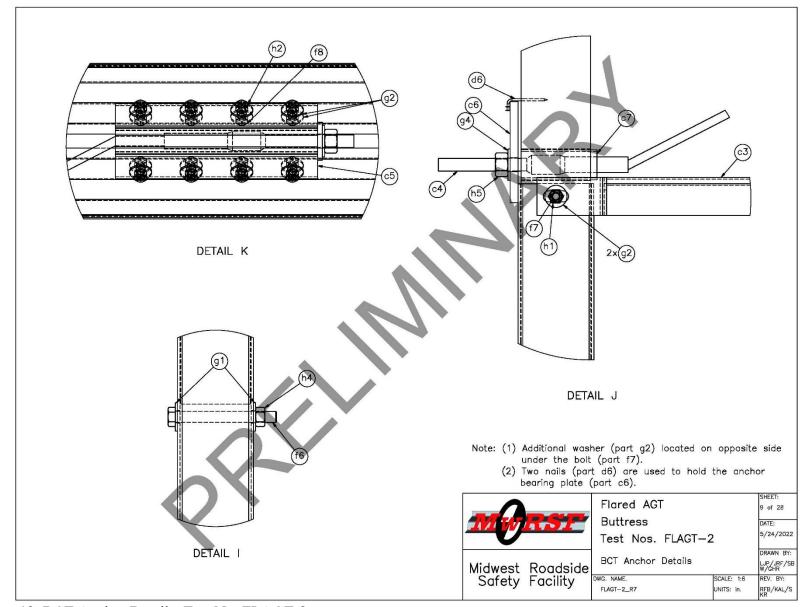


Figure 12. BCT Anchor Details, Test No. FLAGT-2

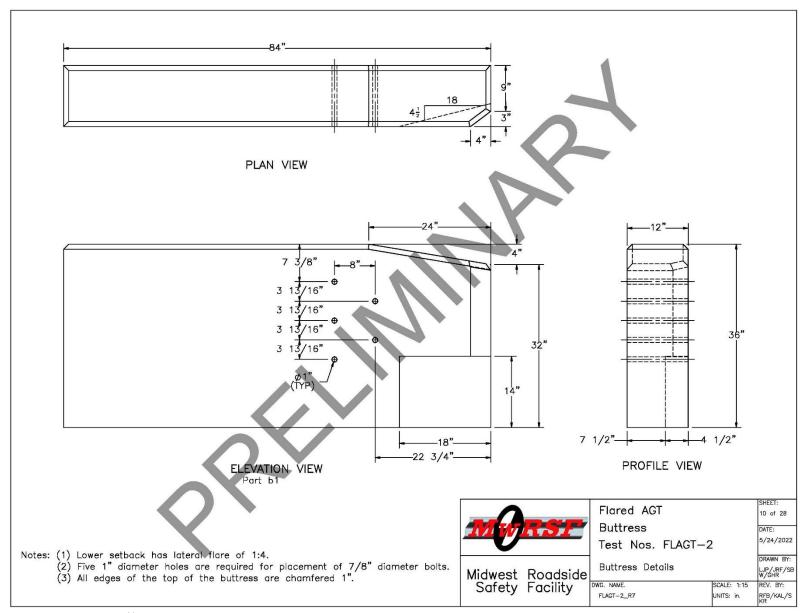


Figure 13. Buttress Details, Test No. FLAGT-2

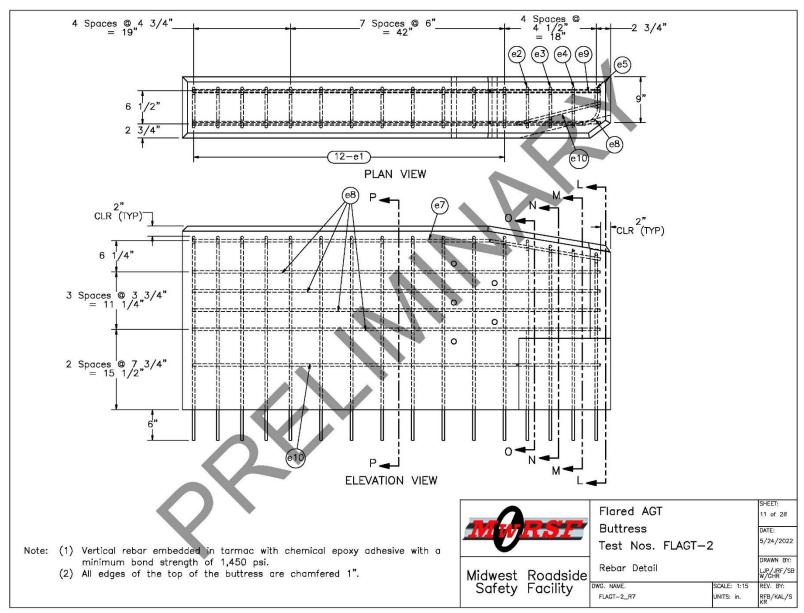


Figure 14. Buttress Rebar Detail, Test No. FLAGT-2

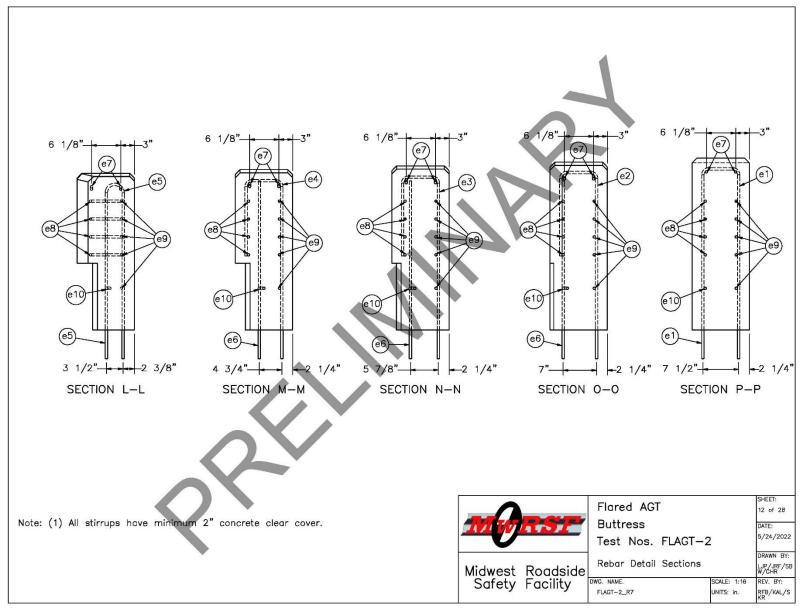


Figure 15. Buttress Rebar Detail Sections, Test No. FLAGT-2

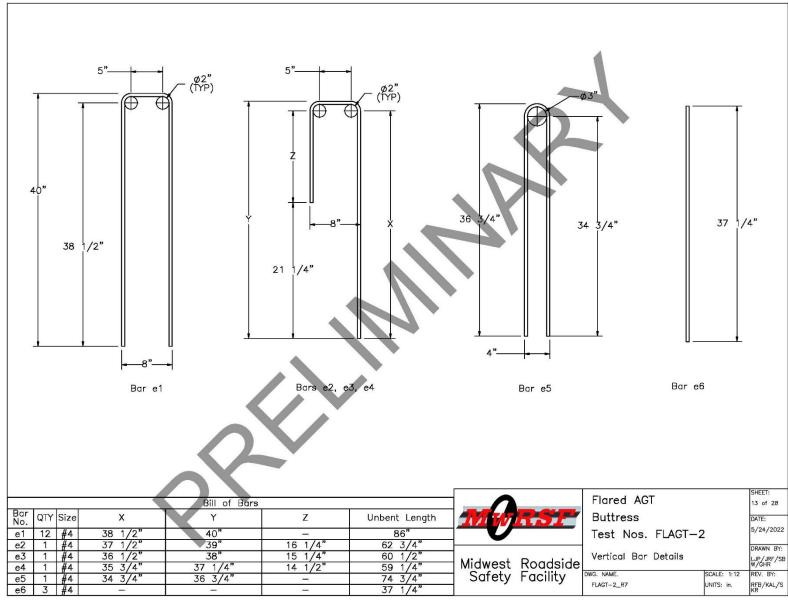


Figure 16. Vertical Rebar Details, Test No. FLAGT-2

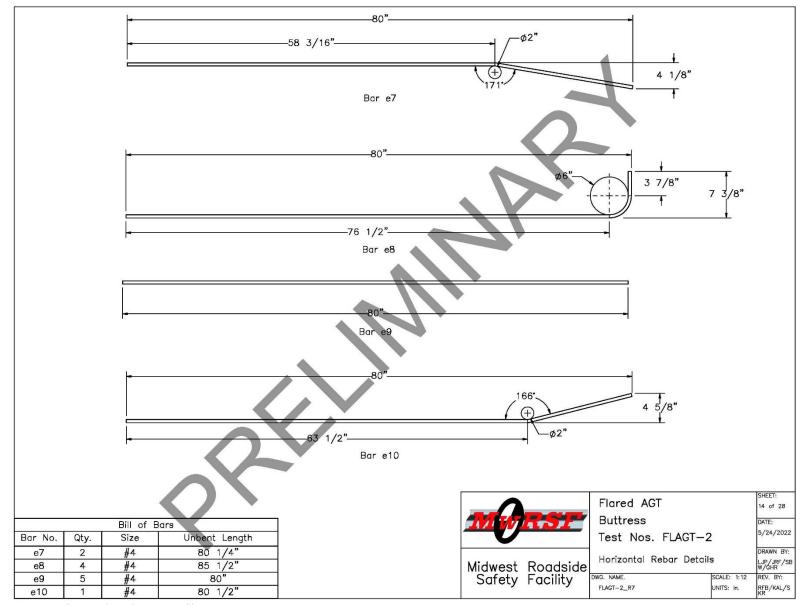


Figure 17. Horizontal Rebar Details, Test No. FLAGT-2

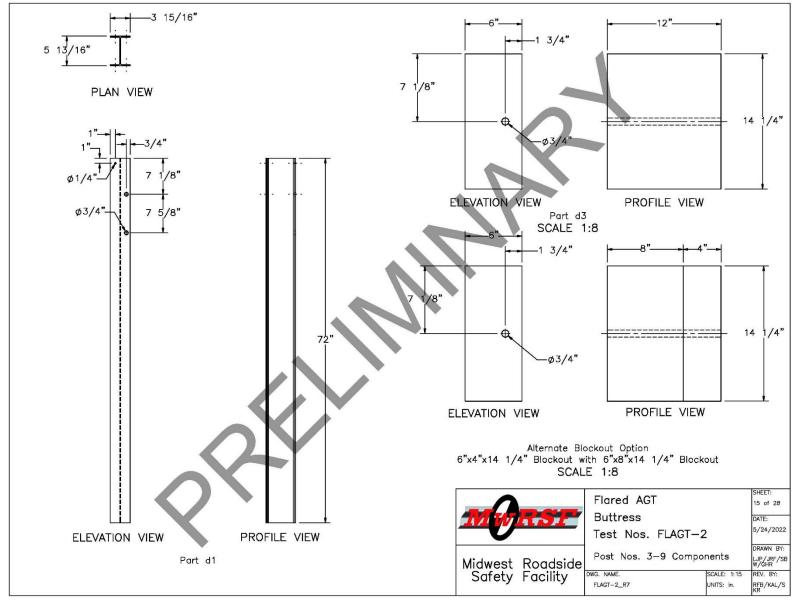


Figure 18. Post Nos. 3 through 9 Components, Test No. FLAGT-2

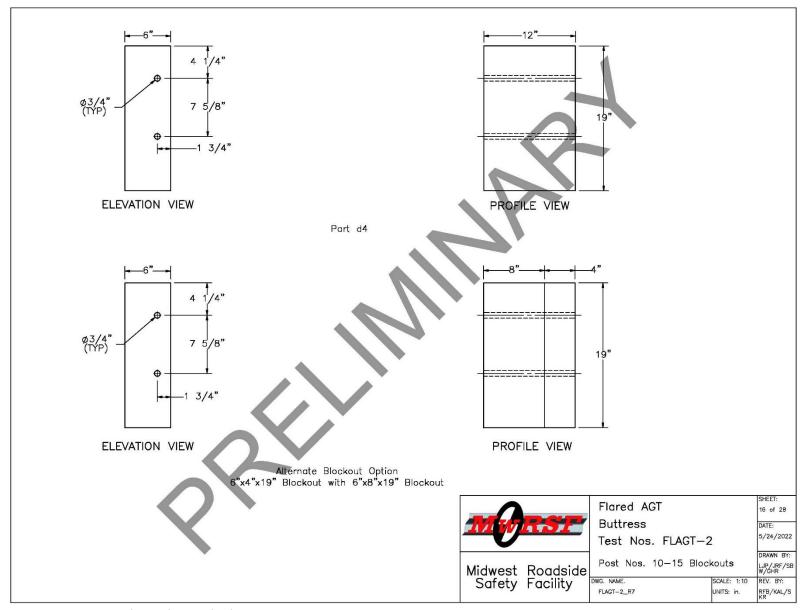


Figure 19. Post Nos. 10 through 15 Blockouts, Test No. FLAGT-2

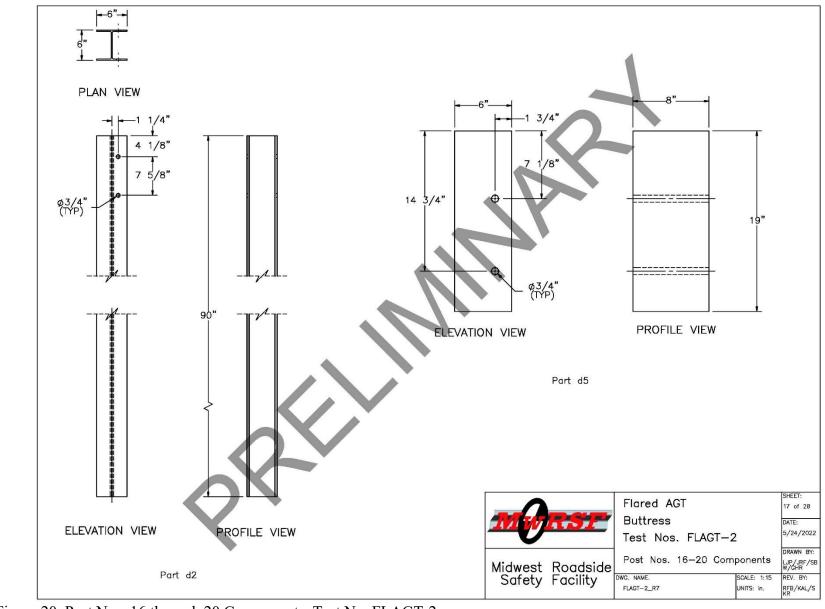


Figure 20. Post Nos. 16 through 20 Components, Test No. FLAGT-2

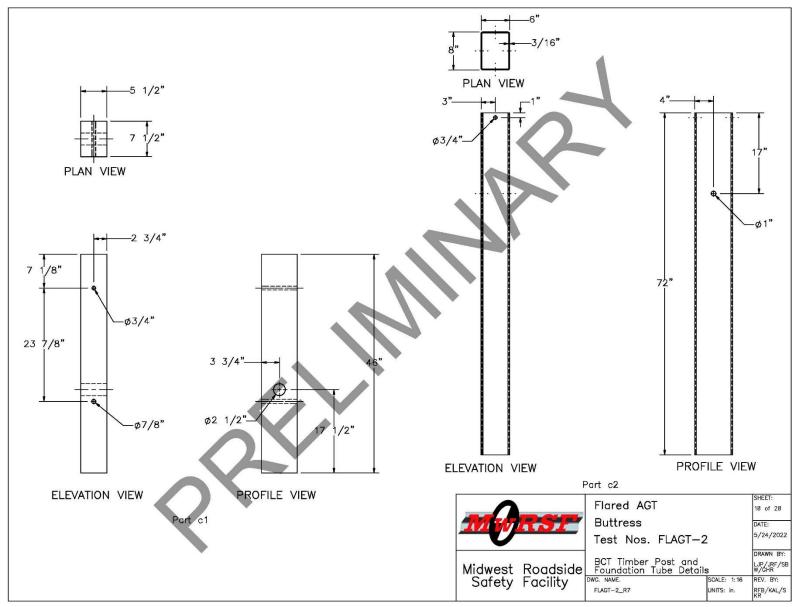


Figure 21. BCT Timber Post and Foundation Tube Details, Test No. FLAGT-2

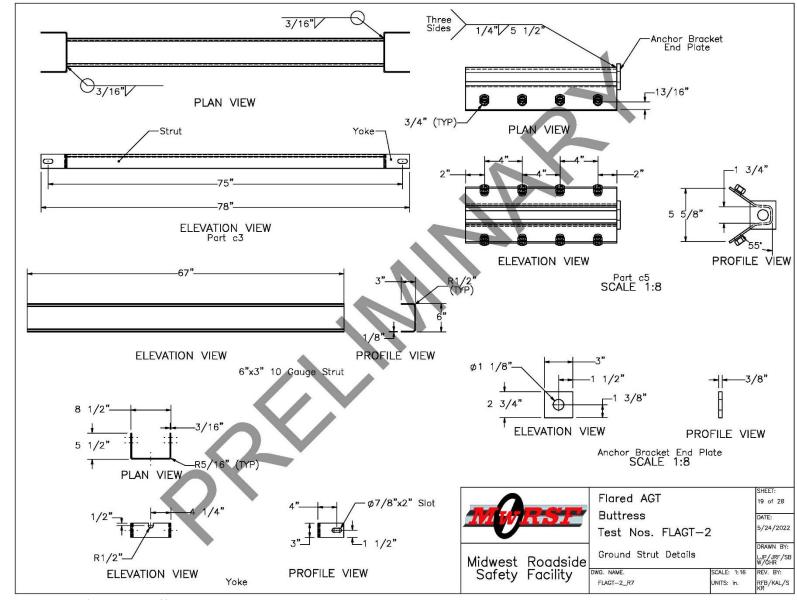


Figure 22. Ground Strut Details, Test No. FLAGT-2

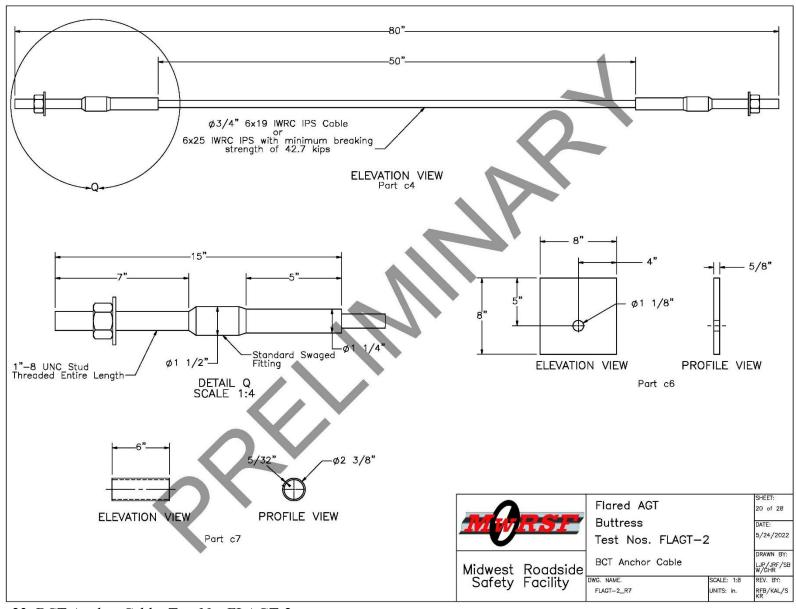


Figure 23. BCT Anchor Cable, Test No. FLAGT-2

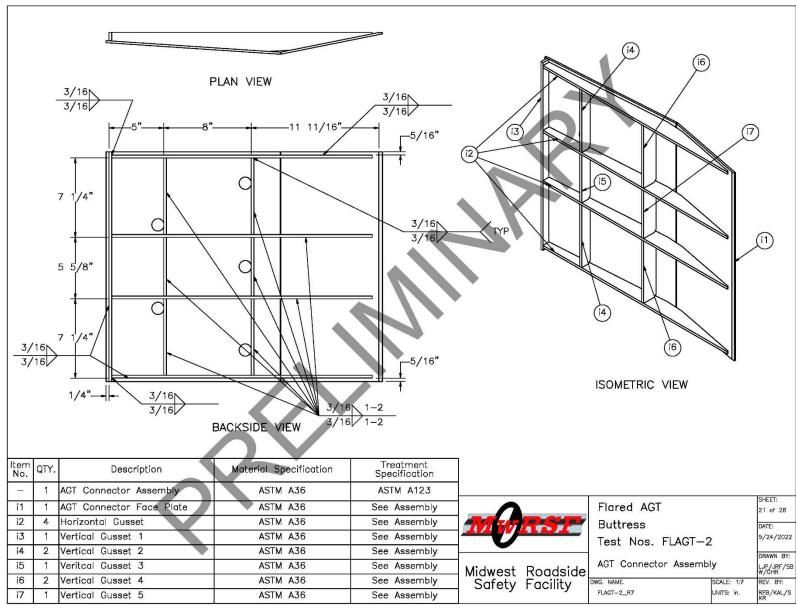


Figure 24. AGT Connector Assembly, Test No. FLAGT-2

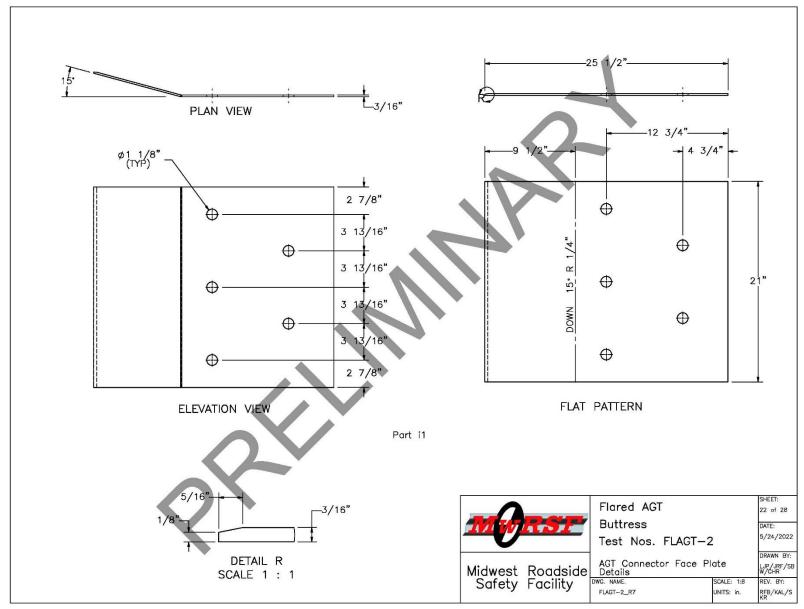


Figure 25. AGT Connector Face Plate Details, Test No. FLAGT-2

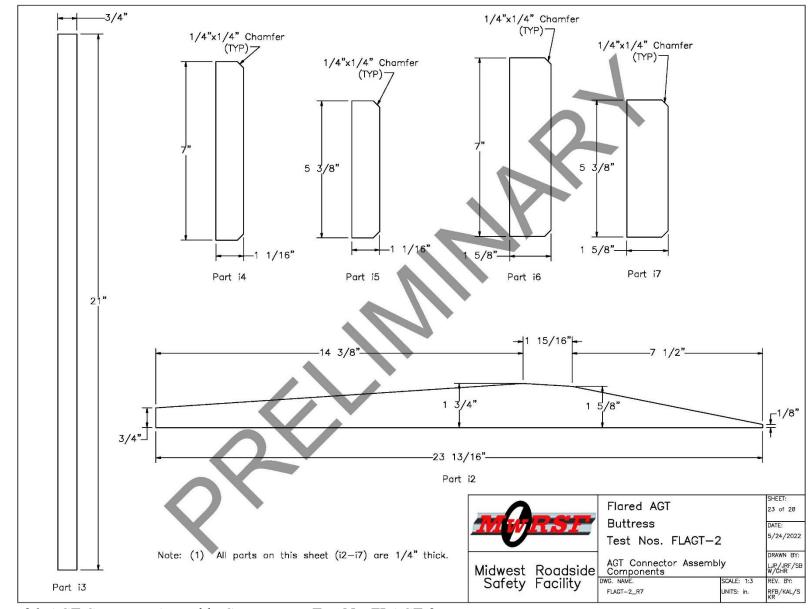


Figure 26. AGT Connector Assembly Components, Test No. FLAGT-2

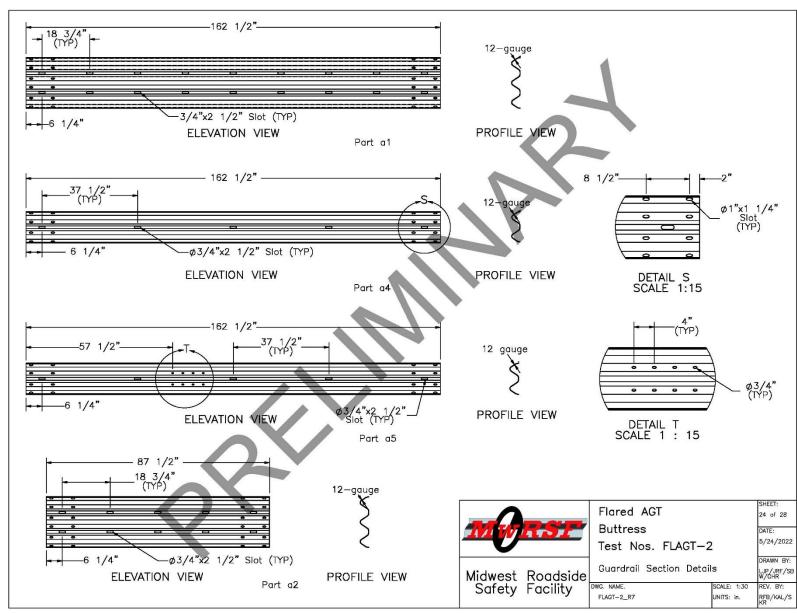


Figure 27. Guardrail Section Details, Test No. FLAGT-2

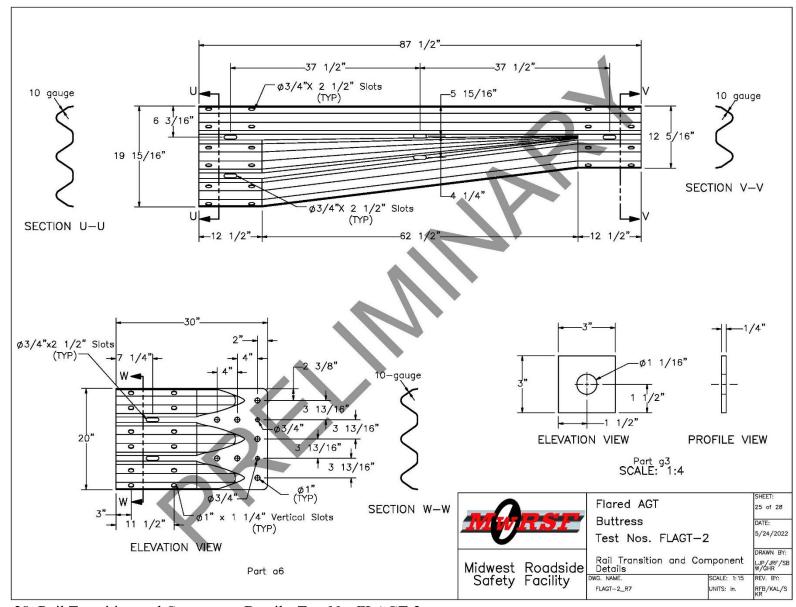


Figure 28. Rail Transition and Component Details, Test No. FLAGT-2

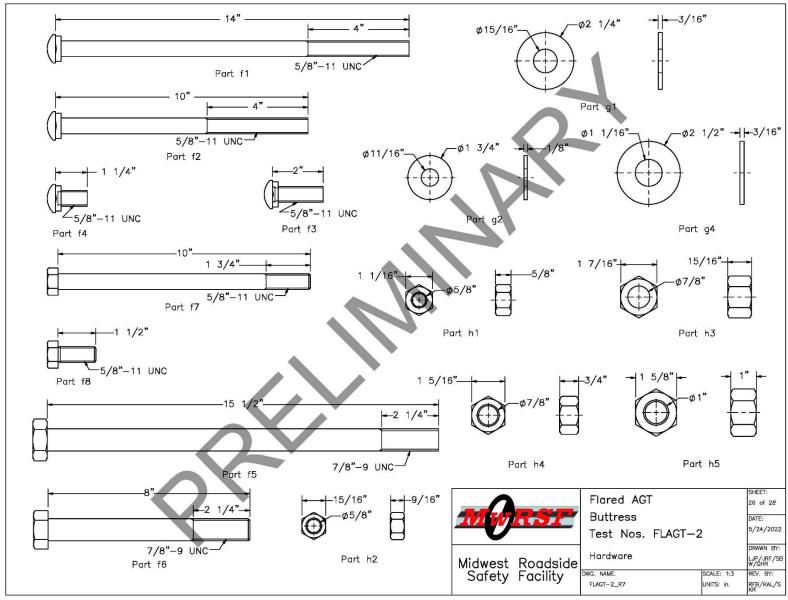
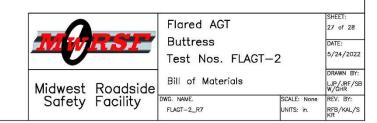


Figure 29. Hardware, Test No. FLAGT-2

Item No.	QTY.	Description	Material Specification	Treatment Specification	Hardware Guide
a1	2	12'-6" 12-gauge Thrie Beam Section	AASHTO M180	ASTM A123 or A653	RTM08a
a2		6'-3" 12-gauge Thrie Beam Section	AASHTO M180	ASTM A123 or A653	RTM19a
a3	1	6'-3" 10-gauge W-Beam to Ihrie-Beam Asymmetric Transition Section	AASHTO M180	ASTM A123 or A653	RWT02
<b>a4</b>		12'-6" 12-gauge W-Beam MGS Section	AASHTO M180	ASTM A123 or A653	RWM04a
a5	1	12'-6" 12-gauge W-Beam MGS End Section	AASHTO M180	ASTM A123 or A653	RWM14a
a6	1	10—gauge Thrie Beam Terminal Connector	AASHTO M180 Gr. 50 Min. yield strength = 50 ksi Min. ultimate strength = 70 ksi	ASTM A123 or A653	RTE01b
Ь1	1	Concrete - 21.9 cubic ft	Min. $f'c = 4,000 psi$	-	_
c1	2	BCT Timber Post — MGS Height	SYP Grade No. 1 or better (No knots +/- 18" from ground on tension face)	-	PDF01
c2	2	72" Long Foundation Tube	ASTM A500 Gr. B	*ASTM A123	PTE06
с3	1	Ground Strut Assembly	ASTM A36	*ASTM A123	-
c4	1	BCT Cable Anchor Assembly	-	-	FCA01
с5	1	Anchor Bracket Assembly	ASTM A36	*ASTM A123	FPA01
с6	1	8"x8"x5/8" Anchor Bearing Plate	ASTM A36	*ASTM A123	FPB01
c7	1	2 3/8" O.D. x 6" Long BCT Post Sleeve	ASTM A53 Gr. B Schedule 40	*ASTM A123	FMM02
d1	13	W6x8.5 or W6x9, 72" Long Steel Post	ASTM A992	*ASTM A123	PWE06
d2		W6x15, 90" Long Steel Post	ASTM A992	*ASTM A123	-
d3	7	6"x12"x14 1/4" Timber Blockout	SYP Grade No.1 or better		PDB10a
d4	167	6"x12"x19" Timber Blockout	SYP Grade No.1 or better	-	_
d5	2000	6"x8"x19" Timber Blockout	SYP Grade No.1 or better	-	-
d6	-	16D Double Head Nail	-		10-
e1		86" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	
e2		62 3/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
еЗ		60 1/2" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
e4		59 1/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
e5	101	74 3/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
e6		37 1/4" Long #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
e7	77700	80 1/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
e8	4	85 1/2" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	_

<sup>\*</sup> Component does not need to be galvanized for testing purposes.

\*\* Rebar does not need to be epoxy-coated for testing purposes.



Note: (1) Quantities listed herein are only for 1 system installation.
(2) For testing purposes part b1 used NE Mix 47B1S/1PF4000HW.

ltem No.	QTY.	Description	Material Specification	Treatment Specification	Hardware Guide
е9	5	80" Long #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	
e10	1	80 1/2" Unbent Length #4 Rebar	ASTM A615 Gr. 60	**Epoxy Coated (ASTM A775 or A934)	-
f1	18	5/8" Dia. UNC, 14" Long Guardrail Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBB06
f2	12	5/8" Dia. UNC, 10" Long Guardrail Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBB03
f3	24	5/8" Dia. UNC, 2" Long Guardrail Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBB02
f4	44	5/8" Dia. UNC, 1 1/4" Long Guardrail Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBB01
f5	5	7/8 Dia. UNC, 15 1/2" Long Heavy Hex Head Bolt	ASTM F3125 Gr. 120 (A325) or A354 Gr. BC	ASTM A153 or B695 Class 55 or F1136 Gr. 3 or F2329 or F2833 Gr. 1	FBX22b
f6	2	7/8" Dia. UNC, 8" Long Hex Head Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBX22a
f7	2	5/8" Dia. UNC, 10" Long Hex Head Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBX16a
f8	8	5/8" Dia. UNC, 1 1/2" Long Hex Head Bolt	ASTM A307 Gr. A	ASTM A153 or B695 Class 55 or F2329	FBX16a
g1	4	7/8" Dia. Plain Round Washer	ASTM F844	ASTM A123 or A153 or F2329	FWC20a
g2		5/8" Dia. Plain Round Washer	ASTM F844	ASTM A123 or A153 or F2329	FWC16a
g3	5	3 <sup>"</sup> x3"x1/4" or 3 1/2"x3 1/2"x1/4" Square Washer Plate	ASTM A572 Gr. 50	*ASTM A123	-
g4	-	1" Dia. Plain Round Washer	ASTM F844	ASTM A153 (AASHTO M232) for Class D or ASTM B695 (AASHTO M298) for Class 50	FWC24a
h1	100	5/8" Dia. Heavy Hex Nut	ASTM A563A	ASTM A153 (AASHTO M232) for Class C or ASTM B695 (AASHTO M298) for Class 50	FNX16b
h2	8	5/8" Dia. Hex Nut	ASTM A563A	ASTM A153 (AASHTO M232) for Class C or ASTM B695 (AASHTO M298) for Class 50	FNX16a
h3	5	7/8" Dia. UNC Heavy Hex Nut	ASTM A563DH or A194 Gr. 2H	_	FNX22b
h4	2	7/8" Dia. Hex Nut	ASTM A307	ASTM A153 (AASHTO M232) for Class C or ASTM B695 (AASHTO M298) for Class 50	-
h5	2	1"-8 UNC Heavy Hex Nut	ASTM A563DH or equivalent	ASTM A153 or B695 Class 55 or F2329	FNX24b
i1	1	AGT Connector Face Plate	ASTM A36	See Assembly	=
i2	4	Horizontal Gusset	ASTM A36	See Assembly	_
i3	1	Vertical Gusset 1	ASTM A36	See Assembly	_
i4	10000	Vertical Gusset 2	ASTM A36	See Assembly	_
i5	1	Veritcal Gusset 3	ASTM A36	See Assembly	
i6	2	Vertical Gusset 4	ASTM A36	See Assembly	<u> 2010</u>
i7	1	Vertical Gusset 5	ASTM A36	See Assembly	-

		Flared AGT		SHEET: 28 of 28
THE WAY	RSF	Buttress		DATE:
		Test Nos. FLAGT-2	2	5/24/2022
Midwest	Roadside	Bill of Materials		DRAWN BY: LJP/JRF/SB W/GHR
		DWG. NAME. FLAGT-2_R7	SCALE: None UNITS: in.	REV. BY: RFB/KAL/S KR

Figure 31. Bill of Material, Test No. FLAGT-2, Cont.

<sup>\*</sup> Component does not need to be galvanized for testing purposes.
\*\* Rebar does not need to be epoxy-coated for testing purposes.





Figure 32. Test Installation Photos, Test No. FLAGT-2





Figure 33. Test Installation Photos, Test No. FLAGT-2





Figure 34. Test Installation Photos, Test No. FLAGT-2





# 4 TEST REQUIREMENTS AND EVALUATION CRITERIA

# **4.1 Test Requirements**

Longitudinal barriers, such as AGTs, must satisfy impact safety standards in order to be declared eligible for federal reimbursement by the Federal Highway Administration (FHWA) for use on the National Highway System. For new hardware, these safety standards consist of the guidelines and procedures published in MASH [4]. According to TL-3 of MASH, longitudinal barriers must be subjected to two full-scale vehicle crash tests, MASH test designation no. 3-20 with the 1100C small car and MASH test designation no. 3-21 with the 2270P pickup truck. However, recent testing has demonstrated that there are two CIPs for an AGT: (1) near the downstream end to maximize snagging on the buttress and (2) near the upstream end to maximize snagging and pocketing at the W-to-thrie transition section. Thus, four full-scale crash tests are required to evaluate the flared AGT, as summarized in Table 1.

TD 11 1	MACTI	TT A	$\sim$ 1		Conditions	0	T '. 1'	1 1	•
Inhiai	$N/I/N \subseteq H$	11 4	rach	Lact	Conditions	tor	Longified	กดไ	Zarriare
I abic i	. WIASII	1 [,	Clasii	1 051	Conditions	IOI	Longitual	nai i	Darricis

Test	l est		Vehicle		oact itions	Evaluation	I Div	
Article	Designation No.	Vehicle Weight lb		Speed mph	Angle deg.	Criteria <sup>1</sup>	Impact Point	
	3-20	1100C	2,420	62	25	A,D,F,H,I	Downstream AGT	
Longitudinal	3-21	2270P	5,000	62	25	A,D,F,H,I	Downstream AGT	
Barrier	3-20	1100C	2,420	62	25	A,D,F,H,I	Upstream AGT	
	3-21	2270P	5,000	62	25	A,D,F,H,I	Upstream AGT	

<sup>&</sup>lt;sup>1</sup> Evaluation criteria explained in Table 2.

## 4.2 Evaluation Criteria

Evaluation criteria for full-scale vehicle crash testing are based on three appraisal areas: (1) structural adequacy; (2) occupant risk; and (3) vehicle trajectory after collision. Criteria for structural adequacy are intended to evaluate the ability of the flared AGT to contain and redirect impacting vehicles. In addition, controlled lateral deflection of the test article is acceptable. Occupant risk evaluates the degree of hazard to occupants in the impacting vehicle. Post-impact vehicle trajectory is a measure of the potential of the vehicle to result in a secondary collision with other vehicles and/or fixed objects, thereby increasing the risk of injury to the occupants of the impacting vehicle and/or other vehicles. These evaluation criteria are summarized in Table 2 and defined in greater detail in MASH. Each full-scale vehicle crash test was conducted and reported in accordance with the procedures provided in MASH.

In addition to the standard occupant risk measures, the Post-Impact Head Deceleration (PHD), the Theoretical Head Impact Velocity (THIV), and the Acceleration Severity Index (ASI)

were determined and reported. Additional discussion on PHD, THIV and ASI is provided in MASH [4].

Table 2. MASH Evaluation Criteria for Longitudinal Barrier

Structural Adequacy	A.	Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.						
	D.	Detached elements, fragment should not penetrate or show compartment, or present an un or personnel in a work zone. I occupant compartment should 5.2.2 and Appendix E of MAS	potential for penetradue hazard to other to Deformations of, or a l not exceed limits s	ating the occupant raffic, pedestrians, intrusions into, the				
	F.	The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.						
Occupant	H.	Occupant Impact Velocity (OIV) (see Appendix A, Section A5.2.2 of MASH for calculation procedure) should satisfy the following limits:						
Risk		Occupant In	npact Velocity Limit	ts				
		Component	Preferred	Maximum				
		Longitudinal and Lateral	30 ft/s	40 ft/s				
	I.	The Occupant Ridedown Acceleration (ORA) (see Appendix A, Section A5.2.2 of MASH for calculation procedure) should satisfy the following limits:						
		Occupant Ridedown Acceleration Limits						
		Component	Preferred	Maximum				
		Longitudinal and Lateral	15.0 g's	20.49 g's				

# **4.3 Soil Strength Requirements**

In accordance with Chapter 3 and Appendix B of MASH, foundation soil strength must be verified before any full-scale crash testing can occur. During the installation of a soil dependent system, W6x16 posts are installed near the impact region utilizing the same installation procedures as the system itself. Prior to full-scale testing, a dynamic impact test must be conducted to verify a minimum dynamic soil resistance of 7.5 kips at post deflections between 5 and 20 in. measured at a height of 25 in. If dynamic testing near the system is not desired, MASH permits a static test to be conducted instead and compared against the results of a previously established baseline test. In this situation, the soil must provide a resistance of at least 90% of the static baseline test at deflections of 5, 10, and 15 in. Further details can be found in Appendix B of MASH [4].

#### **5 TEST CONDITIONS**

## **5.1 Test Facility**

The Outdoor Test Site is located at the Lincoln Air Park on the northwest side of the Lincoln Municipal Airport and is approximately 5 miles northwest of the University of Nebraska-Lincoln.

# 5.2 Vehicle Tow and Guidance System

A reverse-cable tow system with a 1:2 mechanical advantage was used to propel the test vehicle. The distance traveled and the speed of the tow vehicle were one-half that of the test vehicle. The test vehicle was released from the tow cable before impact with the barrier system. A digital speedometer on the tow vehicle increased the accuracy of the test vehicle impact speed.

A vehicle guidance system developed by Hinch [14] was used to steer the test vehicle. A guide flag, attached to the left-front wheel and the guide cable, was sheared off before impact with the barrier system. The 3/8-in. diameter guide cable was tensioned to approximately 3,500 lb and supported both laterally and vertically every 100 ft by hinged stanchions. The hinged stanchions stood upright while holding up the guide cable, but as the vehicle was towed down the line, the guide flag struck and knocked each stanchion to the ground.

#### 5.3 Test Vehicles

For test no. FLAGT-2, a 2015 Ram 1500 quad cab pickup truck was used as the test vehicle. The curb, test inertial, and gross static vehicle weights were 4,900 lb, 5,000 lb, and 5,161 lb, respectively. The test vehicle is shown in Figures 35 and 36, and vehicle dimensions are shown in Figure 37.

The longitudinal component of the center of gravity (c.g.) was determined using the measured axle weights. The Suspension Method [15] was used to determine the vertical component of the c.g. for the pickup truck. This method is based on the principle that the c.g. of any freely suspended body is in the vertical plane through the point of suspension. The vehicle was suspended successively in three positions, and the respective planes containing the c.g. were established. The intersection of these planes pinpointed the final c.g. location for the test inertial condition. The location of the final c.g. is shown in Figures 37 and 38. Data used to calculate the location of the c.g. and ballast information are shown in Appendix C.

Square, black- and white-checkered targets were placed on the vehicle for reference to be viewed from the high-speed digital video cameras and aid in the video analysis, as shown in Figure 38. Round, checkered targets were placed at the c.g. on the left-side door, the right-side door, and the roof of the vehicle.

The front wheels of the test vehicle were aligned to vehicle standards except the toe-in value was adjusted to zero such that the vehicles would track properly along the guide cable. A 5B flash bulb was mounted under the vehicle's windshield wiper and was fired by a pressure tape switch mounted at the impact corner of the bumper. The flash bulb was fired upon initial impact with the test article to create a visual indicator of the precise time of impact on the high-speed

digital videos. A remote-controlled brake system was installed in the test vehicle so the vehicle could be brought safely to a stop after the test.







Figure 35. Test Vehicle, Test No. FLAGT-2









Figure 36. Test Vehicle's Interior Floorboards and Undercarriage, Test No. FLAGT-2

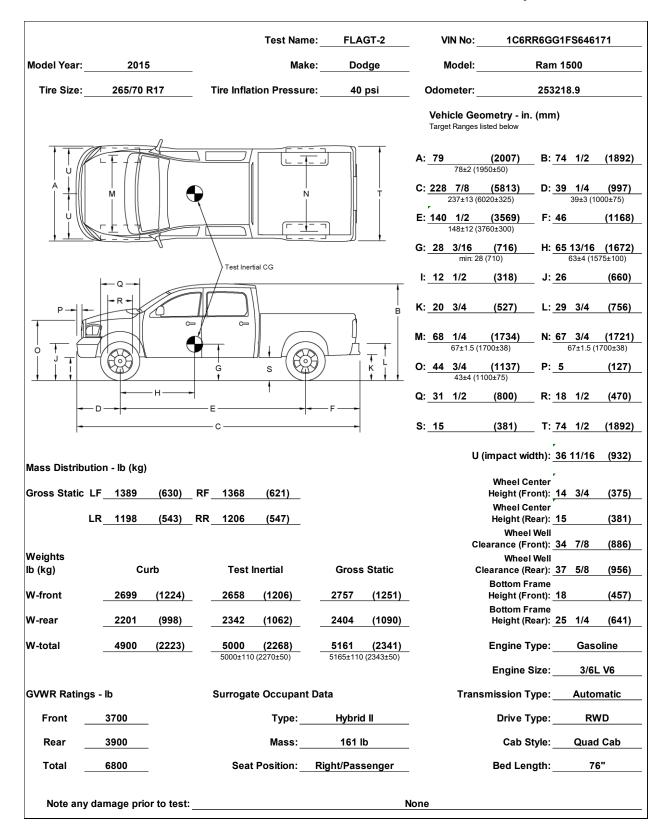


Figure 37. Vehicle Dimensions, Test No. FLAGT-2

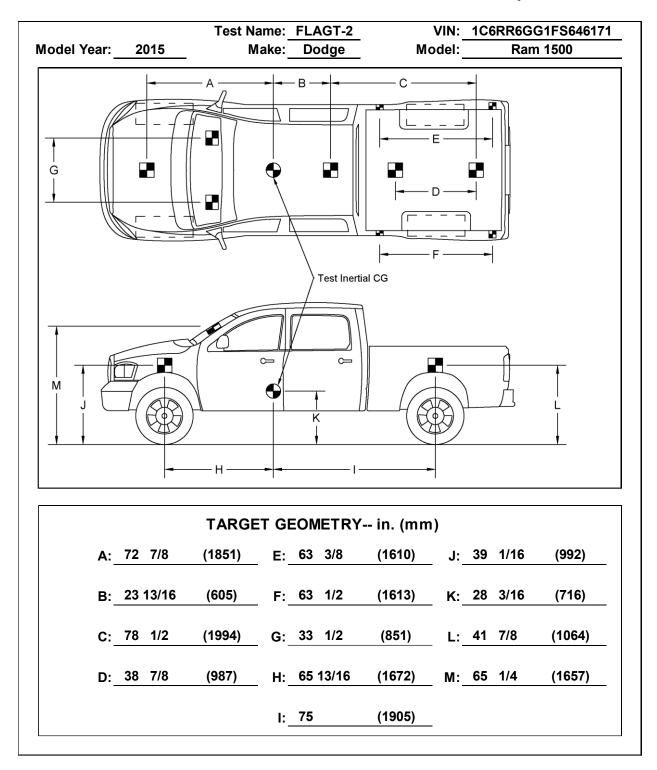


Figure 38. Target Geometry, Test No. FLAGT-2

## **5.4 Simulated Occupant**

For test no. FLAGT-2, a Hybrid II 50<sup>th</sup>-Percentile Adult Male Dummy equipped with footwear was placed in the right-front seat of the test vehicle with the seat belt fastened. The simulated occupant had a final weight of 160 lb. As recommended by MASH, the simulated occupant weight was not included in calculating the c.g. location.

# **5.5 Data Acquisition Systems**

#### 5.5.1 Accelerometers and Rate Transducers

The accelerometer and rate transducer systems used in the full-scale crash testing were the SLICE-1 and SLICE-2 units described below. The units were positioned near the c.g. of the test vehicle and the SLICE-2 unit was designated as primary. Data obtained in dynamic testing was filtered using the SAE Class 60 and the SAE Class 180 Butterworth filter conforming to the SAEJ211/1 specifications [16].

The SLICE-1 and SLICE-2 units were modular data acquisition systems manufactured by Diversified Technical Systems, Inc. of Seal Beach, California. Triaxial acceleration and angular rate sensor modules were mounted inside the bodies of custom-built SLICE 6DX event data recorders equipped with 7GB of non-volatile flash memory and recorded data at 10,000 Hz to the onboard microprocessor. The accelerometers had a range of ±500g's in each of three directions (longitudinal, lateral, and vertical) and a 1,650 Hz (CFC 1000) anti-aliasing filter. The SLICE MICRO Triax ARS had a range of 1,500 degrees/sec in each of three directions (roll, pitch, and yaw). The raw angular rate measurements were downloaded, converted to the proper Euler angles for analysis, and plotted. The "SLICEWare" computer software program and a customized Microsoft Excel worksheet were used to analyze and plot both the accelerometer and angular rate sensor data.

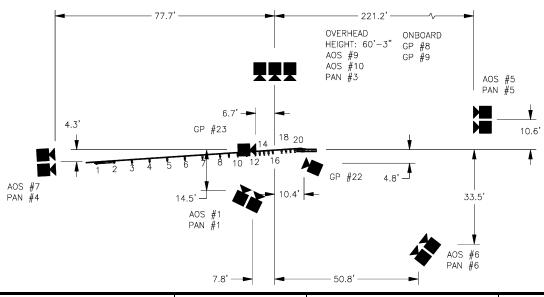
## **5.5.2** Retroreflective Optic Speed Trap

The retroreflective optic speed trap was used to determine the speed of the test vehicle before impact. Five retroreflective targets, spaced at approximately 18-in. intervals, were applied to the side of the vehicle. When the emitted beam of light was reflected by the targets and returned to the Emitter/Receiver, a signal was sent to the data acquisition computer, recording at 10,000 Hz, as well as the external LED box activating the LED flashes. The speed was then calculated using the spacing between the retroreflective targets and the time between the signals. LED lights and high-speed digital video analysis are only used as a backup in the event that vehicle speeds cannot be determined from the electronic data.

## 5.5.3 Digital Photography

Six AOS high-speed digital video cameras, four GoPro digital video cameras, and five Panasonic digital video cameras were utilized to film test no. FLAGT-2. Camera details, camera operating speeds, lens information, and a schematic of the camera locations relative to the system are shown in Figure 39. Due to technical difficulties, camera AOS-10 did not record the impact event.

The high-speed videos were analyzed using TEMA Motion software program. Actual camera speed and camera divergence factors were considered in the analysis of the high-speed videos. A digital still camera was also used to document pre- and post-test conditions.



No.	Туре	Operating Speed (frames/sec)	Lens	Lens Setting
AOS-1	AOS Vitcam	500	KOWA 25mm Fixed	-
AOS-5	AOS X-PRI Gigabit	500	100mm Fixed	-
AOS-6	AOS X-PRI Gigabit	500	Fujinon 50mm Fixed	-
AOS-7	AOS X-PRI Gigabit	500	Fujinon 35mm Fixed	-
AOS-9	AOS TRI-VIT 2236	1000	KOWA 12mm Fixed	-
AOS-10*	AOS TRI-VIT 2236	500	KOWA 16mm Fixed	-
GP-8	GoPro Hero 4	120		
GP-9	GoPro Hero 4	120		
GP-22	GoPro Hero 7	240		
GP-23	GoPro Hero 7	240		
PAN-1	Panasonic HC-V770	120		
PAN-3	Panasonic HC-V770	120		
PAN-4	Panasonic HC-V770	120		
PAN-5	Panasonic HC-VX981	120		
PAN-6	Panasonic HC-VX981	120		

<sup>\*</sup>Camera did not record impact event due to technical difficulties.

Figure 39. Camera Locations, Speeds and Lens Settings, Test No. FLAGT-2

#### 6 FULL-SCALE CRASH TEST NO. FLAGT-2

#### **6.1 Static Soil Test**

Before full-scale crash test no. FLAGT-2 was conducted, the strength of the foundation soil was evaluated with a static test, as described in MASH. The static test results, as shown in Appendix D, demonstrated a soil resistance above the baseline test limits. Thus, the soil provided adequate strength, and full-scale crash testing could be conducted on the barrier system.

## **6.2 Weather Conditions**

Test no. FLAGT-2 was conducted on September 21, 2021 at approximately 2:00 p.m. The weather conditions as per the National Oceanic and Atmospheric Administration (station 14939/KLNK) were reported and are shown in Table 3.

Table 3. Weather Conditions, T	Γest No. FLA	GT-2
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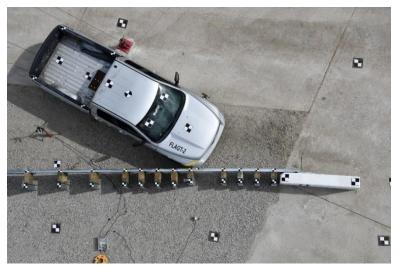
Temperature	73° F
Humidity	30%
Wind Speed	11 mph
Wind Direction	350° from True North
Sky Conditions	Overcast
Visibility	10.00 Statute Miles
Pavement Surface	Dry
Previous 3-Day Precipitation	0.02 in.
Previous 7-Day Precipitation	0.05 in.

## **6.3 Test Description**

Similar to previous test no. FLAGT-1, initial vehicle impact was to occur 93 in. from the upstream end of the concrete buttress, as shown in Figure 40, which was selected during Phase I of this project using computer simulations to maximize the probability of vehicle snag [5]. In test no. FLAGT-2, the 5,000-lb quad cab pickup truck impacted the 15:1 flared AGT at a speed of 62.6 mph and at an angle of 25.4 degrees, which corresponded to 29.2 degrees relative to the flared guardrail. The actual point of impact was 91.5 in. from the upstream end of the buttress.

The vehicle was contained and redirected by the AGT with a 15:1 flare and enlarged transition posts located within the nested thrie beam region of the AGT. The larger posts helped reduce the system deflections to roughly half of those observed during the previous test, test no. FLAGT-1. During redirection, contact with the guardrail caused the front wheel to disengage from the vehicle and be pushed backward against the vehicle's toe pan, causing significant occupant compartment deformations. After the vehicle exited the system, the absence of a front wheel allowed the pickup truck to continue rolling toward the barrier as it traveled away from the test installation. The vehicle eventually stabilized and returned to an upright position without rolling over. The brakes were applied remotely, and the vehicle came to rest 120.7 ft downstream from the impact location and 35 ft front of the barrier.

A detailed description of the sequential impact events is contained in Table 4. Sequential photographs are shown in Figures 41 and 42. Documentary photographs of the crash test are shown in Figure 43. The vehicle trajectory and final position are shown in Figure 44





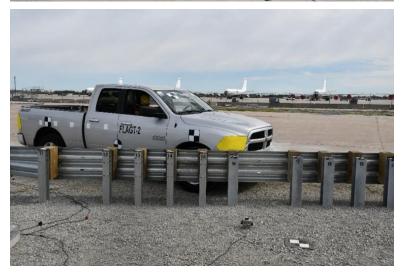


Figure 40. Impact Location, Test No. FLAGT-2

Table 4. Sequential Description of Impact Events, Test No. FLAGT-2

TIME	EVENT
(sec)	E VEIVI
0.000	Vehicle's front bumper contacted rail from the target impact location.
0.012	Vehicle's right fender, right headlight contacted rail and deformed. Vehicle's right-front tire contacted rail.
0.016	Rail deformed between post nos. 16 and 17. Post nos. 14 through 20 rotated backward.
0.018	Vehicle's grille contacted rail and deformed, vehicle's hood deformed, and vehicle's right-front tire deflated.
0.036	Vehicle's front bumper detached from left fender. Vehicle yawed away from barrier and rolled toward barrier.
0.046	Vehicle's right-front door contacted rail and bent.
0.050	Vehicle's right-front wheel disengaged from vehicle.
0.062	Blockouts at post nos. 17 and 18 fractured.
0.078	Vehicle's roof deformed and vehicle's right headlight disengaged.
0.086	Vehicle's windshield cracked.
0.102	Vehicle pitched downward and right-front wheel disengaged from the vehicle.
0.122	Concrete spalled on upstream end of terminal and vehicle's right-front window glass shattered.
0.126	Vehicle's left-front wheel became airborne.
0.155	Vehicle's grille disengaged.
0.170	Vehicle's left headlight disengaged. Vehicle pitched upward.
0.214	Vehicle was parallel to system at a speed of 41.7 mph.
0.242	Vehicle's right quarter panel and rear bumper contacted rail and deformed.
0.365	Vehicle exited system at a speed of 36.9 mph and an angle of -6.6 degrees.
1.148	Vehicle reached its maximum roll position and began to roll away from barrier. Vehicle slid on its front-right fender.
2.920	Vehicle's left wheels contacted ground.
4.258	Vehicle came to rest

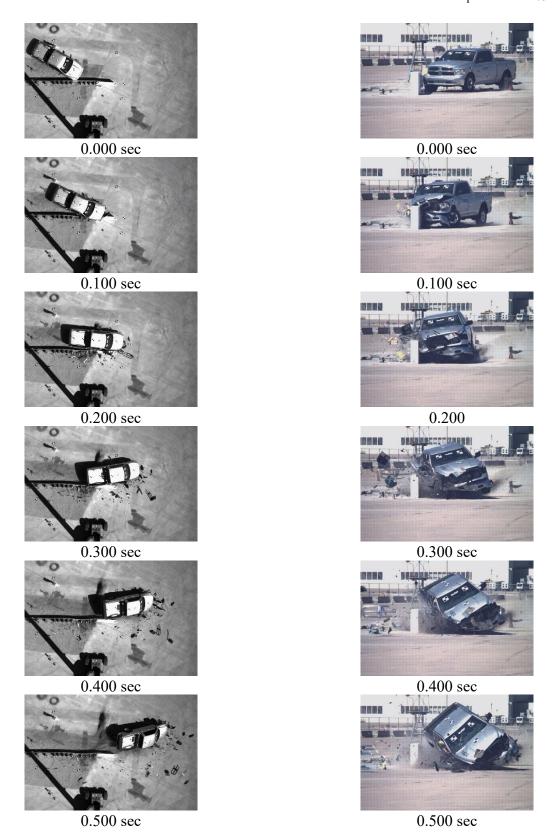


Figure 41. Sequential Photographs, Test No. FLAGT-2

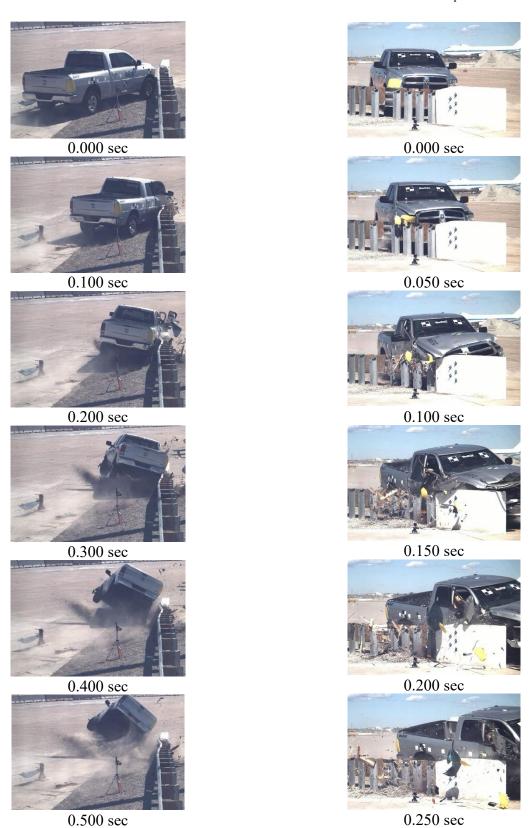


Figure 42. Sequential Photographs, Test No. FLAGT-2

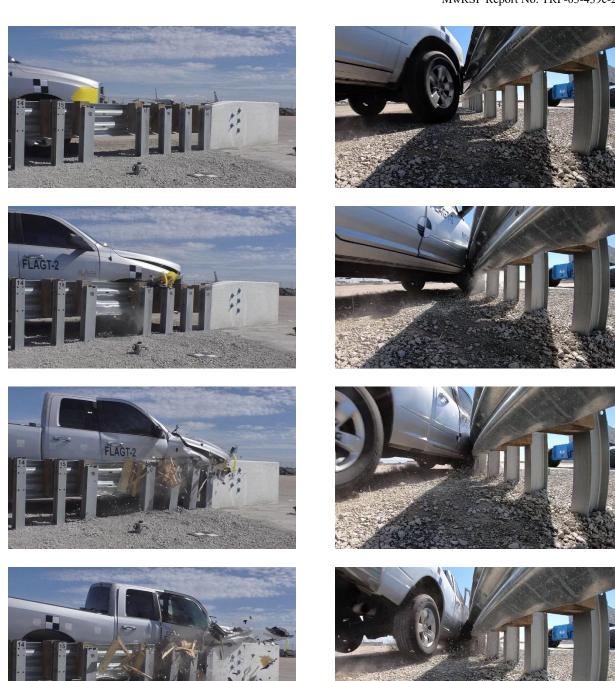


Figure 43. Documentary Photographs, Test No. FLAGT-2

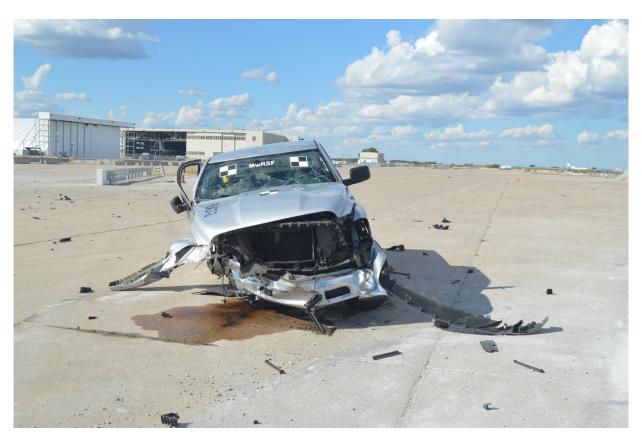




Figure 44. Vehicle Final Position and Trajectory Marks, Test No. FLAGT-2

## **6.4 Barrier Damage**

Damage to the barrier was moderate, as shown in Figures 45 through 49. Barrier damage consisted of contacts marks, deformation to the thrie-beam rail, post rotations, and blockout fracture. The length of vehicle contact along the barrier was 12 ft - 4 in., beginning 4 in. downstream from post no. 16 and extending downstream. Contact marks were observed on the top surface of the concrete buttress beginning at the upstream end and continuing downstream for 47 in., as shown in Figure 46. No contact marks were observed on the lower chamfer of the buttress below the guardrail.

The thrie beam was displaced laterally between post no. 16 and the buttress, and localized kinking, bending, denting, and flattening of the thrie beam were present throughout this region, as show in Figure 47. The bottom corrugation of the rail was flattened and bent backward between post nos. 16 and 19. The middle corrugation of the guardrail was gouged and sharply dented at various locations between post nos. 16 and 20 due to contact with the front wheel rim. An 11-in. long tear through both thrie beam segments was present on the bottom corrugations near post no. 18. It was believed that this tear was caused by the vehicle's upper control arm punching through the guardrail after the front wheel disengaged.

Post nos. 15 through 20 were displaced backward and a small crack in the soil formed along the front flanges of these posts. A soil heave formed behind the posts. Post nos. 18 and 19 were also twisted and deflected downstream. The wooden blockouts at post nos. 17 and 18 were fractured and the attachment bolts at these posts were bent. Contact marks were found on the upstream front flanges of post nos. 17 and 19 and the upstream face of the blockout at post no. 19 above the guardrail.

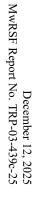














Figure 46. Concrete and Thrie-Beam Damage, Test No. FLAGT-2





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Figure 47. Thrie-Beam Damage, Test No. FLAGT-2





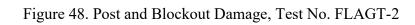










Figure 49. Soil Disturbance, Test No. FLAGT-2

The maximum lateral permanent set of the barrier system was 4.7 in., which occurred at the rail at post no. 17, as measured in the field. The maximum lateral dynamic barrier deflection was 7.9 in. located at the rail at post no. 17, as determined from high-speed video analysis. The working width of the system was found to be 25.0 in., also determined from high-speed video analysis. A schematic of the permanent set deflection, dynamic deflection, and working width is shown in Figure 50.

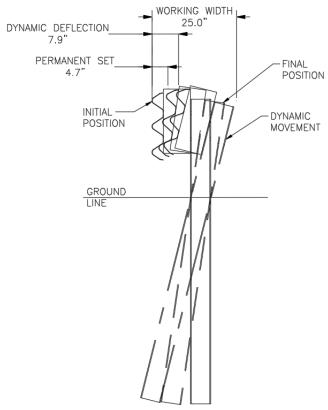


Figure 50. Permanent Set, Dynamic Deflection, and Working Width, Test No. FLAGT-2

## 6.5 Vehicle Damage

The damage to the vehicle was severe, as shown in Figures 51 through 53. The maximum occupant compartment intrusions are listed in Table 5, along with the intrusion limits established in MASH for various areas of the occupant compartment. There were no penetrations into the occupant compartment, however, the toe pan/wheel well deformation of 9.9 in. violated the MASH limit of 9.0 in. Complete occupant compartment and vehicle deformations and the corresponding locations are provided in Appendix E. MASH defines intrusion or deformation as the occupant compartment being deformed and reduced in size with no observed penetration. Outward deformations, which are denoted as negative numbers in Appendix E, are not considered crush toward the occupant and are not evaluated by MASH criteria.

The majority of the damage was concentrated on the right-front corner and right side of the vehicle where the impact had occurred. The bumper cover was disengaged on the right end and the radiator was crushed inward on the right side. The right fender was barely attached to the vehicle after impact, and the frame was completely crushed inward approximately 1 ft at the front

of the vehicle. The right-front door was crushed, and both right-side doors were scraped. The right-rear fender was scraped and gouged. The front end of the left fender was bent outward slightly.

The undercarriage was damaged as well. The right-front shock and spring were disengaged, and the rear shock was dented slightly. The sway bar was shifted to the right side, and there was a scrape on the outside of the right sway bar. The right passenger control arm was fractured and disengaged from the vehicle. The right passenger steering knuckle was disengaged at the ball joint, while the left side steering knuckle was scraped. The right tie rod was disengaged from the vehicle. The gear box was detached from the frame and steering shaft, and the gear box was cracked in several places. The rear axle was bent, and there was a puncture in the oil pan. There were several bends and kinks in the right side of the frame, and the entire frame appeared twisted. There was scraping on the bottom of every cross member on the right side, and the middle cross member appeared to be buckled in the middle. The right-front cab mount was crushed and disengaged. The toe pan was visibly damaged, and there was a small gouge on the right side of the gas tank from contact with the drive shaft.

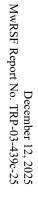










Figure 51. Vehicle Damage, Test No. FLAGT-2

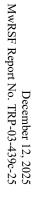




















Figure 53. Vehicle Interior and Undercarriage Damage, Test No. FLAGT-2

Table 5. Maximum Occupant Compartment Intrusion by Location, Test No. FLAGT-2

LOCATION	Maximum Intrusion in.	MASH Allowable Intrusion in.
Wheel Well & Toe Pan	9.9	≤ 9
Floor Pan & Transmission Tunnel	6.5	≤ 12
A-Pillar	0.1	≤ 5
A-Pillar (Lateral)	0.0*	≤ 3
B-Pillar	0.0	≤ 5
B-Pillar (Lateral)	0.0*	≤ 3
Side Front Panel (in Front of A-Pillar)	8.1	≤ 12
Side Door (Above Seat)	0.0*	≤ 9
Side Door (Below Seat)	1.2	≤ 12
Roof	0.0*	≤ <b>4</b>
Windshield	2.5	≤ 3
Side Window	Intact	No shattering resulting from contact with structural member of test article
Dash	4.3	N/A

<sup>\*</sup> Negative values (outward deformation) reported as 0.0. See Appendix E for further information. N/A – No MASH criteria exist for this location.

### 6.6 Occupant Risk

The calculated occupant impact velocities (OIVs) and maximum 0.010-sec average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions, as determined from the accelerometer data, are shown in Table 6. Note that the OIVs and ORAs were within suggested limits, as provided in MASH. The calculated THIV, PHD, and ASI values are also shown in Table 6. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix F.

Table 6. Summary of OIV, ORA, THIV, PHD, and ASI Values, Test No. FLAGT-2

		Trans	ducer	MACH
Evaluation (	Criteria	SLICE-1 (backup)	SLICE-2 (primary)	MASH Limits
OIV	Longitudinal	-31.65	-30.38	±40
ft/s	Lateral	-23.43	-25.56	±40
ORA	Longitudinal	11.41	11.74	±20.49
g's	Lateral	-12.12	-11.48	±20.49
Maximum	Roll	76.0	72.7	±75
Angular Displacement	Pitch	-9.3	-10.6	±75
deg.	Yaw	-58.9	-59.3	not required
THIV –	ft/s	37.80	38.29	not required
PHD – ş	g's	14.67	16.07	not required
ASI		1.45	1.57	not required

### 6.7 Discussion

Analysis of the results from test no. FLAGT-2 showed that the system adequately contained and redirected the 2270P vehicle with controlled lateral displacements of the barrier. The test vehicle did not penetrate nor ride over the barrier. Vehicle roll, pitch, and yaw angular displacements, as shown in Appendix F, were deemed acceptable because they did not adversely influence occupant risk nor cause rollover. The vehicle rolled significantly after exiting the system with a maximum roll angle of 73 degrees recorded by the primary data recorder, but the vehicle remained upright and within the 75-degree maximum roll angle allowed by MASH. After impact, the vehicle exited the barrier at an angle of –6.6 degrees, and its trajectory did not violate the bounds of the exit box. Detached elements, fragments, or other debris from the test article did not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or work-zone personnel. However, deformations of the wheel well and toe pan area of 9.9 in. exceeded the MASH deformation limit of 9 in. Therefore, test no. FLAGT-2 failed to satisfy the safety performance criteria for MASH test designation no. 3-21. A summary of the test results and sequential photographs are shown in Figure 54.

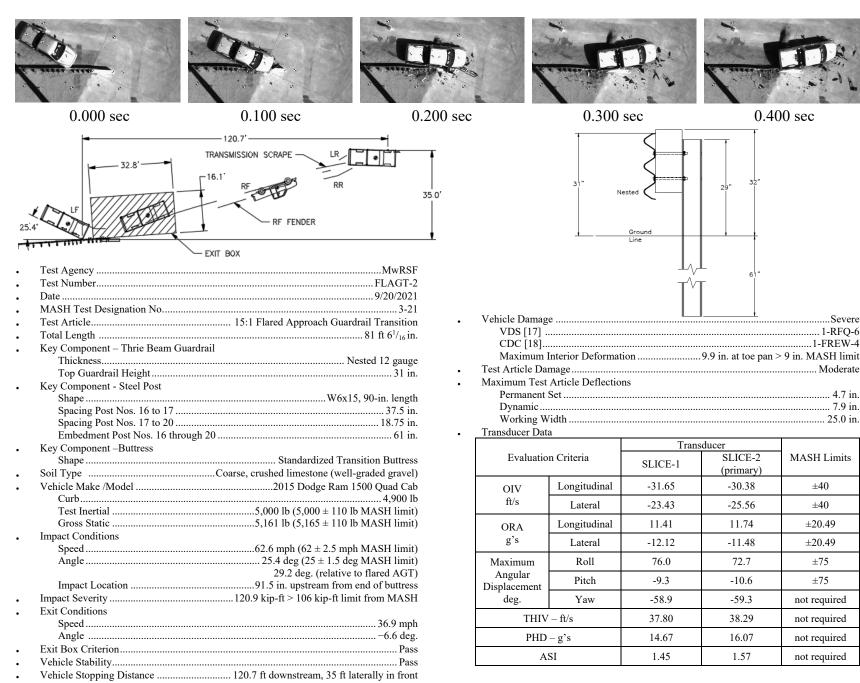


Figure 54. Summary of Test Results and Sequential Photographs, Test No. FLAGT-2

### 7 SUMMARY AND CONCLUSIONS

The main research objective for this project was to develop guidelines for flaring approach guardrail transitions. Previously, an LS-DYNA simulation study selected a maximum flare rate of 15:1 and a critical AGT to evaluate to MASH TL-3 criteria [5]. However, test no. FLAGT-1 resulted in the 2270P vehicle snagging on the concrete buttress, a longitudinal ORA value exceeding the MASH limits, and occupant compartment deformations that exceeded MASH limits for the toe pan and floorboard regions of the vehicle [6]. Thus, the research efforts documented herein focused on redesigning the flared AGT and retesting the modified barrier system to MASH test designation no. 3-21 criteria.

An analysis of test no. FLAGT-1 revealed the main reason for the test failure was excessive deflections of the AGT posts, which allowed the vehicle to snag on the upstream end of the buttress. To stiffen the AGT, the 6.5-ft long W6x9 posts located within the nested thrie beam area were replaced by 7.5-ft long W6x15 posts. The new posts had increased cross sectional strength and an increased embedment depth to create higher soil resistance in the system. No other modifications were made to the 15:1 flared AGT installation.

In test no. FLAGT-2, the 5,000-lb quad pickup truck impacted the modified, flared AGT 91.5 in. upstream from the end of the buttress at a speed of 62.6 mph and an angle of 25.4 degrees. The flared AGT contained and redirected the 2270P vehicle, and all ORA and OIV values were within MASH recommended limits. However, during the impact event, the front wheel was disengaged from the vehicle and pushed backward toward the occupant compartment. Maximum deformations to the toe pan and floorboard area of the vehicle measured 9.9 in., which exceeded the MASH limit of 9.0 in. for this region of the occupant compartment. Thus, test no. FLAGT-2 failed to satisfy MASH TL-3 safety performance criteria. After the vehicle exited the system, the absence of the front wheel allowed the vehicle to continue to roll toward the test installation, ultimately reaching a maximum roll angle of 73 degrees prior to returning to its upright position. Note, this maximum roll angle remained within the MASH limit of 75 degrees. A summary of the test evaluation is shown in Table 7.

A review of the high-speed video from test no. FLAGT-2 showed that the front wheel had disengaged from the vehicle early in the impact event. The wheel stopped rotating and was pushed backward against the wheel well when the wheel was located between post nos. 16 and 17 of the AGT. Inspection of the guardrail between post nos. 16 and 17 revealed sharp denting or gouging to the middle corrugation. Researchers believe the front wheel rim gouged into the middle corrugation, thus causing rim snag and high longitudinal forces to be imparted to the wheel and ultimately disengaging the wheel from the vehicle. The disengaged wheel remained within the wheel well and pressed against the guardrail. Gouging of the middle corrugation of the thrie beam was found at various locations between post nos. 17 and 20, indicating that the wheel rim continued to gouge and snag on the guardrail after it had disengaged form the vehicle. This continued rim snag likely led to increased loading and deformation to the toe pan region of the occupant compartment. Based on this test failure analysis, it was determined that the severity of the rim-to-guardrail gouging and snag within the flared AGT must be reduced in order to reduce the occupant compartment deformation and satisfy MASH TL-3 safety performance criteria.

Table 7. Summary of Safety Performance Evaluation

Evaluation Factors		Evaluat	Test No. FLAGT-2								
Structural Adequacy	A.	to a controlled stop; the vehi	Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.								
	D.	should not penetrate or show	1. Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.								
		2. Deformations of, or intrushould not exceed limits set for MASH.	U								
	F.	F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.									
Occupant	Н.	H. Occupant Impact Velocity (OIV) (see Appendix A, Section A5.2.2 of MASH for calculation procedure) should satisfy the following limits:									
Risk		Occupant Is	S								
		Component	Preferred	Maximum							
		Longitudinal and Lateral	30 ft/s	40 ft/s							
	I.	I. The Occupant Ridedown Acceleration (ORA) (see Appendix A, Section A5.2.2 of MASH for calculation procedure) should satisfy the following limits:									
		Occupant Ride	down Acceleration Lin	nits	S						
		Component	Preferred	Maximum							
		Longitudinal and Lateral	15.0 g's	20.49 g's							
MASH Test Designation No.											
Final Evaluation (Pass or Fail)											

 $S-Satisfactory \qquad U-Unsatisfactory \qquad NA-Not Applicable$ 

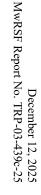
### **8 REFERENCES**

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# 9 APPENDICES

# **Appendix A. A2LA Accreditation Certificates**





# **Accredited Laboratory**

A2LA has accredited

# MIDWEST ROADSIDE SAFETY FACILITY (MWRSF)

Lincoln, NE

for technical competence in the field of

# Mechanical Testing

This laboratory is accredited in accordance with the recognized International Standard ISO/IEC 17025:2017

General requirements for the competence of testing and calibration laboratories. This accreditation demonstrates technical competence for a defined scope and the operation of a laboratory quality management system (refer to joint ISO-ILAC-IAF Communiqué dated April 2017).



Presented this 12th day of February 2020.

Vice President, Accreditation Services For the Accreditation Council Certificate Number 2937.01 Valid to November 30, 2021

For the tests to which this accreditation applies, please refer to the laboratory's Mechanical Scope of Accreditation.

Figure A-1. Midwest Roadside Safety Facility A2LA Accreditation Certificate No. 2937.01



#### SCOPE OF ACCREDITATION TO ISO/IEC 17025:2017

MIDWEST ROADSIDE SAFETY FACILITY (MwRSF)1 University of Nebraska-Lincoln 4630 NW 36th Street Lincoln, NE 68524 Ms. Karla Lechtenberg Phone: 402 472 9070

#### MECHANICAL

Valid To: November 30, 2021 Certificate Number: 2937.01

In recognition of the successful completion of the A2LA evaluation process, accreditation is granted to this laboratory to perform the following tests:

<u>Tests</u>	Test Methods <sup>2</sup>
Full-Scale Vehicle Crash Tests of Highway Safety Features	NCHRP Report 350; MASH; EN 1317
Full-Scale Vehicle Crash Tests of Perimeter Protection Systems and Access Control Devices	ASTM F2656; SD-STD-02.01 Revision A
Bogie Dynamic Tests of Highway Safety Features	Non-Standard Test Method: Dynamic Testing of Steel Post and Rigid Foundation; Non-Standard Test Method: Dynamic Testing of Post in Soil; Non-Standard Test Method: Dynamic Testing of Spacer Blocks
Crushable Nose Bogie Testing for Breakaway Supports	Non-Standard Test Method: Dynamic Testing of Breakaway Supports; AASHTO Breakaway Poles and Supports; NCHRP Report 350

On the following types of products, materials, and/or structures:

Metal, Wood, Concrete and Plastic Structures, Components of Structures, Fasteners, and Roadway Pavements.

(A2LA Cert. No. 2937.01) 02/12/2020

5202 Presidents Court, Suite 220 | Frederick, MD 21703-8515 | Phone: 301 644 3248 | Fax: 240 454 9449 | www.A2LA.org

Figure A-2. Midwest Roadside Safety Facility Scope of Accreditation to ISO/IEC 17025

<sup>&</sup>lt;sup>1</sup> Administrative office located at: 2200 Vine Street, 130 Whittier Building, Lincoln, NE 68583-0853.

<sup>&</sup>lt;sup>2</sup> This laboratory meets A2LA R104 - General Requirements: Accreditation of Field Testing and Field Calibration Laboratories for these tests.

# Appendix B. Material Specifications

Table B-1. Bill of Materials, Test No. FLAGT-2

Item No.	Description	Material Specification	Reference
a1	12'-6" 12-gauge Thrie Beam Section	AASHTO M180	H#L33120
a2	6'-3" 12-gauge Thrie Beam Section	AASHTO M180	H#L33720
a3	6'-3" 10-gauge W-Beam to Thrie- Beam Asymetric Transition Section	AASHTO M180	H#250344
a4	12'-6" 12-gauge W-Beam Section	AASHTO M180	H#C85187
a5	12'-6" 12-gauge W-Beam End Section	AASHTO M180	H#9411949
a6	10-ga Thrie Beam Terminal Connector	AASHTO M180 Gr. 50	H#833M66260
bl	Concrete - 21.9 cubic ft	Min. $f'c = 4,000 \text{ psi}$	Sample #011, FLAGT1, FLAGT2
c1	BCT Timber Post - MGS Height	SYP Grade No. 1 or better	C.o.C. 9/21/2020
c2	72" Long Foundation Tube	ASTM A500 Gr. B	H#821T08220
c3	Ground Strut Assembly	ASTM A36	H#163375
c4	BCT Cable Anchor Assembly	-	C.o.C. 9/24/2018
c5	Anchor Bracket Assembly	ASTM A36	H#V911470
c6	8"x8"x5/8" Anchor Bearing Plate	ASTM A36	H#4181496
c7	2 3/8" O.D. x 6" Long BCT Post Sleeve	ASTM A53 Gr. B Schedule 40	H#8712810
d1	W6x8.5 or W6x9, 72" Long Steel Post	ASTM A992	H#55066501/03
d2	W6x15, 90" Long Steel Post	ASTM A992	H#59098849/03
d3	6"x12"x14 1/4" Timber Blockout	SYP Grade No.1 or better	C.o.C. 4/1/2020
d4	6"x12"x19" Timber Blockout	SYP Grade No.1 or better	C.o.C. 9/24/2020
d5	6"x8"x19" Timber Blockout	SYP Grade No.1 or better	C.o.C. 6/21/2021
d6	16D Double Head Nail	-	C.o.C. for PO E000548963
e1	86" Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e2	62 3/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e3	60 1/2" Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e4	59 1/4" Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e5	74¾" Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e6	371/4 " Long #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e7	801/4 " Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740

Table B-2. Bill of Materials, Test No. FLAGT-2, Cont.

Item No.	Description	Material Specification	Reference
e8	85½ " Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e9	80" Long #4 Rebar	ASTM A615 Gr. 60	H#3600014740
e10	80½ " Unbent Length #4 Rebar	ASTM A615 Gr. 60	H#3600014740
f1	5/8" Dia. UNC, 14" Long Guardrail Bolt	ASTM A307 Gr. A	H#100897520
f2	5/8" Dia. UNC, 10" Long Guardrail Bolt	ASTM A307 Gr. A	H#10666100
f3	5/8" Dia. UNC, 2" Long Guardrail Bolt	ASTM A307 Gr. A	H#10439100
f4	%Dia. UNC, 1 1/4" Long Guardrail Bolt	ASTM A307 Gr. A	H#10634210
f5	%" Dia. UNC, 15 1/2" Long Heavy Hex Bolt	ASTM F3125 Gr. 120 (A325)	H#3093334
f6	7/8" Dia. UNC, 8" Long Hex Bolt	ASTM A307 Gr. A	FASTENAL C.o.C. 04/12/2018
f7	5/8" Dia. UNC, 10" Long Hex Bolt	ASTM A307 Gr. A	H#JK18104124
f8	5/8" Dia. UNC, 1 1/2" Long Hex Bolt	ASTM A307 Gr. A	H#5-01571
g1	7/8" Dia. Plain Round Washer	ASTM F844	L#1844804 PO#170089822
g2	5/8" Dia. Plain Round Washer	ASTM F844	L#20200515 C#000825
g3	3"x3"x½" or 3½"x½"x½"x¼" Square Washer Plate	ASTM A572 Gr. 50	H#B9L648
g4	1" Dia. Plain Round Washer	ASTM F844	P#33188
h1	5/8" Dia. Heavy Hex Nut	ASTM A563A	H#10640980
h2	5/8" Dia. Hex Nut	ASTM A563A	H#331608011
h3	%" Dia. UNC Heavy Hex Nut	ASTM A563DH or A194 Gr. 2H	H#190841
h4	7/8" Dia. Hex Nut	ASTM A307	H#331704677
h5	1"-8 UNC Heavy Hex Nut	ASTM A563DH	FASTENAL C.o.C. 11/29/2018
i1	AGT Connector Face Plate	ASTM A36	H#Y6325
i2	Horizontal Gusset	ASTM A36	H#813L65970
i3	Vertical Gusset 1	ASTM A36	H#813L65970
i4	Vertical Gusset 2	ASTM A36	H#813L65970
i5	Veritcal Gusset 3	ASTM A36	H#813L65970
i6	Vertical Gusset 4	ASTM A36	H#813L65970
i7	Vertical Gusset 5	ASTM A36	H#813L65970

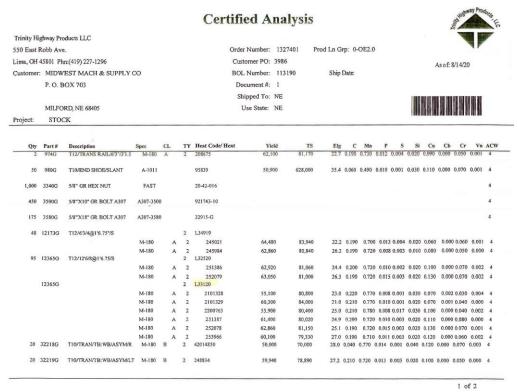


Figure B-1. 12.5-ft Thrie Beam Guardrail, Test No. FLAGT-2 (Item No. a1)

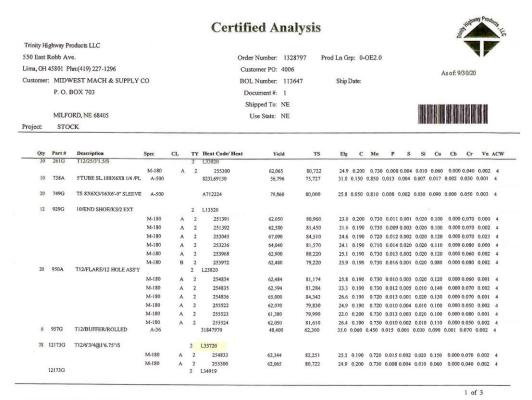


Figure B-2. 6.25-ft Thrie Beam Guardrail, Test No. FLAGT-2 (Item No. a2)



PO Box 699 - Pleasant Grove, UT 84062 Phone (801) 785-0505 www.uisutah.com

Customer PO Number: FLAGT

### **Material Certificate Of Compliance**

1/1

Date: 12/08/20 Order Number: 78327 Ship To: Customer:

**University of Nebraska Lincoln** 

University of Nebraska Lincoln Midwest Roadside Safety Facility PO Box 880439

4630 NW 36 ST

NE 68588-Lincoln Lincoln 0439

NE 68524

Project ID:

**Transitions** 

Project Description: (GR) Nebraska

Line #	QTY	Units	Description
1	4	EACH	10 / 6'-3 / 3'-1 1/2" Transition A Sym. Left (IMH # RWT-ALbB-Leading)

This is to certify that the materials shipped meet the requirements of the above Contract Specifications and Special Provisions. Guardrail meets the requirements of AASHTO M-180, Type I, II, III, or IV as stamped. Steel Posts meet the requirements of AASHTO M-270 / M-183, ASTM A992-06a: A36 and are Galvanized per ASTM A-123 OR Steel Posts meet the requirements of ASTM A588 (if required per Contract Specifications). Anchor Cable meets the requirements of ASTM 741-11, AASHTO M30. Hardware meets the requirement of AASHTO M-180, ASTM A-307 and/or A-325 or A449 per contract requirements. Galvanized per ASTM A-153. All Structural Steel conforms to AASHTO M-270 / M-183 and the Buy America Act 23 CFR 635.410. All other Galvanized Materials conform to ASTM A-123 or ASTM A-153. The materials covered by this certification conform to the requirements specified in the contract documents. The individual signing has the legal authority to bind the manufacturer or supplier of material. STATE OF UTAH, COUNTY OF UTAH

Sworn and Subscribed before me

Universal Industrial Sales, Inc.

SCOTT TURNE

ROBERT JON MCDONALD NOTARY PUBLIC - STATE OF UTAH COMMISSION#713304 COMM. EXP. 08-06-2024

**Quality Control** 

Sign Structures, Bridge Rail, Steel Fabrication, **Anchor Bolts, Highway Construction Products** 

Figure B-3. 10-ga. W-to-Thrie Transition Segment, Test No. FLAGT-2 (Item No. a3)

# GREGORY HIGHWAY PRODUCTS, INC. 4100 13th St. SW Canton, Ohio 44710

Customer:	UNIVERSITY OF 401 CANFIELD P O BOX 88043 LINCOLN,NE,68	ADMIN BLDG	LINCOLN				Test Report Ship Date: Customer P O: Shipped to: Project: GHP Order No.:	1/26/2018 36263 UNIVERSITY OF 319AA	NEBRASKA-LIN	ICOLN			
HT#code	Heat#	c.	MN.	P.	S. 0.003	Si. 0.03	Tensile 80433	Yield 59371	Elong. 16.35	Quanity 150	Class	Type 2	Description 12GA 12FT6IN/3FT1 1/2IN WB T2

Bolts comply with ASTM A-307 specifications and are galvanized in accordance with ASTM A-153, unless otherwise stated. 
Nuts comply with ASTM A-563 specifications and are galvanized in accordance with ASTM A-153, unless otherwise stated. 
All other galvanizing has occurred in the United States 
All Galvanizing has occurred in the United States 
All Stele used in the manufacture is of Domestic Origin, "Made and Melted in the United States" 
All Stele used meets Title 23CFR 55.4.10 - Buy America 
All Guardrail and Terminal Sections meets AASHTO M-180, All structural steel meets AASHTO M-183 & M270 
All Bolts and Nuts are of Domestic Origin 
All material fathorized in accordance with Nebraska Department of Transportation 
All controlled oxidized/corrosion resistant Guardrail and terminal sections meet ASTM A606, Type 4.

Jeffry & Conoren

James P Dehnke Notary Public - State of Ohio My Commission Expires October 19, 2019

STATE OF OHIO: COUNTY OF STARK Sworn to and subscribed before me, a Not Jeffery Grover this 29 day of January (201

Figure B-4. 12.5-ft W-beam Guardrail, Test No. FLAGT-2 (Item No. a4)

#### GREGORY HIGHWAY PRODUCTS, INC. 4100 13th St. SW Canton, Ohio 44710

Customer:	UNIVERSITY OF 401 CANFIELD A P O BOX 880439 LINCOLN,NE,68	D ADMIN BLDG 439					Ship Date: 7/9/2015   Customer P. O.: 45002/47091 07/07/2015     Shipped to: UNIVERSITY OF REBRASKA-LINCOLN				er P.O.: 4500274709/ 07/07/2015 to: UNIVERSITY OF NEBRASKA-LINCOLN TESTING COIL				4500274709/ 07/07/2015 UNIVERSITY OF NEBRASKA-LINCOLN TESTING COIL		
HT # code	Heat #	C.	Mn.	P.	S.	Si.	Tensile	Yield	Elong.	Quantity	Class	Type	Description				
8534	9411949	0.21	0.75	0.01	0.006	0.01	75774	56527	27.15	10	A	2	12GA 25FT WB T2 MGS ANCHOR PANEL				
8534	9411949	0.21	0.75	0.01	0.006	0.01	75774	56527	27.15	100	A	2	12GA 12FT6IN/3FT1 1/2IN WB T2				
8534	9411949	0.21	0.75	0.01	0.006	0.01	75774	56527	27.15	20	A	2	12GA 25FT0IN 3FT1 1/2IN WB T2				

7 specifications and are garvanized in a 3 specifications and are galvanized in a conforms with ASTM-123 & ASTM-653 usterial conforms with a Surviva on our move coursed in the United States sundiscurse is of Domestic Cirjin; "Made and Meled in the United States" Title 20FR 08.541 De. Buy America ferminal Sections meets AASHTO M-180, All structural steel meets AASHTO M-183 & M270

braska Department of Transportation avdrail and terminal sections meet ASTM A606, Type 4

DAWN R. BATTON NOTARY PUBLIC STATE OF OHIO OCCUMENTATIONS Warch 03, 2018 Recorded in Portage County

Figure B-5. 12.5-ft W-beam End Section, Test No. FLAGT-2 (Item No. a5)

10000000000000000000000000000000000000	(190205) CONSTRUCTION PRODUCTS T MAIN STREET N KY 42726	ROPERTY CORE EDG WEST MED CHARREON IN	TRIC TON PROPER WITTERS 91794	11
Tel: 57	0-365-4875 Pax: 270-242-928	8		
	CERTIFICATE of ANALYSIS	and TESTS	Cert No. )	2000
Part No G100 Hot Roll She	46BS			157130
10GA1270	Min X 51.0000" X 92.0000"		Pcs	Not
YIELD=50,000 TENSILE= 70,000 P= .02 MAX SI= .04 MAX C= .26 MAX S= .05 MAX DRY MATERIAL	PSI MIN 000 PSI MIN SUITABLE FOR GALVANIZING		264	47,420
Heat Number	Tag No			
833M66260	506876 YLD=<66600>/TEN=<748	100- /FX G	Pcs 28	Wgt: 5,030
833M66260	500877		28	5,030
833M66260	YLD=<66600>/TEN=<748		28	5,030
833M66260	YLD=<66600>/TEN=<748 506879		28	
833M66260	YLD=<66600>/TEN=<748			5,030
833M66260	YLD=<66600>/TEN=<748		28	5,030
833M66260	YLD=<66600>/TEN=<7480	00>/ELG=<29>	28	5,030
833M66260	YLD=<66600>/TEN=<7480	00>/ELG=<29>	28	5,030
833M66260	YLD=<66600>/TEN=<7480	00>/ELG=<29>	28	5,030
833M66260	YLD=<66600>/TEN=<7480		25	4,490
0331100280	506885 YLD=<66600>/TEN=<7480		15	2,690
Heat Number Arcelor Mittal 833M66260	*** Chominal a	s *** S=<.005> Si=<.02	9> Al=<.034> Cr .03> Ni=<.01>	1=<.019>
PROCESSED IN U	SA		00217	

Figure B-6. 10-ga Thrie Beam Terminal Connector, Test No. FLAGT-2 (Item No. a6)



Page 1 of 1

# Concrete Sample Test Report Cylinder Compressive Strength

Project Name:	Midwest Roadside Safety - Misc Testing
Project Number:	00110546.00
Client:	Midwest Roadside Safety Facility
Location:	MNPD
Sample:	011
Description:	FLAGT

### Field Data (ASTM C172, C143, C173/C231, C138, C1064)

Supplier:		Property	Test Result
Mix Name:		Slump (in):	
Ticket Number:		Air Content (%):	
Truck Number:		Unit Weight (lb/ft³):	
Load Volume (yd³):		Air Temp (°F):	
Mold Date:	12/16/2020	Mix Temp (°F):	
Molded By:		Min Temp (°F):	
Initial Cure Method:		MaxTemp (°F):	

### Laboratory Test Data (ASTM C39)

https://forneytools.forneyvault.com/Report?id=abb450a3-e139-43b0-af03-6755a5f2fe83

Sample Number:	011	011		
Set Number:	FLAGT1	FLAGT2		
Specimen Number:	1	1		
Age:	20	20		
Length (in):	8	12		
Diameter (in):	3.99	5.98		
Area (in²):	12.50	28.09		
Test Date:	01/05/2021	01/05/2021		
Break Type:	3	5		
Max Load (lbf):	72,045	142,222		
Strength (psi):	5,760	5,060		
Spec Strength (psi):				

Remarks: Average 20-	-day Compres	sive Strengt	th (psi):	5,410	)	Date received: 01/05/2021 Curing: ☑ Standard ☐ Field ASTM C511
						Submitted by:  Mall Roculer
$\times$	X X	1				Distribution:
Type 1	Type 2	Type 3	Type 4	Type 5	Type 6	Report Date: 1/5/21

This report shall not be reproduced, except in full, without prior approval of Alfred Benesch & Company. Results relate only to items tested.

825 M Street Suite 100 Lincoln, NE 68508

Alfred Benesch & Company

Figure B-7. Buttress Concrete, Test No. FLAGT-2 (Item No. b1)



1098 East Maple St Sutton, NE 68979 Phone: 402.773.4319

Email: nick@nebraskawood.com

### CERTIFICATE OF COMPLIANCE

Shipped To: Midwest Machinery and Supply

BOL# N32346

Customer PO# 3988

Preservative: CCA - C 0.60D pcf AWPA UC4B

(A)		Charge #	Retention
5.5x7.5-46" BCT	42	2538	.716
6x8-19" Block	84	2580	.632

I certify the above referenced material has been produced, treated and tested in accordance with and conforms to AASHTO M133 & M168 standards.

VA: Iowa Wood Preservers certifies that the treated wood products listed above have been treated in accordance with AWPA standards, Section 236 of the VDOT Road & Bridge Specifications and meets the applicable minimum penetration and retention requirements.

Nick Sowl, General Counsel

GENERAL NOTARY-State of Nebraska LINDA L. SCHROETLIN My Comm. Exp. May 20, 2024

Figure B-8. BCT Timber Posts, Test No. FLAGT-2 (Item No. c1)

# GREGORY HIGHWAY PRODUCTS, INC. 4100 13th St. SW

	MIDWEST MA P. O. BOX 703		SUPPLY CO	i.			Test Report Ship Date: Customer P.O.:	10/26/2017 3501					
							Shipped to:	MIDWEST MA	CHINERY & SU	JPPLY CO.			
	MILFORD, NE,	68405					PROJECT:	STOCK					
							GHP Order No:	7044AA					
HT CODE	Lot#	C.	Mn.	P.	s.	Si.	Tensile	Yield	Elong.	Quantity	Class	Туре	Description
616137		0.21	0.93	0.011	0.003	0.02	73148	58210	32	15		2	3/16 X 6IN X 8IN X 5FT0IN TUBE SLEEVE
821T08220		0.22	0.81	0.013	0.006	0.006	70934	57275	32	10		2	3/16IN X 6IN X 8IN X 6FT0IN TUBE SLEEVE
214482		0.04	0.83	0.014	0.005	0.02	75275	68023	28.6	25	В		10GA MGS TB TRAN APPROACH END-RIGHT
214143		0.04	0.81	0.015	0.006	0.02	75565	69618	29.7	18	В		10GA MGS TB TRAN DEPARTURE END-LEFT

Bolls comply with ASTM A-307 specifications and are galvanized in accordance with ASTM A-153, unless otherwise stated. Nuts comply with ASTM A-563 specifications and are galvanized in accordance with ASTM A-153, unless otherwise stated. All other galvanized material conforms with ASTM-128 ASTM-553
All Galvanizing has occurred in the United States
All State lused in the manufacture is of Domestic Origin, "Made and Melted in the United States"
All Steel used meets Title 23CPF 85.5.410 - Buy America
All Galvardrail and Terminal Sections meets AASHTO M-180, All structural steel meets AASHTO M-183 & M270
All Botts and Nuts are of Domestic Origin
All material fabricated in accordance with Nebraska Department of Transportation
All sheet, zinc-coated or zinc-iron alloy-coated by the hot dip process that meets ASTM Specifications A653

Jeffry & Gracier

Figure B-9. BCT Foundation Tube, Test No. FLAGT-2 (Item No. c2)

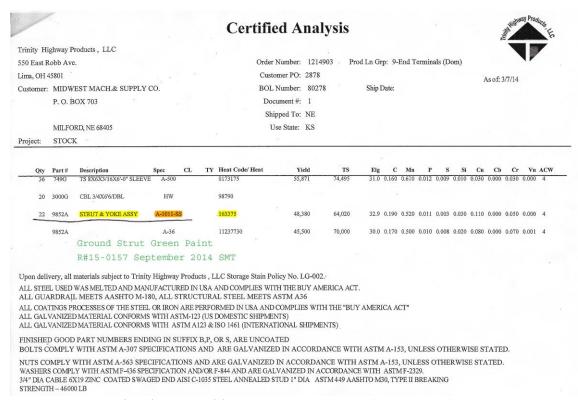


Figure B-10. Strut and Yoke Assembly, Test No. FLAGT-2 (Item No. c3)



PH 216.676.5600 FX 216.676.6761 www.assemblyspecialty.com

ISO 9001:2008

14700 Brookpark Rd Cleveland, OH 44135-5166 customerservice@assemblyspecialty.com

## **Certificate of Conformance**

Date: September 24, 2018

To: Gregory Industries, Inc.

Gregory Galv. & Metal Processing

4100 13th St. SW Canton, OH 44710

We certify that our system and procedures for the control of quality assures that all items furnished on the order will meet applicable tests, requirements and inspection requirements as required by the purchase order and applicable specifications and drawings.

PURCHASE ORDER #: 40299

DATE SHIPPED: 09/24/18

ASPI SALES ORDER #: 122160

MANUFACTURER: ASSEMBLY SPECIALTY PRODUCTS, INC.

QTY	CUST P/N	ASPI P/N	ASPI LOT#	DESCRIPTION
250	3012G	C-2028	89315	6' 6" BCT Cable Assembly
250	3012G	C-2028	89316	6' 6" BCT Cable Assembly
250	3012G	C-2028	89318	6' 6" BCT Cable Assembly
250	3012G	C-2028	89864	6' 6" BCT Cable Assembly
250	3012G	C-2028	89865	6' 6" BCT Cable Assembly
250	3012G	C-2028	89866	6' 6" BCT Cable Assembly
250	3012G	C-2028	89929	6' 6" BCT Cable Assembly
250	3012G	C-2028	89930	6' 6" BCT Cable Assembly
250	3012G	C-2028	89931	6' 6" BCT Cable Assembly
250	3012G	C-2028	89932	6' 6" BCT Cable Assembly

REMARKS: NOMINAL BREAKING STRENGTH: 46,000 lbs

WIRE ROPE MANUFACTURED IN ACCORDANCE WITH AASHTO DESIGNATION: M30-02 and ASTM A741 TYPE 2, CLASS A FITTINGS GALVANIZED IN ACCORDANCE WITH ASTM A-153 CLASS C.

STEEL USED TO MANUFACTURE THESE ITEMS WAS MELTED AND MANUFACTURED IN THE U.S.A.
ALL MANUFACTURING PROCESSES SUPPLIED OR PERFORMED BY ASSEMBLY SPECIALTY PRODUCTS, INC. TOOK PLACE IN THE U.S.A.

Signature:

Certification and Compliance Manager

Figure B-11. BCT Anchor Cable Assembly, Test No. FLAGT-2 (Item No. c4)

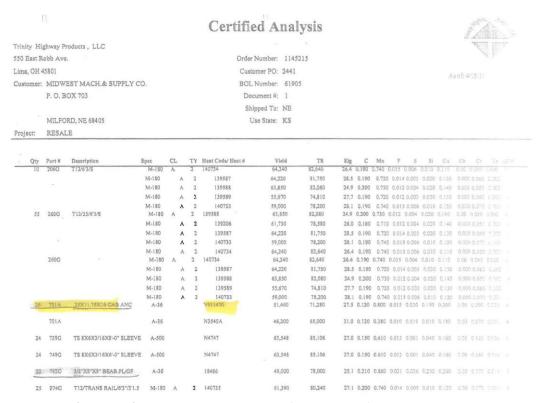


Figure B-12. Anchor Bracket, Test No. FLAGT-2 (Item No. c5)

#### Test Report Ship Date: Customer P.O.: MIDWEST MACHINERY & SUPPLY CO. 11/17/2017 3515 MIDWEST MACHINERY & SUPPLY CO. Shipped to: MILEORD NE 68405 HT # code 25.2 34 27 25 A74070 0.21 0.24 0.46 0.84 0.012 0.002 0.03 76100 58800 12GA TB TRANS 0.014 0.01 4181496 0,01 72400 5/8IN X 8IN X 8IN BRG. PL. 4181489 0.09 0.45 0.012 0.004 0.01 58000 43100 350 STRUT & YOKE 0.84 0.014 76000 350 STRUT & YOKE E22985 0.17 0.51 0.013 0.008 0.008 72510 64310 29.5 2IN X 5 1/2IN PIPE SLEEVE 811T08220 3/16IN X 6IN X 8IN X 6FTOIN TUBE SLEEVE

GREGORY HIGHWAY PRODUCTS, INC. 4100 13th St. SW Canton, Ohio 44710

All Galvanizing has occurred in the United States
All steel used in the manufacture is of Domestic Origin, "Made and Melted in the United States"
All Steel used meets Title 23CFR 635.410 - Buy America
All Guardrail and Terminal Sections meets AASHTO M-180, All structural steel meets AASHTO M-183 & M270
All Bolts and Nuts are of Domestic Origin
All material fabricated in accordance with Nebraska Department of Transportation
All controlled oxidized/corrosion resistant Guardrail and terminal sections meet ASTM A606, Type 4.

STATE OF OHIO: COUNTY OF STARK Sworn to and subscribed before me, a Notary Po Andrew Aftar this 21 day of November, 2017

James P. Dehnke

Notary Public, State of Ohio De Commission Expires 10-19-2019

Figure B-13. Anchor Bearing Plate, Test No. FLAGT-2 (Item No. c6)

Atlas Tube (Alabama), Inc. 171 Cleage Dr Birmiagham; Alabama, USA 35217 Tel: Fax:



Ref.B/L: 80791452 Date: 11.10.2017 Customer: 179

### MATERIAL TEST REPORT

Sold to

Steel & Pipe Supply Compan PO Box 1688 MANHATTAN KS 66505 USA

Shipped to

Steel & Pipe Supply Compan 401 New Century Parkway NEW CENTURY KS 66031 USA

•		3x40'0"0(5x	4).				o: 0300						in: USA		
Sales order:	122697	6			Pu	ırchase (	Order: 4	5002966	556	Cust Ma	terial #:	663002	0018840	)	
Heat No	С	Mn	P	S	Si	AI	Cu	СЬ	Мо	Ni	Cr	V	Ti	В	N
B704212	0.200	0.450	0.010	0.004	0.020	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Bundle No	PCs	Yield	Ten		Etn.:				Се	rtification			С	E: 0.2	8
40867002	20	064649 Ps	0.000000	652 Psi	24 %			Ā	STM A5	00-13 GR	ADE B&	C			
Material Note Sales Or.Note															
Material: 2.37	75x154x	42'0"0(34x1	1).		М	aterial N	o: R023	751544	200			Made in	n: USA		
Sales order:	122697	6			Pu	ırchase (	Order: 4	5002966	556	Cust Ma	terial #:				
Heat No	С	Mn	P	s	Si	AI	Cu	СР	Мо	Ni	Cr	v	Ti	В	N
B712810	0.210	0.460	0.012	0.002	0.020	0.024	0.100	0.002	0.020	0.030	0.060	0.004	0.002	0.000	0.008
Bundle No	PCs	Yield	Ten	sile	Eln.:	2in	RЬ		Се	rtification			С	E: 0.3	2
MC00006947		063688 Ps		220 Psi	25 %	 01			-	00-13 GR	ADE DO				
Material Note: Sales Or.Note	:	003080 FS	000	220 F31	20 70	31			STW AS	00-13 GR	AUE BQ				
Material: 2.37	75x154x		).				o: RO23						in: USA		
Sales order:	122697	6			Pu		Order: 4	5002966	556	Cust Ma	terial #:	642004	042		
Heat No	С	Mn	Р	S	Si	AI	Cu	Cb	Мо	Ni	Cr	V	Ti	В	N
17037261	0.210	0.810	0.005	0.004	0.020	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		Yield	Ten	sile	Eln.	2in			Ce	rtification			c	E: 0.3	5
Bundle No	PCs	Tiela													

Authorized by Quality Assurance:

The results reported on this report represent the actual attributes of the material furnished and indicate full compliance with all applicable specification and contract requirements.

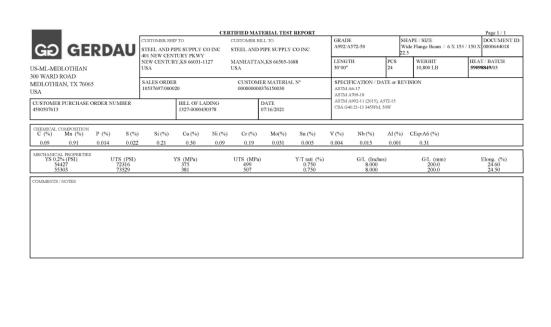
D1.1 method.

Page: 2 04

Figure B-14. BCT Post Sleeve, Test No. FLAGT-2 (Item No. c7)

				CEI	RTIFIED MAT	TERIAL TE	ST REPOR	T .						Page 1/1
		CUSTOMER SHIP			CUSTOMER B				GRADE A992/A70	0.26		HAPE / SE		DOCUMENT
GO GERD	ΑU	HIGHWAY SAI 473 W FAIRGR			HIGHWAY S.	AFETY COI	RP		ASSEATO	9-30	13		Beam / 6 X 8.3	5# / 150 × 0000320239
- Anna Anna Anna Anna Anna Anna Anna Ann		MARION, OH 4:			GLASTONBU	RY,CT 060	33-0358		LENGTH		PCS	WEIG		HEAT/BATCH
US-ML-CARTERSVILLE		USA			USA			i	42'00"		105	37,48	5 LB	55066501/03
384 OLD GRASSDALE ROAD NE CARTERSVILLE, GA 30121		SALES ORDER	ξ		CUSTOM	ER MATER	IAL Nº		SPECIFIC	ATION / DA'	TE or REV	ISION		
USA		8525742/00006	0						ASTM A6-1				IB-1	30600800
CUSTOMER PURCHASE ORDER N	IMBER		BILL OF LADIN	iG		DATE		-	ASTM A99					30600800 32176
1832 .			1323-00001569			05/08/2020		i	CSA G40.2	I-13 345WM			183	32176
													,	
CHEMICAL COMPOSITION	D	e e	e;	01	Ni		C-	М.		Sn.	v		NTs.	
C Mn 0.11 0.82	P 0.015	0.027	§i 0.21	Сµ 0.29	Ni 0.1	7	% 0.18	Ma 0.04	48	Sn 0.008	0.002		№ 0.009	
MECHANICAL PROPERTIES				1122									01003	
YS.0.2%	Ϋ́	ş	MPa			MPa			Y/T rati			Elong.		
62200 61000	770 784	00	429 421			531 541			0.810			23.40 24.40		
	704	-	761			341			0.780			24.40		
COMMENTS / NOTES														
and the second s														
The above figs	res are certi	fied chemical and	physical test reco	rds as c	ontained in the	permanent re	ecords of con	npany. W	e certify tha	t these data a	re correct a	and in com	pliance with	
specified requi	irements. We	eld repair has not	been performed or	this m	aterial. This ma	terial, includ	ling the billet	s, was me	elted and ma	mufactured in	the USA.	CMTR co	mplies with EN	
/sh.	acka	THASE OFFICE	AR YALAMANCHILL TY DIRECTOR						: 1			N WANG	RANCE MOR.	
	-	- QUALL							70	10) 207 cm10	,			
Phone: (409	) 40;-10/1 HB	man: Dueskill, Yalkimi	anchili@gerdan.com						raone: (7	70) 387 5718	eman: yar.	w angl@gerda	ni.com	

Figure B-15. 6-ft W6x8.5 Posts, Test No. FLAGT-2 (Item No. d1)



The above figures are certified chemical and physical test records as contained in the permanent re specified requirements. No weld repair was performed on this material. The material has not been billets, was produced (Elseric Are Furnace melded, Continuously est, and/or Hot rolled) in the U.	in contact with mercury while in Gerdau possession. This material, including the
Mackay BHASKAR YALAMANCHILI QUALITY DIRECTOR	Wale A. LL WADELUMPKINS QUALITY ASSURANCE MOR
Phone: (409) 267-1071 Email: Bhaskar. Yalamanchili@gerdau.com	Phone: 972-779-3118 Email: Wade.Lumpkins@gerdau.com

Figure B-16. 7.5-ft W6x15 Posts, Test No. FLAGT-2 (Item No. d2)



1098 East Maple St Sutton, NE 68979 Phone: 402.773.4319 Email: nick@nebraskawood.com

### CERTIFICATE OF COMPLIANCE

Shipped To: Midwest Machinery and Supply

BOL# <u>N26219</u> Customer PO# <u>3930</u>

Preservative: CCA - C 0.60D pcf AWPA UC4B

Part #	Physical Description	# Pieces	Charge #	Retention
6117b	6x8-6.5' CRT	35	1413	.626
GR6814 BLK	6x8-14" OCD Block	126	1696	.621
GR61214 BLK	6x12-14" OCD Block	168	1695	.625
GR61222 BLK	6x12-22" OCD Block	168	1695	.625
				* *

I certify the above referenced material has been produced, treated and tested in accordance with and conforms to AASHTO M133 & M168 standards.

VA: Iowa Wood Preservers certifies that the treated wood products listed above have been treated in accordance with AWPA standards, Section 236 of the VDOT Road & Bridge Specifications and meets the applicable minimum penetration and retention requirements.

Nick Sowl, General Counsel

Date

Figure B-17. 6-in. x 12-in. x 14-in. Timber Blockout, Test No. FLAGT-2 (Item No. d3)



1098 East Maple St Sutton, NE 68979 Phone: 402.773.4319 Email: nick@nebraskawood.com

### CERTIFICATE OF COMPLIANCE

Shipped To: Midwest Machinery and Supply

BOL# N34266

Customer PO# 4008

Preservative: CCA - C 0.60D pcf AWPA UC4B

Part #	Physical Description	# Pieces	Charge #	Retention
GR6819 BLK	6x8-19" OCD Block	168	2580	.632
GR61219 BLK	6x12-19" OCD Block	504	2580	.632

I certify the above referenced material has been produced, treated and tested in accordance with and conforms to AASHTO M133 & M168 standards.

VA: Iowa Wood Preservers certifies that the treated wood products listed above have been treated in accordance with AWPA standards, Section 236 of the VDOT Road & Bridge Specifications and meets the applicable minimum penetration and retention requirements.

Nick Sowl, General Counsel

9/24/20 Date

GENERAL NOTARY-State of Nebraska LINDA L. SCHROETLIN My Gomm. Exp. May 20, 2024

Figure B-18. 6-in. x 12-in. x 19-in. Timber Blockout, Test No. FLAGT-2 (Item No. d4)



1098 East Maple St Sutton, NE 68979 Phone: 402.773.4319 Email: nick@nebraskawood.com

# CERTIFICATE OF COMPLIANCE

Shipped To: Midwest Machinery and Supply

BOL# N42634

Customer PO# 5011

Preservative: CCA - C 0.50D pcf AWPA UC4

Part #	Physical Description	# Pieces	Charge #	Retention
GR6819 BLK	6x8-19" Thrie OCD Block	252	4369	.623

I certify the above referenced material has been produced, treated and tested in accordance with and conforms to AASHTO M133 & M168 standards.

VA: Iowa Wood Preservers certifies that the treated wood products listed above have been treated in accordance with AWPA standards, Section 236 of the VDOT Road & Bridge Specifications and meets the applicable minimum penetration and retention requirements.

Nick Sowl, General Counsel

GENERAL NOTARY-State of Nebraska

LINDA L. SCHROETLIN My Comm. Exp. May 20, 2024



# Certificate of Compliance

600 N County Line Rd Elmhurst IL 60126-2081 630-600-3600 chi.sales@mcmaster.com University of Nebraska
Midwest Roadside Safety Facility
M W R S F
4630 Nw 36TH St
Lincoln NE 68524-1802
Attention: Shaun M Tighe
Midwest Roadside Safety Facility

Purchase Order E000548963 Order Placed By Shaun M Tighe McMaster-Carr Number 7204107-01 Page 1 of 1 08/02/2018

Line Product Ordered Shipped

1 97812A109 Raised-Head Removable Nails, 16D Penny Size, 3" Long, Packs of 5
Packs

Certificate of compliance

This is to certify that the above items were supplied in accordance with the description and as illustrated in the catalog. Your order is subject only to our terms and conditions, available at www.mcmaster.com or from our Sales Department.

Sarah Weinberg Compliance Manager

Figure B-20. 16D Nail, Test No. FLAGT-2 (Item No. d6)

# **NUCOR®**

### Mill Certification

09/02/2020

MTR#:458890-2 Lot #:360001474020 ONE NUCOR WAY BOURBONNAIS, IL 60914 US 815 937-3131 Fax: 815 939-5599

Sold To: SIMCOTE INC 1645 RED ROCK RD ST PAUL, MN 55119 US Ship To: SIMCOTE INC 1645 RED ROCK RD ST PAUL, MN 55119 US

Customer PO	MN-3748	Sales Order #	36013225 - 1.31
Product Group	Rebar	Product #	2110206
Grade	A615 Gr 60/AASHTO M31	Lot #	360001474020
Size	#4	Heat #	3600014740
BOL#	BOL-567414	Load #	458890
Description	Rebar #4/13mm A615 Gr 60/AASHTO M31 60' 0" [720"] 6001- 10000 lbs	Customer Part #	
Production Date	08/12/2020	Qty Shipped LBS	22725
Product Country Of Origin	United States	Qty Shipped EA	567
Original Item Description		Original Item Number	

I hereby certify that the material described herein has been manufactured in accordance with the specifications and standards listed above and that it satisfies those requirements Melting Date: 08/07/2020 Melt Country of Origin: United States Nb (%) C (%) Mn (%) P (%) S (%) Si (%) Ni (%) Cr (%) Mo (%) Cu (%) 0.043 0.06 0.012 0.002 0.34 0.90 0.015 0.198 0.18 0.23 0.40

 Other Test Results
 Yield (PSI): 66100
 Tensile (PSI): 99200
 Average Deformation Height (IN): 0.036

 Elongation in 8" (%): 14.5
 Bend Test: Pass
 Weight Percent Variance (%): -4.00

#### Comments:

All manufacturing processes of the steel materials in this product, including melting, have occurred within the United States. Products produced are weld free. Mercury, in any form, has not been used in the production or testing of this material.

Figure B-21. #4 Rebar, Test No. FLAGT-2 (Item Nos. e1 through e10)

#### CERTIFICATE OF COMPLIANCE

ROCKFORD BOLT & STEEL CO. 126 MILL STREET ROCKFORD, IL 61101 815-968-0514 FAX# 815-968-3111

CUSTOMER NAME:

TRINITY INDUSTRIES

**CUSTOMER PO:** 

200208

SHIPPER #: 067063 DATE SHIPPED: 08/21/2019

LOT#:

32086-B

SPECIFICATION:

ASTM A307, GRADE A MILD CARBON STEEL BOLTS

TENSILE:

SPEC:

60,000 psi\*min

RESULTS:

74,700

74,000 89.20

HARDNESS: 100 max

88.60

\*Pounds Per Square Inch.

COATING:

ASTM SPECIFICATION F-2329 HOT DIP GALVANIZE

AZZ GALVANIZING:

32086-B

#### CHEMICAL COMPOSITION

GRADE	HEAT#	· C	Mn	P	S	Si
1010	100897520	.11	.51	.012	.007	.18
	CIVIDE	ON DE HEAT	SINDE HEATT	ONADE HEATT O WIII	ONDE HEATE O WILL P	CIVIDE HEATE O WILL P S

10,650

PCS 5/8" X 14" GUARD RAIL BOLT P/N 3540G

WE HEREBY CERTIFY THE ABOVE BOLTS HAVE BEEN MANUFACTURED BY ROCKFORD BOLT AND STEEL AT OUR FACILITY IN ROCKFORD, ILLINOIS, USA. THE MATERIAL USED WAS MELTED AND MANUFACTURED IN THE USA. WE FURTHER CERIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIALS SUPPLIER, AND THAT OUR PROCEDURES FOR THE CONTROL OF PRODUCT QUALITY ASSURE THAT ALL ITEMS FURNISHED ON THIS ORDER MEET OR EXCEED ALL APPLICABLE TESTS, PROCESS, AND INSPECTION REQUIREMENT PER ABOVE SPECIFICATION.

STATE OF ILLINOIS

COUNTY OF WINNEBAGO

20 DAY OF August 20,

APPROVED SIGNATORY

8/20/1

Official Seal Merry F Shane Notary Public State of Illinois My Commission Expires 10/03/2022

Figure B-22. %-in. x 14-in. Guardrail Bolt, Test No. FLAGT-2 (Item No. f1)

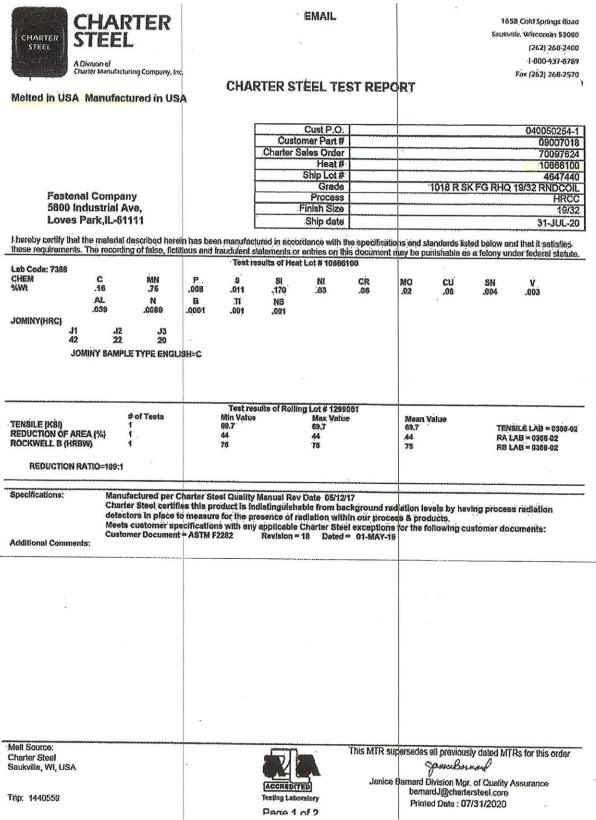


Figure B-23. %-in. x 10-in. Guardrail Bolt, Test No. FLAGT-2 (Item No. f2)

### CERTIFICATE OF COMPLIANCE

**ROCKFORD BOLT & STEEL CO.** 126 MILL STREET ROCKFORD, IL 61101 815-968-0514 FAX# 815-968-3111

**CUSTOMER NAME:** 

TRINITY INDUSTRIES

CUSTOMER PO:

182402

SHIPPER #: 059943

DATE SHIPPED: 03/07/2017

LOT#:

29221

SPECIFICATION:

ASTM A307, GRADE A MILD CARBON STEEL BOLTS

TENSILE:

60,000 psi\*min

RESULTS:

HARDNESS:

100 max

66,327 71.30

71.60

\*Pounds Per Square Inch.

COATING: ASTM SPECIFICATION F-2329 HOT DIP GALVANIZE

ROGERS GALVANIZE: 29221

#### CHEMICAL COMPOSITION

MILL	GRADE	HEAT#	С	Mn	Р	S	Si
	4					• •	
CHARTER	1010	10439100	.09	.40	.008	.011	.090

### QUANTITY AND DESCRIPTION:

10,400

PCS 5/8" X 2" GUARD RAIL BOLT P/N 3400G

WE HEREBY CERTIFY THE ABOVE BOLTS HAVE BEEN MANUFACTURED BY ROCKFORD BOLT AND STEEL AT OUR FACILITY IN ROCKFORD, ILLINOIS, USA. THE MATERIAL USED WAS MELTED AND MANUFACTURED IN THE USA. WE FURTHER CERIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL'S SUPPLIER, AND THAT OUR PROCEDURES FOR THE CONTROL OF PRODUCT QUALITY ASSURE THAT ALL ITEMS FURNISHED ON THIS ORDER MEET OR EXCEED ALL APPLICABLE TESTS, PROCESS, AND INSPECTION REQUIREMENT PER ABOVE SPECIFICATION.

STATE OF ILLINOIS COUNTY OF WINNEBAGO SIGNED BEFORE ME ON THIS

PROVED SIGNATORY

OFFICIAL SEAL MERRY F. SHANE

NOTARY PUBLIC - STATE OF ILLINOIS MY COMMISSION EXPIRES OCTOBER 3. 2018

Figure B-24. %-in. x 2-in. Guardrail Bolt, Test No. FLAGT-2 (Item No. f3)

### CERTIFICATE OF COMPLIANCE

#### ROCKFORD BOLT & STEEL CO. **126 MILL STREET** ROCKFORD, IL 61101 815-968-0514 FAX# 815-968-3111

CUSTOMER NAME:

TRINITY INDUSTRIES

CUSTOMER PO:

203160

SHIPPER #: 068184 DATE SHIPPED: 02/06/2020

LOT#:

32539-P

SPECIFICATION:

ASTM A307, GRADE A MILD CARBON STEEL BOLTS

TENSILE:

SPEC:

60,000 pai\*min

RESULTS:

69,000

HARDNESS:

100 max

69,300 68.80

68,30

"Pounds Per Square Inch.

COATING: ASTM SPECIFICATION F-2329 HOT DIP GALVANIZE

AZZ GALVANIZING:

32539-P

#### CHEMICAL COMPOSITION

MILL	GRADE	HEAT#	С	Mn	Р	S	Si
CHARTER STEEL	1010	10634210	.10	.52	.008	.009	.08

### QUANTITY AND DESCRIPTION:

12,000

PCS 5/8" X 1.25" GUARD RAIL BOLT

P/N 3360G

WE HEREBY CERTIFY THE ABOVE BOLTS HAVE BEEN MANUFACTURED BY ROCKFORD BOLT AND STEEL AT OUR FACILITY IN ROCKFORD, ILLINOIS, USA, THE MATERIAL USED WAS MELTED AND MANUFACTURED IN THE USA. WE FURTHER CERIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIALS SUPPLIER, AND THAT OUR PROCEDURES FOR THE CONTROL OF PRODUCT QUALITY ASSURE THAT ALL ITEMS FURNISHED ON THIS ORDER MEET OR EXCEED ALL APPLICABLE TESTS, PROCESS, AND INSPECTION REQUIREMENT PER ABOVE SPECIFICATION.

STATE OF ILLINOIS

COUNTY OF WINNEBAGO SIGNED BEFORE ME ON THIS

Official Seal Merry F Shane Notary Public State of Illinois My Commission Expires 10/03/2022

Figure B-25. %-in. x 11/4-in. Guardrail Bolt, Test No. FLAGT-2 (Item No. f4)



Phone: 800-547-6758 | Fax: 503-227-4634 3441 NW Guam Street, Portland, OR 97210 Web: www.portlandbolt.com | Email: sales@portlandbolt.com +-----+ CERTIFICATE OF CONFORMANCE

For: MIDWEST ROADSIDE SAFETY FACIL

**PB Invoice#:** 136724

FL AGT ITEM#F5/H 11/13/2020 Cust PO#:

Date: 11/16/2020 Shipped:

We certify that the following items were manufactured and tested in accordance with the chemical, mechanical, dimensional and thread fit requirements of the specifications referenced.

Description: 7/8 X 15-1/2 GALV ASTM F3125 GRADE A325 HEAVY HEX BOLT

Не	<b>at#:</b> 3093	334	į	Base S	teel:	4140	Diam:	7/8		
•	ce: COMM			s co		Proof Load	<b>i:</b> 39,	,250 L	BF	
С:	.400	Mn:	.810	P :	.016	Hardness:	293	HBN		
s :	.019	Si:	.240	Ni:	.190	Tensile:	67,180	LBF	RA:	.00%
Cr:	.870	Mo:	.208	Cu:	.320	Yield:	0		Elon:	.00%
Pb:	.000	v :	.024	Cb:	.000	Sample Le	ngth:	0		
N :	.000			CE:	.6329	Charpy:			CVN Temp:	
LOT#	19878									

TOI#138/8

#### Nuts:

ASTM A563DH HVY HX

#### Coatings:

ITEMS HOT DIP GALVANIZED PER ASTM F2329/A153C

Certification Department Quality Assurance Dane McKinnon

Figure B-26. 7/8-in. x 151/2-in. Heavy Hex Bolt, Test No. FLAGT-2 (Item No. f5)

Apr. 12. 2018 4:32PM . Fastenal-NELI

No. 4682 P. 3

## Certificate of Compliance

Sold To:	Purchase Order	
UNL TRANSPORTATION	Job:	TL-2 and Bullnose
	Invoice Date:	03/27/2018
THIS IS TO CERTIFY THAT WE THESE PARTS WERE PUR	HAVE SUPPLIED YOU WITH THE CHASED TO THE FOLLOWING SE	FOLLOWING PARTS. PECIFICATIONS.
5 PCS 7/8"-9 x 8" ASTM A307 Grade A Hot Dipped G UNDER PART NUMBER 92005	alvanized Hex Bolt SUPPLIED UNDE	R OUR TRACE NUMBER line35042 AND
20 PCS 7/8"-9 Hot Dip Galvanized Finish Grade A Fini UNDER PART NUMBER 36717	shed Hex Nut SUPPLIED UNDER O	JR TRACE NUMBER 110254885 AND
5 PCS 7/8"-9 x 8" ASTM A307 Grade A Hot Dipped G UNDER PART NUMBER 92005	alvanized Hex Bolt SUPPLIED UNDE	R OUR TRACE NUMBER line35042 AND
5 PCS 7/8"-9 x 8" ASTM A307 Grade A Hot Dipped G UNDER PART NUMBER 92005	alvanized Hex Bolt SUPPLIED UNDE	R OUR TRACE NUMBER line35042 AND
5 PCS 7/8"-9 x 8" ASTM A307 Grade A Hot Dipped Ga UNDER PART NUMBER 92005	alvanized Hex Bolt SUPPLIED UNDE	R OUR TRACE NUMBER IIne35042 AND
		i
This is to certify that the above document is true and accurate to the best of my knowledge.	Please check current	revision to avoid using obsolete copies.
Spir MM	This document was prime.	rinted on 04/12/2018 and was current at that
Fastenal Account Representative Signature	Fastenal Store Local	tion/Address
Printed Name	3201 N. 23rd Street S LINCOLN, NE 68521 Phone #: (402)476-79	,
4/12/18	Fax #: 402/476-7958	
Date	Page 1 of 1	

Figure B-27. %-in. x 8-in. Hex Bolt, Test No. FLAGT-2 (Item No. f6)

### Certificate of Compliance

Birmingham Fastener Manufacturing PO Box 10323 Birmingham, AL 35202 (205) 595-3512

Customer _	Midwest M	achinery 8	Supply	ι	Date Shipp	ed	11/28	/2018
Customer Orde	er Number _	36	64	i	BFM Order	Number	1553	3751
			Item	Descrip	tion			
Description_		5	/8"-11 x 10"	Hex Bolt			Qty	298
Lot#	81342	Spe	ecification_	ASTM A30	7-14 Gr A	Finish	ASTM	F2329
			Raw Ma	terial A	nalysis			
Heat#	JK	(18104124						
Chemical Co C 0.18	mposition (w Mn 1.19	/t% Heat A P 0.012	nalysis) By S 0.034	Material S Si 0.20	upplier Cu 0.29	Ni 0.13	Cr 0.11	Mo 0.04
			Mechan	ical Pro	perties			
Sample # 1 2 3 4 5	Hardness 93 HRBW		Tensile Str 22,0			Tensile Str 99,4		)
This informaticustomer order All steel melte	er. The samp	les tested o	conform to the				stated	
Authorized Signature: _		Sul ian Hughe			Date:	11/29	/2018	

Figure B-28. 5%-in. x 10-in. Hex Bolt, Test No. FLAGT-2 (Item No. f7)

# CERTIFIED MATERIAL TEST REPORT TO DIN EN 10204-2005 3.1 FOR ASTM A307, GRADE A - MACHINE BOLTS

FACTORY: IFI & MORGAN LTD. REPORT DATE:2019/4/9

ADDRESS: No.583-28, Chang'an North Road, Wuyuan Town, Haiyan,

Zhejiang, China MANUFACTURE DATE:2019/3/28

CUSTOMER: FASTENAL MFG LOT NUMBER:M-2019HT200-9

SAMPE SIZE: ACC. TO ASME B18.18 CATEGORY 2-2011; ASTM F1470-12 TABLE 3

MANU QTY: 28130PCS SHIPPED QTY:28080PCS

SIZE: 5/8-11X1 1/2 HDG

HEADMARKS: 307A PLUS NY
PO NUMBER:180170611
PART NO:91919

STEEL PROPERTIES:

MATERIAL TYPE:Q195C HEAT NUMBER:5-01571

CHEMISTRY SPEC: Grade A ASTM A307-12 TEST:

	S %*1000
0.29max 1.20 max 0.04max	0.15max
0.08 0.33 0.016	0.024

DIMENSIONAL INSPECT	TIONS Unit:inch		SPECIFICATION: ASM	ME B18.2.1	- 2012
CHARACTERISTICS	SPECIFIE	D	ACTUAL RESULT	ACC.	REJ.
*******	******	******	******	*****	*****
VISUAL	ASTM F788-20	013	PASSED	29	0
THREAD	ASME B1.1-20	003, 3A GO, 2A NO GO	PASSED	15	0
WIDTH A/F	0.906-0	.938	0.915-0.929	4	0
WIDTH A/C	1.033-1	1.048-1.057	4	0	
HEAD HEIGHT	0.378-0	0.395-0.411	4	0	
THREAD LENGTH	1.420-1	1.434-1.486	15	0	
LENGTH	1.420-1	.560	1.434-1.486	15	0
MECHANICAL PROPER	TIES:	SPECIFICAT	ΓΙΟΝ: ASTM A307 - 14e	GR.A	
CHARACTERISTICS	TEST METHOD	SPECIFIED	ACTUAL RESULT	ACC.	REJ.
******	******	******	******	*****	*****
CORE HARDNESS:	ASTM F606/F606M-2016	69-100 HRB	76-80 HRB	4	0
WEDGE TENSILE:	ASTM F606/F606M-2016	Min 60 KSI	65-69 KSI	4	0
CHARACTERISTICS	TEST METHOD	SPECIFIED	ACTUAL RESULT	ACC.	REJ.
COATINGS OF ZINC:		SPECIFIATION: ASTM	F2329/F2329M-15		
HOT DIP GALVANIZED	ASTM B568-98(2014)	Min 0.0020"	0.0021" -0.0022"	4	0

We hereby certify that above products supplied are in compliance with all the requirements of the order.

We here by certify that this MTR is in compliance to DIN EN 10204 3.1 content.

ALL TESTS IN ACCORDANCE WITH THE METHODS PRESCRIBED IN THE APPLICABLE ASTM SPECIFICATION. WE CERTIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL SUPPLIER AND OUR RESTRICTION OF LABORATORY.

Maker's ISO 9001:2015 SGS Certificate # HK04/0105

(SIGNATURE ON C.A. LAB MCR.) (NAME OF MANUFACTURER)

Figure B-29. %-in. x 1½-in. Hex Bolt, Test No. FLAGT-2 (Item No. f8)

33187

### CERTIFIED MATERIAL TEST REPORT FOR USS FLAT WASHERS HDG

IFI & Morgan Ltd FACTORY: REPORT DATE: 23/4/2019

Chang'an North Road, Wuyuan Town, Haiyan, Zhejiang, China ADDRESS:

MFG LOT NUMBER:

SAMPLING PLAN PER ASME B18.18-11 PO NUMBER: 170089822

USS 7/8 HDG QNTY(Lot size):

7200PCS HEADMARKS: NO MARK PART NO:

DIMENSIONAL INSPECTIONS SPECIFICATION: ASTM B18.21.1-2011 **CHARACTERISTICS SPECIFIED** ACTUAL RESULT ACC. REJ. \*\*\*\*\*\*\*\*\* \*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\*\* **APPEARANCE** 100 0 ASTM F844 PASSED **OUTSIDE DIA** 2.243-2.280 2.246-2.254 10 0 **INSIDE DIA** 0.931-0.968 0.956-0.965 10 0 **THICKNESS** 0.136-0.192 0.136-0.157 10 0 CHARACTERISTICS TEST METHOD **SPECIFIED** ACTUAL RESULT REJ. ACC. \*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\*\*\*\*\* \*\*\*\*\*\* \*\*\*\*\*\* HOT DIP GALVANIZED ASTM F2329-13 Min 0.0017"

ALL TESTS IN ACCORDANCE WITH THE METHODS PRESCRIBED IN THE APPLICABLE ASTM SPECIFICATION. WE CERTIFY THAT THIS DAIA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL SUPPLI TESTING LABORATORY.

ISO 9001:2015 SGS Certificate # HK04/0105

Figure B-30. %-in. Round Washer, Test No. FLAGT-2 (Item No. g1)

### SSF INDUSTRIAL CO., LIMITED

### **MILL TEST CERTIFICATION**

Certification Conforms to EN1024 3.1B

**Supplier:** SSF INDUSTRIAL CO.,LIMITED **Certificate No.:** 000825

Buyer: FASTENAL COMPANY PURCHASING Invoice No.: FASTCO2020051501

Product Description: 5/8 USS F/W GALV

 Product Size:
 5/8
 Shipped Q'ty:
 12 MPCS

 Quality Acceptance:
 ISO 3269
 Lot No.:
 20200515

RAW MATERIAL scrap

Element C Si Mn S P Ni Cr Cu

#### SURFACE

Test Item	Spec.	Standard	Remark
Appearance	Flawless	1	ок

#### DIMENSION MEASUREMENT(L According to: USS

Test Item	Standard (mm)		Campling	Remark	Test Result	
rest item	Min	Max	Sampling	Remark	rest Result	
INNER DIAMETER (d1)	17.3	18.23	80	ок		
OUTTER DIAMETER (d2)	44.28	45.21	80	ок		
THICKNESS (h)	2.75	4.06	80	ок		

MACHANICAL PROPERTIES According to: ISO 6507

Test Item Spec.		Sampling	Remark	Test Result
HARDNESS (HRC/HV)	HV10 140 ~ HV10 250	10	ок	HV10 145 ~ HV10 150

COATING According to: ISO 4042

Test Item	Spec.	Sampling	Remark	Test Result
Plating thickness	min.3 µm	5	ок	4.573 μm - 5.328 μm
ISST	2 hours no white corrosion and 12 hours no red rust	5	ок	ок

We hereby certify that all the above material were manufactured, sampled, tested, and inspected in accordance with the relevant specification and any supplementary requirements or other requirements designated in the purchase order and was found to meet those requirements.

Inspector: QC Chen Inspec. Date: 2020.11.16



Figure B-31. %-in. Round Washer, Test No. FLAGT-2 (Item No. g2)

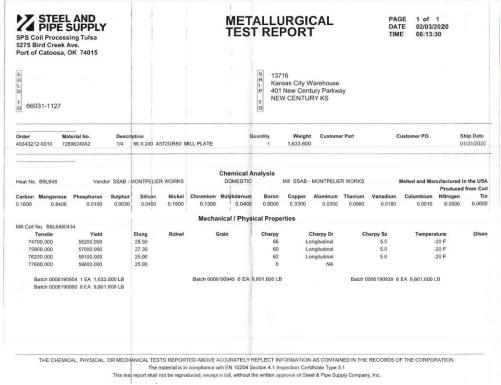


Figure B-32. Square Washer Plate, Test No. FLAGT-2 (Item No. g3)

#### CERTIFIED MATERIAL TEST REPORT FOR USS FLAT WASHERS HDG FACTORY: IFI & Morgan Ltd REPORT DATE: 22/10/2018 ADDRESS: Chang'an North Road, Wuyuan Town, Haiyan, Zhejiang, China SAMPLING PLAN PER ASME B18.18-11 PO NUMBER: 3240PCS SIZE: USS 1 HDG QNTY(Lot size): HEADMARKS: NO MARK PART NO: 33188 DIMENSIONAL INSPECTIONS SPECIFICATION: ASTM B18.21.1-2011 CHARACTERISTICS SPECIFIED ACTUAL RESULT ACC. \*\*\*\*\*\*\*\*\*\*\*\* APPEARANCE ASTM F844 PASSED 100 0 2.492-2.529 2.496-2.504 0 OUTSIDE DIA 10 INSIDE DIA 1.055-1.092 1.080-1.089 0 THICKNESS 0.135-0.192 0.135-0.157 0 10 CHARACTERISTICS TEST METHOD SPECIFIED ACTUAL RESULT REJ. \*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\*\*\*\*\* \*\*\*\*\*\*\* HOT DIP GALVANIZED ASTM F2329-13 Min 0.0017" 0.0017-0.0020 ALL TESTS IN ACCORDANCE WITH THE METHODS PRESCRIBED IN THE APPLICABLE ASTM SPECIFICATION. WE CERTIFY THAT THIS DAIA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL SUPPLIES TESTING LABORATORY. ISO 9001:2015 SGS Certificate # HK04/0105 检验专用章

Figure B-33. 1-in. Round Washer, Test No. FLAGT-2 (Item No. g4)



DECKER MANUFACTURING CORPORATION 703 N. Clark Street Albion, Michigan 49224 P: 517.629.3955 • F: 517.629.8535

### LABORATORY AND TESTING FACILITY

Reaffirmed to be in compliance to current Rev Level. Form 8.0
ORIGINAL LABORATORY AND/OR INSPECTION REPORT
THIS IS A LEGAL DOCUMENT

NAME AND ADDRESS OF CLIENT:	
PAGE _ L OF _ 2 _ LAB FILE ID NUMBER/LOT NUMBER:	DATE OF MANUFACTURE: 3-5-2020
DMC PART NUMBER #: 035-10	031-26
ITEM DESCRIPTION: 5/8 X [1 +.03] 2B GU	
GRADE ID MARK AND INSIGNIA: DMC	S
NAME (S) OF PERSON (S) SAMPLINGM.S	ORTH / J. Country SAMPLING
	SION OF DECKER MANUFACTURING CORPORATION'S
QUALITY DEPARTMENT.	FABILITY/CONDITION OF TEST SPECIMENS: _ACCEPTABLE
TOTAL NO. OF SAMPLES INSPECTED AND/	
	PECTIONS AND/OR TESTS:
INSPECTION/TEST DATE (S): 3-6-2	
DESCRIPTION (S): ROCKWELL HRB	PROOFLOAD
SPECIFICATION (S): ASTM E18	ASTM F606
REQUIREMENTS: ASTM-A563 GRADE P. Gr. HRB 69	
EQUIPMENT ID: # FH10000120120012	# 184280
EQUIPMENT ID. 9	INSPECTION / TEST RESULTS:
UNIT OF MEASUREMEN	
(1) 88.2 (5) 85	
Commence of the commence of th	7.0 (2) 21 300 (6) 21 100
	3) 21,200 (7) 21,300
/ / discourse from the same of	The state of the s
The state of the s	6,3 (4) 21,200 (8) 21,300
RESULTS OBTAINED FROM: WRENCH FLATS	THE PROPERTY OF THE PARTY OF TH
SPECIFICATION OR MATERIAL GRADE AS	EVIDENCED: C-1010 NGAT - 104-0980
REMARKS OR DEVIATIONS: MEET AND	EXCEED ASTM A563 (99) GRADE A REOUIREMENTS OF EACH SAMPLE IS THE AVERAGE OF TWO READINGS.
PER ASIM FOUR SECTION 4 THE BARDNESS	OF EACH SAMPLE IS THE AVENOVE OF TWO REGULATORS.
HEATTREAT SUBSACE TREATMENT COA	TING, ETC.;: PROOFLOAD SAMPLES WERE GALVANIZED.
TO THE SPECIFICATIONS A	BOVE, THE SAMPLES INSPECTED AND/OR TESTED
IO 1115 OF PON 4C119 4D140 10	the A. and to users set the set the comment of the set of a series and
CONFORM: X ARE RESULT	TS ONLY: DO NOT CONFORM:
COM CAMPA	Contractive and Contractive an
APPROVED SIGNATORY	
OUALTY MANAGER	INSPECTED/AND/OR TESTED BY:
ON THE WANTER	White I have
Russell L. Wilson	Authorized Lab Technician
CERTIEV THAT THE ARCIVE TEST(S) WERE CONDUCT	TED IN ACCORDANCE WITH THE ABOVE STATED SPECIFICATION(S) AND THAT THE
SESURTS ARE CORRECT AS INTERED. THE ABOVE RESI	ulte only pertain to the sample items tested, see the quality manual for
MANDATORY REPORT CONTENT. THIS DOCUMENT	SHALL NOT BE REPRODUCED IN FULL WITHOUT THE APPROVAL OF DECKER
MANUFACTURING CORPORATION, DO NOT BRASE OR A	lter any errors draw a straight line through and initial. See reverse of
THIS DOCUMENT FOR THE TERMS AND CONDITIONS OF	THIS TEST REPORT. THE DECISION RULE IS SIMPLE EXCEPTANCE.
	ACCREDITED
	TESTING CERT #0499-01

Figure B-34. %-in. Heavy Hex Nut, Test No. FLAGT-2 (Item No. h1)



# GEM-YEAR TESTING LABORATORY CERTIFICATE OF INSPECTION

MANUFACTURER : GEM-YEAR INDUSTRIAL CO., LTD.

ADDRESS: NO.8 GEM-YEAR

ROAD, E.D.Z., JIASHAN, ZHEJIANG, P.R. CHINA

PURCHASER: FASTENAL COMPANY PURCHASING

PO. NUMBER: 110216407

COMMODITY: FINISHED HEX NUT GR-A SIZE: 5/8-11 NC 0/T 0.51MM

LOT NO: 1N1680027

SHIP QUANTITY: 23, 400 PCS LOT QUANTITY 170, 278 PCS

**HEADMARKS:** 

MANUFACTURE DATE: 2016/08/26

COUNTRY OF ORIGIN: CHINA

Tel: (0573)84185001(48Lines) Fax: (0573)84184488 84184567

DATE: 2017/03/23

PACKING NO: GEM160919007 INVOICE NO: GEM/FNL-160929WI

PART NO: 36713 SAMPLING PLAN:

ASME B18.18-2011 (Category.2) / ASTM F1470-2012

HEAT NO: **331608011**MATERIAL: ML08

FINISH: HOT DIP GALVANIZED PER ASTM A153-

2009/ASTM F2329-2013

R#17-507 H#331608011 BCT Cable Bracket Nuts

### PERCENTAGE COMPOSITION OF CHEMISTRY:ACCORDING TO ASTM A563-2007

Chemistry	AL%	C%	MN%	P%	S%	SI%
Spec. : MIN.						
MAX.		0.5800		0.1300	0. 2300	
Test Value	0. 0350	0.0700	0. 4100	0.0160	0. 0060	0.0500

#### DIMENSIONAL INSPECTIONS :ACCORDING TO ASME B18. 2. 2-2010

SAMPLED BY: DWTING

INSPECTIONS ITEM	SAMPLE	SP	ECIFIED	ACTUAL RESULT	ACC.	REJ.
WIDTH ACROSS CORNERS	6 PCS		1.0510-1.0830 inch	1.0560-1.0690 inch	6	0
FIM	15 PCS	ASME B18. 2. 2-2010	Max. 0.0210 inch	0.0020-0.0040 inch	15	0
THICKNESS	6 PCS		0.5350-0.5590 inch	0.5390-0.5570 inch	6	0
WIDTH ACROSS FLATS	6 PCS		0.9220-0.9380 inch	0.9240-0.9340 inch	6	0
SURFACE DISCONTINUITIES	29 PCS		ASTM F812-2012	PASSED	29	0
THREAD	15 PCS		GAGING SYSTEM 21	PASSED	15	0

#### MECHANICAL PROPERTIES: ACCORDING TO ASTM A563-2007

SAMPLED BY: GDAN LIAN

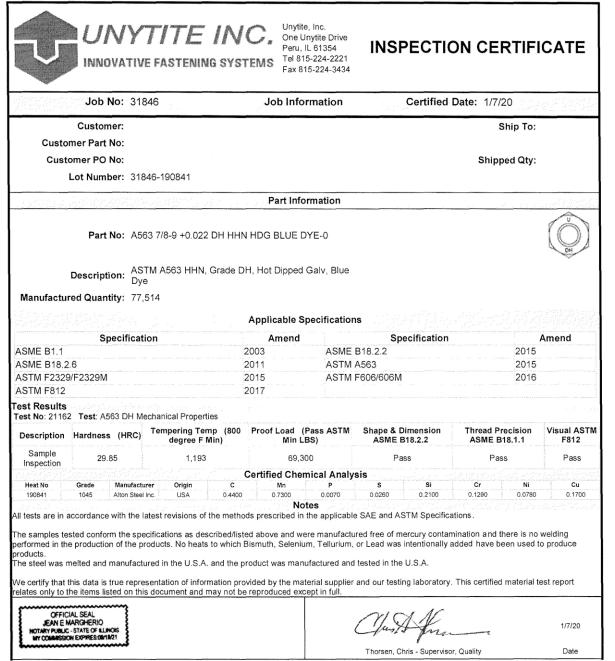
19 riv

INSPECTIONS ITEM	SAMPLE	TEST METHOD	REF	SPECIFIED	ACTUAL RESULT	ACC.	REJ.
CORE HARDNESS	15 PCS	ASTM F606-2014		68-107 HRB	79-81 HRB	15	0
PROOF LOAD	4 PCS	ASTM F606-2014		Min. 90 KSI	OK	4	0
PLATING THICKNESS ( µ m)	5 PCS	ASTM B568-1998		>=53	70. 02-75. 81	5	0

WE CERTIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL SUPPLIER AND OUR TESTING LABORATORY .WHICH ACCREDITED BY ISO/IEC17025(CERTIFICATE NUMBER:3358.01)
WE CERTIFY THAT THE PRODUCTS SUPPLIED ARE IN COMPLIANCE WITH THE REQUIREMENTS OF THE ORDER

Quality Supervisor:

Figure B-35. %-in. Hex Nut, Test No. FLAGT-2 (Item No. h2)



Plex 1/7/20 2:39 PM cthorsen Page 1

Figure B-36. %-in. Heavy Hex Nut, Test No. FLAGT-2 (Item No. h3)



# GEM-YEAR TESTING LABORATORY CERTIFICATE OF INSPECTION

MANUFACTURER : GEM-YEAR INDUSTRIAL CO., LTD.

ADDRESS: NO.8 GEM-YEAR

ROAD.E.D.Z..JIASHAN.ZHEJIANG.P.R.CHINA

PURCHASER: FASTENAL COMPANY PURCHASING

PO. NUMBER: 110254885

COMMODITY: FINISHED HEX NUT GR-A SIZE: 7/8-9 NC 0/T 0.56MM

LOT NO: 1N1810005

SHIP QUANTITY: 9, 000 PCS LOT QUANTITY 55, 748 PCS

**HEADMARKS:** 

MANUFACTURE DATE: 2018/01/05
COUNTRY OF ORIGIN: CHINA

Tel: (0573)84185001(48Lines) Fax: (0573)84184488 84184567

DATE: 2018/03/28

PACKING NO: GEM180115010
INVOICE NO: GEM/FNL-180201WI-1

PART NO: 36717 SAMPLING PLAN:

ASME B18.18-2011 (Category.2) / ASTM F1470-2012

HEAT NO: 331704677 MATERIAL: XGML08

FINISH: HOT DIP GALVANIZED PER ASTM A153-

2009/ASTM F2329-2013

SAMPLED BY: TANCHAO

Grin

PERCENTAGE COMPOSITION OF CHEMISTRY: ACCORDING TO ASTM A563-2015

Chemistry	AL%	C%	MN%	P%	S%	SI%
Spec. : MIN.					-	
MAX.		0.5800		0.1300	0. 2300	
Test Value	0. 0360	0.0600	0. 4500	0.0140	0.0030	0.0300

#### DIMENSIONAL INSPECTIONS :ACCORDING TO ASME B18. 2. 2-2015

			SAMPLED	BY: WDANDAN		
INSPECTIONS ITEM	SAMPLE	SP	PECIFIED	ACTUAL RESULT	ACC.	REJ.
WIDTH ACROSS CORNERS	5 PCS		1.4470-1.5160 inch	1.4850-1.4930 inch	5	0
FIM	15 PCS	ASME B18. 2. 2-2015	Max. 0.0250 inch	0.0110-0.0200 inch	15	0
THICKNESS	5 PCS		0.7240-0.7760 inch	0.7460-0.7570 inch	5	0
WIDTH ACROSS FLATS	5 PCS		1.2690-1.3120 inch	1.2930-1.2980 inch	5	0
SURFACE DISCONTINUITIES	29 PCS		ASTM F812-2012	PASSED	29	0
THREAD	15 PCS		GAGING SYSTEM 21	PASSED	15	0

MECHANICAL PROPERTIES: ACCORDING TO ASTM A563-2015

					TILIOILIO			
INSPECTIONS ITEM	SAMPLE	TEST METHOD	REF	SPECIFIED	ACTUAL RESULT	ACC.	REJ.	
CORE HARDNESS	15 PCS	ASTM F606-2014		68-107 HRB	86-90 HRB	15	0	
PROOF LOAD	5 PCS	ASTM F606-2014		Min. 31,416 LBF	OK	5	0	
PLATING THICKNESS ( µ m)	29 PCS	ASTM B568-1998		>=53	62. 38-62. 57	29	0	į

WE CERTIFY THAT THIS DATA IS A TRUE REPRESENTATION OF INFORMATION PROVIDED BY THE MATERIAL SUPPLIER AND OUR TESTING LABORATORY .WHICH ACCREDITED BY ISO/IEC17025(CERTIFICATE NUMBER:3358.01)
WE CERTIFY THAT THE PRODUCTS SUPPLIED ARE IN COMPLIANCE WITH THE REQUIREMENTS OF THE ORDER

Quality Supervisor:

Figure B-37. %-in. Hex Nut, Test No. FLAGT-2 (Item No. h4)

Nov. 26. 2018 3:47PM Fastenal-NELIN FASTENAL®

No. 5947 P. 2

## **Certificate of Compliance**

Sold To:		urchase Order:	STBR
UNL TRANSPORTATION	J	ob:	Item# f3, h1 and i1
	I	nvoice Date:	11/8/2018
. THIS IS TO CERTIFY THAT WE F			
80 PCS 1"-8 Hot Dipped Galvanized A563 Grade DH He 210157128 AND UNDER PART NUMBER 38210	avy Hex Nut Made I	n USA SUPPLIED UNI	DER OUR TRACE NUMBER
450 PCS:3/4"-10 Hot Dipped Galvanized A563 Grade DF 210169774 AND UNDER PART NUMBER 38208.	I Heavy Hex Nut Ma	ade In USA SUPPLIED	UNDER OUR TRACE NUMBER
80 PCS 1"-8 Hot Dipped Galvanized A563 Grade DH He 210157128 AND UNDER PART NUMBER 38210	avy Hex Nut Made I	n USA SUPPLIED UNI	DER OUR TRACE NUMBER
This is to certify that the above document is true	Plea	se check current revision	to avoid using obsolete copies.
and accurate to the best of my knowledge.			
della	This time		on 11/26/2018 and was current at that
Fastenal Account Representative Signature	Fast	enal Store Location/A	ddress
ALLE	320	I N. 23rd Street STE I	
My Stanczy -		COLN, NE 68521	•
Printed Name		ne #; (402)476-7900	
11/29/12	Fax	#: 402/476-7958	
11101110			

Figure B-38. 1-in. Heavy Hex Nut, Test No. FLAGT-2 (Item No. h5)



### Norfolk Iron & Metal Co.

3001 North Victory Road Norfolk, NE 68701 PH: (402) 371-1810

Document: 01131005

Sold To: RIVERS METAL PRODUCTS 3100 N 38TH ST LINCOLN, NE 68504

Sales Order: 01414254

Ship To: RIVERS METAL PRODUCTS 3100 N 38TH ST LINCOLN, NE 68504

Customer PO: /po 51265

Product Information

30185 - PLATE 3/16 A36 COLD REDUCED

Thickness: .1875 Width: 48.0000

Length: 96.0000

Mill Coil: 5301939 NLMK IN

Heat: Y6325 Supplier: NLMK INDIANA

Specification(s): ASTM A36 PLATE-19, ASME SA36-2019

Chemistry Data

Mechanical Data

Produced From Coil

	Yield (PSI)	Tensile (PSI)	Elongation	Reduction Of Area
1	48873	64548	44.63 2"	72.5300
2	48389	64315	40.39 2"	62.7900

Sample Taken From

> Head Center

The Mechanical Data for the product described above reflect the results of tests made by us in accordance with applicable ASTM or ASME standards and our testing procedures, and we certify that the information included in this Test Certificate with respect to such Mechanical Data is accurate to the best of our knowledge.

The Chemistry Data shown above was reported to us by NLMK INDIANA Test Certificate solely for your information. and have been included in this

Figure B-39. <sup>3</sup>/<sub>16</sub>-in. AGT Connector Face Plate, Test No. FLAGT-2 (Item No. i1)



### Norfolk Iron & Metal Co.

3001 North Victory Road Norfolk, NE 68701 PH: (402) 371-1810

Document: 01130910

Sold To: RIVERS METAL PRODUCTS 3100 N 38TH ST LINCOLN, NE 68504

Sales Order: 01378617

Ship To: RIVERS METAL PRODUCTS 3100 N 38TH ST LINCOLN, NE 68504

Customer PO: /po 50335

Product Information

25872 - PLATE 1/4 A36 COLD REDUCED

Thickness: .2500 Width: 48.0000

Length: 96.0000

Mill Coil: 363757 ARC BH

Heat: 813L65970 Supplier: ARCELORMITTAL

Specification(s):
ASTM A36 PLATE-19, ASME SA36-2019

Chemistry Data

.16	MN .87	P .011	S .004	.009	AL .039	CB .002	.001	.014	CR .02
NI .01	MO .002	SN .003	TI .002	N .004	B .0002	ZR	PB . 00	MG . OO	ZN

Mechanical Data

Sample Taken From	Reduction Of Area	Elongation	Tensile (PSI)	Yield (PSI)	
Head	53.4500	40.15 2"	64129	41580	1
Center	59.7600	42.52 2"	62242	42270	2

Produced From Coil

Melted In: UNITED STATES, Manufacured In: UNITED STATES

The Mechanical Data for the product described above reflect the results of tests made by us in accordance with applicable ASTM or ASME standards and our testing procedures, and we certify that the information included in this Test Certificate with respect to such Mechanical Data is accurate to the best of our knowledge.

The Chemistry Data shown above was reported to us by ARCELORMITTAL Test Certificate solely for your information. and have been included in this

Figure B-40. 1/4-in. AGT Connector Gusset Plates, Test No. FLAGT-2 (Item Nos. i2 through i7)

### Appendix C. Vehicle Center of Gravity Determination

		Test Name:		VIN:		R6GG1FS6	
Model Year:	2015	Make:_	Dodge	Model:		Ram 1500	
Vehicle CG	Determina	tion					
				Weight	Vertical	Vertical M	
Vehicle Equip				(lb)	CG (in.)	(lb-in.)	7
	•••••	d Truck (Curb)		4900	28.379184	139058	000
	Hub			19	14.75	280.25	
		ation cylinder 8		7	28.25	197.75	
		tank (Nitrogen)		30	28	840	_
	Strobe/Bral			10	27.75	277.5	
+	Brake Rece	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		6	53.25	319.5	_
+		ncluding DAQ		30	30.25	907.5	_
	Battery			-42	42.25	-1774.5	_
	Oil			-19	15.25	-289.75	
	Interior			-96	38.75	-3720	_
	Fuel			-176	19.75	-3476	
	Coolant			-10	35.5	-355	4
***********	Washer flui			-4	32.75	-131	
		ast (In Fuel Tan		195	19	3705	_
		upplemental Ba	attery			0	_
+	Steel Balla	st		133	35.75	4754.75	~
		***************************************		***************************************			
Note: (+) is adde		o vehicle, (-) is rei Estimated Tota Vertical CG	al Weight (lb)	4983		0 140594	
		Estimated Total Vertical CG	al Weight (lb) Location (in.)	4983		_	
Vehicle Dim	ensions for	Estimated Total Vertical CG	al Weight (lb) Location (in.) tions	4983 28.21473		140594	_
Vehicle Dim	ensions for	Estimated Total Vertical CG	al Weight (lb) Location (in.) tions Front Ti	4983 28.21473	68.25	140594 in.	_
<b>Vehicle Dim</b> e Wheel Base:	<b>ensions for</b> 140.5	Estimated Tota Vertical CG C.G. Calculat in.	al Weight (lb) Location (in.) tions Front Ti Rear Ti	4983 28.21473 rack Width:	68.25 67.75	140594 in. in.	-
<b>Vehicle Dim</b> o Wheel Base: <b>Center of Gr</b>	ensions for 140.5 avity	Estimated Total Vertical CG C.G. Calculation.	al Weight (lb) Location (in.) tions Front Ti Rear Ti	4983 28.21473 rack Width:	68.25 67.75 Fest Inertia	140594 in. in.	Difference
Vehicle Dime Wheel Base: Center of Gra Test Inertial V	ensions for 140.5 avity Veight (lb)	Estimated Total Vertical CG C.G. Calculation.	al Weight (lb) Location (in.) tions Front To Rear To SH Targets ± 110	4983 28.21473 rack Width:	68.25 67.75 <b>Fest Inertia</b> 5000	140594 in. in.	Difference
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C	ensions for 140.5 avity Veight (lb) CG (in.)	Estimated Total Vertical CG of C.G. Calculation.  2270P MAS 5000 63	al Weight (lb) Location (in.) tions Front To Rear To SH Targets ± 110	4983 28.21473 rack Width:	68.25 67.75 Fest Inertia 5000 65.8102	140594 in. in.	Difference 0.0 2.81020
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C Lateral CG (ii	avity Veight (lb)	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 63 NA	al Weight (lb) Location (in.) tions Front Ti Rear Ti  SH Targets ± 110 ± 4	4983 28.21473 rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68	140594 in. in.	Difference 0.0 2.81020 NA
Vehicle Dime Wheel Base:  Center of Gra Test Inertial Wallongitudinal Cultural CG (in Vertical CG)	avity Veight (lb) CG (in.) n.)	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 of 63 of NA 28 of Calculate In	al Weight (lb) Location (in.) tions Front Ti Rear Ti  SH Targets ± 110 ± 4 or greater	4983 28.21473 rack Width:	68.25 67.75 Fest Inertia 5000 65.8102	140594 in. in.	Difference 0.0 2.81020 NA 0.21473
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (Note: Long. CG Note: Lateral CC	ensions for 140.5 avity Veight (Ib) CG (in.) n.) in.) is measured from the control of the contro	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 63 NA	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 <b>Fest Inertia</b> 5000 65.8102 -0.68 28.21	140594 in. in.	Difference 0.0 2.81020 NA
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (Note: Long. CG Note: Lateral CC	ensions for 140.5 avity Veight (Ib) CG (in.) n.) in.) is measured from the control of the contro	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 of 63 of NA 28 of rom front axle of the second control of the seco	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21	140594 in. in.	Difference 0.0 2.81020 NA 0.21473
Vehicle Dime Wheel Base:  Center of Gr. Test Inertial W Longitudinal C Lateral CG (in Vertical CG (	ensions for 140.5 avity Veight (Ib) CG (in.) n.) in.) is measured from the control of the contro	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 of 63 of NA 28 of rom front axle of the second control of the seco	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21	in.	Difference 0.0 2.81020 NA 0.21473
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (in Vertical CG (in Note: Long. CG Note: Lateral CC	avity Veight (Ib) CG (in.) n.) is measured from the measured from	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 63 NA 28 of rom front axle of the community of the communit	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21	in. in.	Difference 0.0 2.81020 NA 0.21473
Vehicle Dime Wheel Base: Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (Note: Long. CG Note: Lateral CC	avity Veight (Ib) CG (in.) n.) is measured fr HT (Ib) Left	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 63 NA 28 of rom front axle of the form centerline - po	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21 ger) side	in. in. Left	Difference 0.0 2.81020 NA 0.21473
Vehicle Dime Wheel Base:  Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (in Note: Long. CG Note: Lateral CC CURB WEIGH Front Rear	avity Veight (lb) CG (in.) n.) is measured from the measured from	Estimated Total Vertical CG of C.G. Calculate in.  2270P MAS 5000 of 63 of NA 28 of NA 28 of tom centerline - poor Right 1325 1096	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21 ger) side TEST INER	in. in.  I  Left 1373 1177	Difference 0.0 2.81020 NA 0.21473  HT (lb)  Right 1285 1165
Vehicle Dime Wheel Base:  Center of Gra Test Inertial W Longitudinal C Lateral CG (in Vertical CG (in Note: Long. CG Note: Lateral CC CURB WEIGH	avity Veight (lb) CG (in.) n.) is measured from the measured from	Estimated Total Vertical CG   C.G. Calculate in.   2270P MAS   5000   63   NA   28   rom front axle of tom centerline - po   Right   1325	al Weight (lb) Location (in.)  tions  Front Ti Rear Ti  SH Targets ± 110 ± 4  or greater est vehicle	4983 28.21473 rack Width: rack Width:	68.25 67.75 Fest Inertia 5000 65.8102 -0.68 28.21 ger) side TEST INER	in. in. Left 1373	Difference 0.0 2.81020 NA 0.21473  HT (lb)  Right 1285

Figure C-1. Vehicle Mass Distribution, Test No. FLAGT-2

### Appendix D. Static Soil Tests

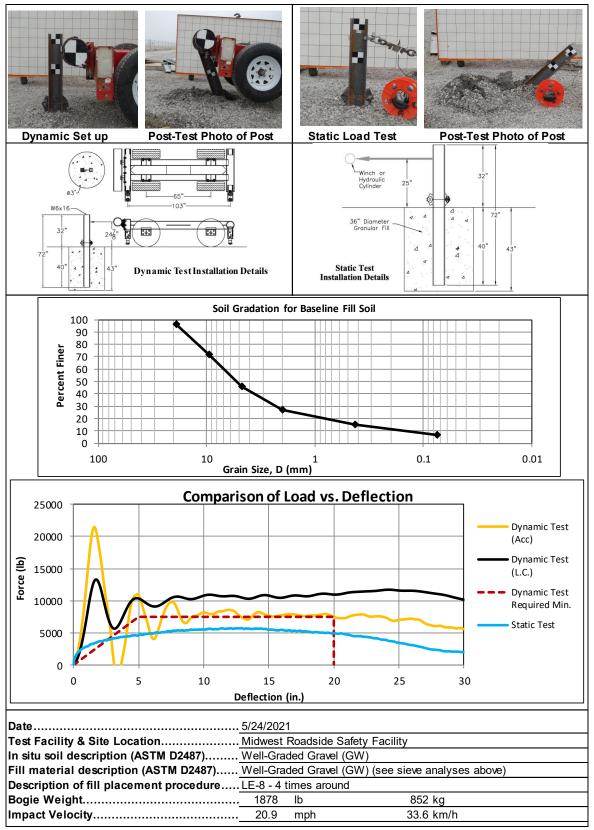


Figure D-1. Soil Strength, Initial Calibration Tests, Test No. FLAGT-2

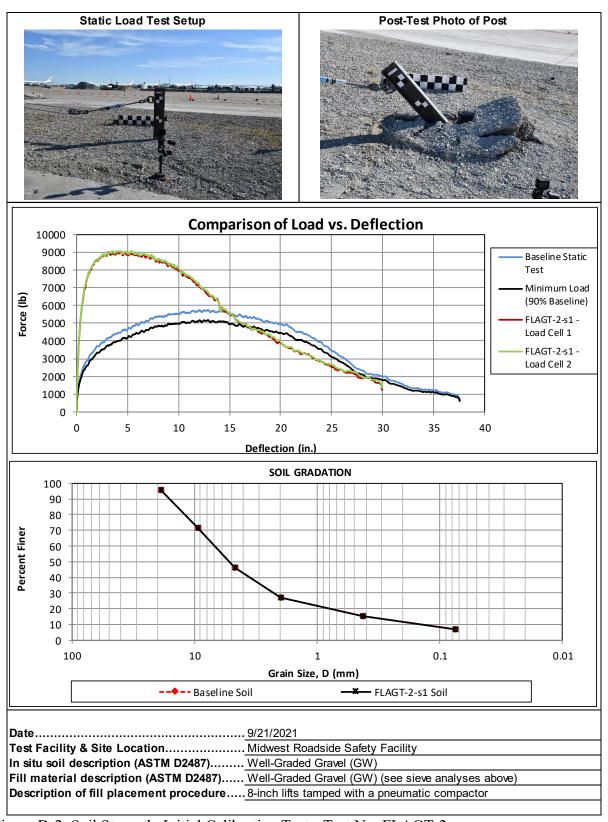


Figure D-2. Soil Strength, Initial Calibration Tests, Test No. FLAGT-2

### Appendix E. Vehicle Deformation Records

The following figures and tables describe all occupant compartment measurements taken on the test vehicles used in full-scale crash testing herein. MASH defines intrusion as the occupant compartment being deformed and reduced in size with no penetration. Outward deformations, which are denoted as negative numbers within this Appendix, are not considered as crush toward the occupant, and are not subject to evaluation by MASH criteria.

		Test Name:	FLAGT-2	VIN:	1C6RR6GG1FS646171	
Model Year: _	2015	Make:	Dodge	Model:	Ram 1500	

## VEHICLE DEFORMATION PASSENGER SIDE FLOOR PAN - SET 1

	POINT	Pretest X (in.)	Pretest Y (in.)	Pretest Z (in.)	Posttest X (in.)	Posttest Y (in.)	Posttest Z (in.)	ΔX <sup>A</sup> (in.)	ΔΥ <sup>A</sup> (in.)	ΔΖ <sup>A</sup> (in.)	Total Δ (in.)	Crush <sup>B</sup> (in.)	Directions for Crush <sup>C</sup>
	1	53.5043	32.2987	-8.2422	51.7693	31.6227	-11.0980	1.7350	0.6760	2.8558	3.4092	3.3415	X, Z
	2	54.4983	37.5048	-5.8078	51.1808	37.5369	-10.3241	3.3175	-0.0321	4.5163	5.6039	5.6038	X, Z
	3	55.7276	41.8300	-1.6980	50.1126	38.8441	-6.5177	5.6150	2.9859	4.8197	7.9796	7.3998	X, Z
╛┪	4	55.8571	46.7938	-1.5774	49.1222	41.9654	-8.1601	6.7349	4.8284	6.5827	10.5832	9.4176	X, Z
TOE PAN - WHEEL WELL (X, Z)	5	56.0314	51.5521	-1.6715	48.5438	45.6075	-7.7679	7.4876	5.9446	6.0964	11.3388	9.6556	X, Z
戸耳べ	6	49.3907	30.9471	-6.4452	48.5483	30.5320	-8.3598	0.8424	0.4151	1.9146	2.1325	2.0917	X, Z
2 불	7	50.9718	37.6328	-3.0213	47.2119	37.3359	-8.2992	3.7599	0.2969	5.2779	6.4870	6.4802	X, Z
>	8	52.0696	41.6561	0.4883	46.7832	39.5512	-5.0595	5.2864	2.1049	5.5478	7.9470	7.6632	X, Z
	9	51.8156	47.5672	0.6746	45.7551	43.2220	-5.2305	6.0605	4.3452	5.9051	9.5121	8.4617	X, Z
	10	51.8293	52.3221	0.6694	44.3843	45.2958	-5.8725	7.4450	7.0263	6.5419	12.1488	9.9108	X, Z
	11	45.8851	31.2525	-5.5707	45.5052	30.9626	-6.8795	0.3799	0.2899	1.3088	1.3933	1.3088	Z
	12	47.8350	36.6860	0.0450	44.2675	36.6049	-5.0271	3.5675	0.0811	5.0721	6.2016	5.0721	Z
	13	48.3024	41.2820	1.2559	43.3963	40.3120	-4.2991	4.9061	0.9700	5.5550	7.4745	5.5550	Z
	14	48.2137	47.6428	1.2703	44.5368	45.0486	-2.7262	3.6769	2.5942	3.9965	6.0184	3.9965	Z
	15	48.2192	52.5581	1.2924	42.6289	48.1872	-5.2537	5.5903	4.3709	6.5461	9.6544	6.5461	Z Z
	16	42.7456	31.6399	-4.5480	42.4920	31.5176	-5.5491	0.2536	0.1223	1.0011	1.0399	1.0011	Z
	17	43.4819	36.6510	1.2606	42.3633	37.5563	-2.0949	1.1186	-0.9053	3.3555	3.6511	3.3555	Z
_	18	43.0909	40.9241	1.2514	41.2924	41.1864	-2.7472	1.7985	-0.2623	3.9986	4.3923	3.9986	Z
PAN	19	43.9895	46.4592	1.2596	41.5854	45.6139	-0.8510	2.4041	0.8453	2.1106	3.3089	2.1106	Z
	20	44.4702	52.7128	1.2980	41.0584	49.8454	-2.3211	3.4118	2.8674	3.6191	5.7411	3.6191	Z
FLOOR (Z)	21	38.4214	31.9696	-3.7231	38.1458	32.3734	-4.6096	0.2756	-0.4038	0.8865	1.0124	0.8865	Z
1 5	22	39.3849	35.8622	1.2441	39.1422	36.3223	-0.0757	0.2427	-0.4601	1.3198	1.4186	1.3198	Z
_	23	39.2211	40.7360	1.2435	38.0036	40.9792	-1.1594	1.2175	-0.2432	2.4029	2.7047	2.4029	Z
	24	39.2249	46.3939	1.2554	38.0966	46.4350	0.0455	1.1283	-0.0411	1.2099	1.6549	1.2099	Z
	25	39.4934	52.8125	1.2784	38.3809	51.7373	0.0241	1.1125	1.0752	1.2543	1.9917	1.2543	Z
	26	34.6309	31.9493	-3.6257	34.5097	32.1849	-4.1654	0.1212	-0.2356	0.5397	0.6012	0.5397	Z
	27	34.7147	35.2391	0.4648	34.6491	35.4162	0.2706	0.0656	-0.1771	0.1942	0.2709	0.1942	Z
	28	34.6963	41.0112	0.4538	34.5037	41.0107	-0.1777	0.1926	0.0005	0.6315	0.6602	0.6315	Z
	29	34.6425	46.1564	0.4646	34.5383	46.2180	-0.1864	0.1042	-0.0616	0.6510	0.6622	0.6510	Z Z
	30	34.6440	52.4057	0.2724	34.5107	52.4230	0.0671	0.1333	-0.0173	0.2053	0.2454	0.2053	Z

A Positive values denote deformation as inward toward the occupant compartment, negative values denote deformations outward away from the occupant compartment

<sup>&</sup>lt;sup>C</sup> Direction for Crush column denotes which directions are included in the crush calculations. If "NA" then no intrusion is recorded, and Crush will be 0.

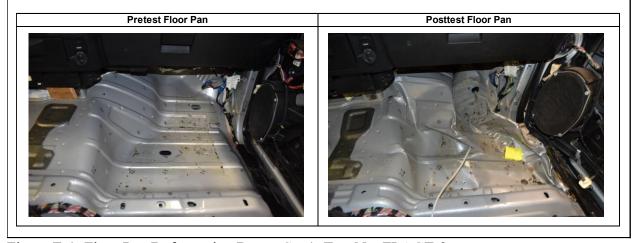


Figure E-1. Floor Pan Deformation Data – Set 1, Test No. FLAGT-2

<sup>&</sup>lt;sup>B</sup> Crush calculations that use multiple directional components will disregard components that are negative and only include positive values where the component is deforming inward toward the occupant compartment.

lodel Year:	20	115			Test Name: Make:		GT-2 dge			VIN: Model:	1C6R	R6GG1FS6 Ram 1500	346171
					VE	HICLE DE	FORMATI	ION					
				PAS			ERIOR CI		ET 1				
		Pretest	Pretest	Pretest	Posttest X	Posttest	Posttest Z	ΔX <sup>A</sup>	ΔY <sup>A</sup>	$\Delta Z^{A}$	Total Δ	Crush <sup>B</sup>	Directions
	POINT	X (in.)	Y (in.)	Z (in.)	(in.)	Y (in.)	(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	for Crush <sup>C</sup>
	1	45.2611	53.6665	-29.3848	44.6186	52.7321	-32.5680	0.6425	0.9344	-3.1832	3.3792	3.3792	X, Y, Z
<b>–</b> $\widehat{N}$	2	45.5987	40.8392	-30.4802	45.2934	39.9588	-32.9395	0.3053	0.8804	-2.4593	2.6299	2.6299	X, Y, Z
DASH (X, Y, Z)	3	42.4432	23.1615	-30.7978	42.3731	22.2010	-32.6415	0.0701	0.9605	-1.8437	2.0801	2.0801	X, Y, Z
Δ×̈	4	42.4587	53.5511	-19.9059	40.5556	52.7242	-23.6335	1.9031	0.8269	-3.7276	4.2662	4.2662	X, Y, Z
•	5	41.6188	41.3442	-19.7923	40.4706	40.5005	-22.7835	1.1482	0.8437	-2.9912	3.3132	3.3132	X, Y, Z
	6	40.0342	23.2954	-17.4752	38.6523	23.0779	-19.7194	1.3819	0.2175	-2.2442	2.6445	2.6445	X, Y, Z
드	7	51.9551	55.5381	-6.2285	46.2810	49.4208	-10.3035	5.6741	6.1173	-4.0750	9.2856	6.1173	Υ
SIDE PANEL (Y)	8	51.5503	55.4167	-2.1767	46.4096	47.8325	-6.0060	5.1407	7.5842	-3.8293	9.9303	7.5842	Υ
۵ ۵	9	53.2878	55.4568	-2.7630	48.0798	47.4030	-6.6191	5.2080	8.0538	-3.8561	10.3371	8.0538	Υ
Ш	10	39.6004	57.8119	-19.3037	37.5905	60.8442	-20.5386	2.0099	-3.0323	-1.2349	3.8418	-3.0323	Υ
SIDE	11	29.3356	57.9904	-18.7124	27.6276	62.9776	-19.3953	1.7080	-4.9872	-0.6829	5.3156	-4.9872	Y
	12	17.6257	57.6315	-18.7427	16.5893	60.6874	-19.2281	1.0364	-3.0559	-0.4854	3.2632	-3.0559	Υ
980	13	37.9698	58.5002	-5.0032	36.1863	57.3070	-6.8460	1.7835	1.1932	-1.8428	2.8285	1.1932	Υ
IMPACT DOO (Y)	14	30.3807	58.1575	-3.7490	28.6960	57.8092	-5.1370	1.6847	0.3483	-1.3880	2.2104	0.3483	Y
=	15	21.6148	57.5923	-4.3966	19.9018	58.9134	-5.1981	1.7130	-1.3211	-0.8015	2.3070	-1.3211	Υ
	16	34.2907	47.3781	-46.0864	34.4105	47.6771	-47.0911	-0.1198	-0.2990	-1.0047	1.0551	-1.0047	Z
	17	37.0563	37.3240	-46.5814	37.0954	37.5987	-47.1691	-0.0391	-0.2747	-0.5877	0.6499	-0.5877	Z
	18	38.3519	23.2377	-46.8594	38.4314	23.4196	-46.9731	-0.0795	-0.1819	-0.1137	0.2288	-0.1137	Z
	19	27.3436	44.7990	-49.3613	27.3092	44.8846	-50.2054	0.0344	-0.0856	-0.8441	0.8491	-0.8441	Z
	20	29.6701	35.5137	-49.7344	29.6973	35.6110	-50.3712	-0.0272	-0.0973	-0.6368	0.6448	-0.6368	Z
Ω	21	31.0914	22.3133	-49.8308	31.1636	22.5245	-50.0495	-0.0722	-0.2112	-0.2187	0.3125	-0.2187	Z
٩	22	12.1595	43.1085	-50.6700	12.1659	43.0594	-51.3706	-0.0064	0.0491	-0.7006	0.7023	-0.7006	Z
ROOF - (Z)	23	12.3199	34.2152	-51.1617	12.4631	34.1711	-51.6156	-0.1432	0.0441	-0.4539	0.4780	-0.4539	Z
õ	24	12.0609	21.4999	-51.4063	12.2081	21.5193	-51.7730	-0.1472	-0.0194	-0.3667	0.3956	-0.3667	Z
-	25	-1.8397	42.7698	-50.9864	-1.8069	42.7805	-51.5073	-0.0328	-0.0107	-0.5209	0.5220	-0.5209	Z
	26	-1.9267	34.9123	-51.3634	-1.7868	34.8050	-51.7955	-0.1399	0.1073	-0.4321	0.4667	-0.4321	Z
	27	-1.8197	20.4053	-51.5829	-1.6386	20.2959	-51.8910	-0.1811	0.1094	-0.3081	0.3738	-0.3081	Z
	28	-18.2937	43.2251	-50.7755	-18.2594	43.0403	-51.2144	-0.0343	0.1848	-0.4389	0.4775	-0.4389	Z
	29	-17.8241	32.8556	-51.1965	-17.6995	32.6406	-51.5947	-0.1246	0.2150	-0.3982	0.4694	-0.3982	Z
	30	-17.3041	20.7433	-51.3169	-17.1845	20.5058	-51.6420	-0.1196	0.2375	-0.3251	0.4200	-0.3251	Z
	31	53.2569	54.8108	-31.1890	53.1602	55.5662	-32.6322	0.0967	-0.7554	-1.4432	1.6318	0.0967	X
유류(2	32	49.6730	53.9555	-33.8205	50.2512	54.5300	-36.0113	-0.5782	-0.5745	-2.1908	2.3375	0.0000	NA
∃	33	44.3189	52.7881	-38.0165	44.7588	53.4900	-39.6803	-0.4399	-0.7019	-1.6638	1.8586	0.0000	NA
A-PILLAR Maximum (X, Y, Z)	34	40.7036	52.0293	-40.8258	40.9815	52.5957	-42.4156	-0.2779	-0.5664	-1.5898	1.7104	0.0000	NA
< ≥	35	38.0849	51.4320	-42.3903	38.3281	51.9126	-43.7704	-0.2432	-0.4806	-1.3801	1.4815	0.0000	NA
	36	34.8623	50.7276	-44.2701	35.0454	51.1116	-45.5729	-0.1831	-0.3840	-1.3028	1.3705	0.0000	NA
	31	53.2569	54.8108	-31.1890	53.1602	55.5662	-32.6322	0.0967	-0.7554	-1.4432	1.6318	-0.7554	Y
A-PILLAR Lateral (Y)	32	49.6730	53.9555	-33.8205	50.2512	54.5300	-36.0113	-0.5782	-0.5745	-2.1908	2.3375	-0.5745	Υ
ᆲᇉ	33	44.3189	52.7881	-38.0165	44.7588	53.4900	-39.6803	-0.4399	-0.7019	-1.6638	1.8586	-0.7019	Y
A-P	34	40.7036	52.0293	-40.8258	40.9815	52.5957	-42.4156	-0.2779	-0.5664	-1.5898	1.7104	-0.5664	Y
ت ۵	35	38.0849	51.4320	-42.3903	38.3281	51.9126	-43.7704	-0.2432	-0.4806	-1.3801	1.4815	-0.4806	Υ
	36	34.8623	50.7276	-44.2701	35.0454	51.1116	-45.5729	-0.1831	-0.3840	-1.3028	1.3705	-0.3840	Y
AR Z ∰ (2	37	10.1177	51.6254	-41.3322	10.0966	51.7204	-41.8631	0.0211	-0.0950	-0.5309	0.5397	0.0211	X
∃ ≝ ≻.	38	7.6464	53.2392	-37.1858	7.6410	53.2931	-37.6808	0.0054	-0.0539	-0.4950	0.4980	0.0054	X
3-PII Maxi (X, Č	39	10.9721	54.2783	-33.1808	10.9227	54.3665	-33.6810	0.0494	-0.0882	-0.5002	0.5103	0.0494	X
m ≥ ⊃	40	8 2207	54 8427	-28 6862	8 1799	54 8783	-29 2245	0.0408	-0.0356	-0.5383	0.5410	0.0408	X

-0.0356

-0.0539

-0.0882

-0.0356

0.0408

-29.2245

0.5410

0.5397

0.4980

0.5103

0.5410

-0.5383

-0.4950

-0.5002

-0.5383

-0.0950 -0.5309

0.0408

-0.0950

-0.0539

-0.0882

Figure E-2. Occupant Compartment Deformation Data – Set 1, Test No. FLAGT-2

10.1177 51.6254 -41.3322 10.0966 51.7204 -41.8631 0.0211

8.1799 54.8783

54.8427

-28.6862

8.2207

37

38

39

40

B-PILLAR Lateral (Y)

 <sup>7.6464
 53.2392
 -37.1858
 7.6410
 53.2931
 -37.6808
 0.0054

 10.9721
 54.2783
 -33.1808
 10.9227
 54.3665
 -33.6810
 0.0494

 8.2207
 54.8427
 -28.6862
 8.1799
 54.8783
 -29.2245
 0.0408</sup> ^ Positive values denote deformation as inward toward the occupant compartment, negative values denote deformations outward away from the occupant

<sup>&</sup>lt;sup>B</sup> Crush calculations that use multiple directional components will disregard components that are negative and only include positive values where the component is deforming inward toward the occupant compartment.

C Direction for Crush column denotes which directions are included in the crush calculations. If "NA" then no intrusion is recorded, and Crush will be 0.

odel Year:	2015			Test Name: FLAGT-2 Make: Dodge			VIN:_ Model:_	1C6RR6GG1FS646171 Ram 1500					
							FORMATION PA	- 0100111					
	POINT	Pretest X (in.)	Pretest Y (in.)	Pretest Z (in.)	Posttest X (in.)	Posttest Y (in.)	(in.)	ΔX <sup>A</sup> (in.)	ΔΥ <sup>A</sup> (in.)	ΔZ <sup>A</sup> (in.)	Total Δ (in.)	Crush <sup>B</sup> (in.)	Direction for Crush
	1	50.5209	12.6277	-5.1699	48.8957	12.2944	-7.8344	1.6252	0.3333	2.6645	3.1388	3.1210	X, Z
	2	51.5730	17.8369	-2.7665	48.3206	18.1910	-6.9273	3.2524	-0.3541	4.1608	5.2930	5.2811	X, Z
[	3	52.8892	22.1683	1.3095	47.3410	19.4196	-3.0713	5.5482	2.7487	4.3808	7.5848	7.0692	X, Z
TOE PAN - WHEEL WELL (X, Z)	4	53.0333	27.1321	1.4162	46.3098	22.5722	-4.6266	6.7235	4.5599	6.0428	10.1249	9.0400	X, Z
TOE PAN HEEL WE (X, Z)	5	53.2175	31.8897	1.3077	45.7377	26.2049	-4.1472	7.4798	5.6848	5.4549	10.8637	9.2576	X, 2
□ II 文	6	46.4382	11.2905	-3.2934	45.7407	11.1444	-5.0440	0.6975	0.1461	1.7506	1.8901	1.8844	X, Z
일 불	7	48.0993	17.9800	0.0848	44.4006	17.9440	-4.8144	3.6987	0.0360	4.8992	6.1387	6.1386	X, 2
` >	8	49.2723	22.0086	3.5639	44.0462	20.0928	-1.5214	5.2261	1.9158	5.0853	7.5394	7.2919	X, 2
	9	49.0363	27.9207	3.7412	43.0113	23.7650	-1.5939	6.0250	4.1557	5.3351	9.0572	8.0476	X, 2
	10	49.0617	32.6756	3.7246	41.6241	25.8496	-2.1615	7.4376	6.8260	5.8861	11.6858	9.4849	X, Z
	11	42.9503	11.6068	-2.3545	42.7329	11.5411	-3.4843	0.2174	0.0657	1.1298	1.1524	1.1298	Z
1	12	45.0178	17.0482	3.2112	41.5343	17.1433	-1.4896	3.4835	-0.0951	4.7008	5.8516	4.7008	Z
	13	45.5190	21.6458	4.4025	40.6774	20.8338	-0.6665	4.8416	0.8120	5.0690	7.0566	5.0690	Z
	14	45.4463	28.0067	4.4037	41.8506	25.5390	0.9747	3.5957	2.4677	3.4290	5.5477	3.4290	Z
	15	45.4644	32.9220	4.4142	39.8814	28.7256	-1.4433	5.5830	4.1964	5.8575	9.1154	5.8575	Z
	16	39.8313	12.0044	-1.2745	39.7512	12.0652	-2.0726	0.0801	-0.0608	0.7981	0.8044	0.7981	Z
	17	40.6880	17.0269	4.5077	39.6987	18.0327	1.5051	0.9893	-1.0058	3.0026	3.3175	3.0026	Z
	18	40.3074	21.3009	4.4958	38.6098	21.6739	0.9516	1.6976	-0.3730	3.5442	3.9474	3.5442	Z
¥	19	41.2198	26.8338	4.4743	38.9436	26.0625	2.9297	2.2762	0.7713	1.5446	2.8569	1.5446	Z
~ c	20	41.7165	33.0862	4.4892	38.3788	30.3221	1.5583	3.3377	2.7641	2.9309	5.2317	2.9309	Z
FLOOR PAN (Z)	21	35.5240	12.3468	-0.3700	35.4276	12.8962	-1.0144	0.0964	-0.5494	0.6444	0.8523	0.6444	Z
	22	36.5895	16.2483	4.5693	36.5269	16.7540	3.5739	0.0626	-0.5057	0.9954	1.1182	0.9954	Z
	23	36.4378	21.1226	4.5604	35.3594	21.4303	2.6116	1.0784	-0.3077	1.9488	2.2484	1.9488	Z
	24	36.4557	26.7804	4.5590	35.4761	26.8608	3.9241	0.9796	-0.0804	0.6349	1.1701	0.6349	Z
	25	36.7405	33.1984	4.5620	35.7556	32.1628	4.0034	0.9849	1.0356	0.5586	1.5344	0.5586	Z
	26	31.7360	12.3363	-0.2020	31.8031	12.6940	-0.4889	-0.0671	-0.3577	0.2869	0.4634	0.2869	Z
1	27	31.9041	15.6352	3.8786	32.0440	15.8352	4.0071	-0.1399	-0.2000	-0.1285	0.2758	-0.1285	Z
İ	28	31.8997	21.4073	3.8544	31.8834	21.4374	3.6756	0.0163	-0.0301	0.1788	0.1820	0.1788	Z
	29	31.8589	26.5527	3.8542	31.9135	26.6439	3.7715	-0.0546	-0.0912	0.0827	0.1347	0.0827	Z
	30	31.8722	32.8014	3.6474	31.8868	32.8425	4.1510	-0.0146	-0.0411	-0.5036	0.5055	-0.5036	Z

A Positive values denote deformation as inward toward the occupant compartment, negative values denote deformations outward away from the occupant compartment.

<sup>&</sup>lt;sup>C</sup> Direction for Crush column denotes which directions are included in the crush calculations. If "NA" then no intrusion is recorded, and Crush will be 0.

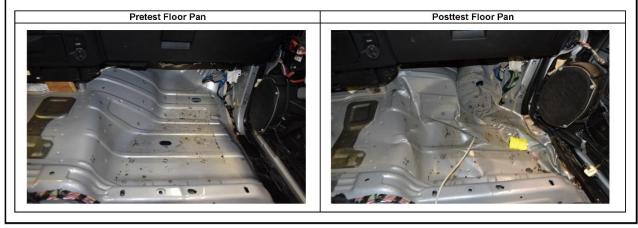


Figure E-3. Floor Pan Deformation Data – Set 2, Test No. FLAGT-2

<sup>&</sup>lt;sup>B</sup> Crush calculations that use multiple directional components will disregard components that are negative and only include positive values where the component is deforming inward toward the occupant compartment.

	Test Name:	FLAGT-2	VIN:	1C6RR6GG1FS646171					
Model Year:	2015	Make:	Dodge	Model:	Ram 1500				
VEHICLE DEFORMATION									

#### **PASSENGER SIDE INTERIOR CRUSH - SET 2** Pretest Pretest Pretest Directions Posttest $\mathsf{Crush}^\mathsf{B}$ Posttest X Posttest 7 $\Lambda X^A$ $\Lambda Y^A$ $\Lambda Z^{A}$ Total A (in.) (in.) (in.) (in.) (in.) <u>Crus</u>h<sup>C</sup> POINT -26.1864 X, Y, Z 33.9781 41.9396 41.2364 33.8208 -28.6923 0.7032 0.1573 -2.5059 2.6074 2.6074 42.2216 21.1482 -27.2670 41.9127 21.0585 -29.3373 0.3089 0.0897 -2.0703 2.0951 2.0951 X, Y, Z DASH (X, Y, Z) 3 39.0130 3.4787 -27.4954 39.0146 3.2945 -29.3302 -0.0016 0.1842 -1.83481.8440 1 8440 X, Y, Z X, Y, Z X, Y, Z 4 39.3184 33.8853 -16.6555 37.3822 33.6274 -19.6677 1.9362 0.2579 -3.0122 3.5901 3.5901 38.4479 -16.5059 21.6809 0.2919 -2.5572 2.8073 2.8073 37.3270 21.3890 -19.0631 1.1209 3.9058 -2.1807 2.5348 2.5348 3.6402 -14.1294 35.5946 49.0795 35.8681 -3.1651 43.4188 30.0632 -6.5438 5.6607 5.8049 -3.3787 8.7838 5.8049 SIDE PANEL (Y) 7.3655 48.7519 35.7542 0.8938 43.6486 28.3887 -2.28345.1033 7.3655 -3.17729.5073 9 50.4779 35.7886 0.2744 45.3044 27.9738 -2.9437 5.1735 7.8148 -3.2181 9.9092 7.8148 -3.5246 -0.3356 36.4836 -16.0058 34.4833 2.0003 IMPACT SIDE DOOR (Y) 38.3623 -15.2189 24.5479 43.7766 0.2948 5.6780 -5.4143 12 14.5232 38.0352 -15.0250 13.5184 41.4694 -14.5467 1.0048 -3.4342 0.4783 3.6100 -3.4342 13 35.1282 38.8702 -1.6779 33.4008 37.8651 -2.6941 2.2421 1.0051 1.7274 1.0051 -1.016214 27.5636 38.5502 -0.2785 25.9519 -0.8017 1.6117 0.2272 -0.5232 1.7097 38.3230 0.2272 15 18.7854 38.0079 17.1578 -0.6362 0.1215 2.1562 -1.4090 -0.757739.4169 1.6276 -1.409016 30.6353 27.6931 -42.6651 30.6974 29.0468 -43.0733 -0.0621 -1.3537 -0.4082 1.4153 -0.4082 17 33.3638 17.6307 -43.1964 33.3880 18.9754 -43.4173 -0.0242 -1.3447 -0.2209 1.3629 -0.2209 18 3.5406 -43.4760 34.7397 4.7970 -43.5389 -0.0629 1.2641 -0.0629 34.6159 -0.1238-1.256419 23.6200 25.1278 -45.8025 26.3085 -46.0776 0.0921 -1.1807 -0.2751 -0.2751 15.8356 -46.2048 25.9190 17.0432 -46.4861 -0.0051 -0.2813 1.2399 -0.2813 25.9139 -1.20761.3369 21 27.2975 2.6313 46.3068 27.4030 3.9548 -46.4630 -0.1055 -1.3235 -0.1562 -0.1562 Ñ 22 8.4092 23.4766 -46.8184 8.3631 24.4877 -46.9272 0.0461 -1.0111 -0.1088 1.0180 -0.1088 ROOF. 8.5363 14.5821 -47.2985 15.6066 -47.3586 -0.0601 1.0339 -0.0601 23 8.6617 -0.1254-1.024524 -47.5173 -47.7655 1.8672 2.9602 -0.1752 -1.09301.1344 -0.24828.2383 8.4135 -0.248225 -5.5944 23.1756 -46.8669 -5.6089 24.1937 -46.7449 0.0145 -1.0181 0.1220 1.0255 0.1220 26 -5.709715.3177 -47.2293 -5.5890 16.2257 -47.1945-0.1207-0.90800.0348 0.9166 0.0348 27 -5.6460 0.8102 -47.4271 -5.4312 1.7217 -47.5865 -0.2148 -0.9115 -0.15940.9499 -0.159428 -22.0400 23.6760 -46.3426 -22.0503 24.4264 -46.0646 0.0103 -0.7504 0.2780 0.8003 0.2780 29 -21.6065 13.3047 -46.7555 -21.4910 14.0373 -46.6678 -0.1155 -0.7326 0.0877 0.7468 0.0877 Ζ 30 -46.8661 1.9066 -46.9722 -0.1061 -21.1215 1.1908 -20.9673 -0.1542-0.7158 -0.1061 0.7399 31 49.9026 35.0977 -28.1448 49.7718 36.6666 -28.8976 0.1308 -1.5689 -0.7528 1.7451 0.1308 Χ A-PILLAR Maximum (X, Y, Z) 32 46.2667 34.2480 -30.7060 46.7859 35.6950 -32.2285 -0.5192 -1.4470 -1.5225 2.1637 0.0000 NA 33 40.8304 33.0886 -34.7970 41.2105 34.7222 -35.7891 -0.3801 -1.6336-0.99211.9487 0.0000 NA 32.3352 -37.5355 33.8784 -0.2114 -1.5432 -0.9179 1.8080 34 37, 1600 37.3714 -38.4534 0.0000 NA 35 34.5103 31.7426 -39.0488 34.6878 33.2194 -39.7597-0.1775-1.4768-0.71091.6486 0.0000 NA 36 31 2506 31 0439 -40 8656 31 3647 32 4507 -41 5013 -0 1141 -1 4068 -0.6357 1 5480 0.0000 NA 31 49.9026 35.0977 -28.1448 49.7718 36.6666 -28.8976 0.1308 -1.5689 -0.7528 A-PILLAR Lateral (Y) 46.2667 34.2480 -30.7060 46.7859 35.6950 -32.2285 -0.5192 -1.4470 -1.5225 2.1637 -1.4470 33 40.8304 33.0886 -34.7970 41.2105 34.7222 -35.7891 -0.3801 -1.6336 -0.99211.9487 -1.6336 34 -37.5355 37.3714 33.8784 37.1600 32.3352 -38.4534 -0.2114 -1.5432 -0.9179 1.8080 -1.5432 -39.0488 1.6486 35 34.5103 31.7426 34.6878 33.2194 -39.7597 -0.1775 -1.4768-0.7109-1.476836 31.2506 -0.6357 1.5480 31.0439 -40.865631.3647 32.4507 -41.5013-0.1141-1.4068B-PILLAR Maximum (X, Y, Z) 37 6.5691 32.0138 -37.4573 6.5083 32.9525 -37.2013 0.0608 -0.9387 0.2560 0.9749 0.2631 X, Z 4.1818 33.6409 -33.2671 34.4374 -32.9321 0.0323 -0.7965 0.3350 0.8647 0.3366 X, Z 34.6773 -29.3280 7.5224 35.4340 -28.9888 -0.7567 0.3392 0.8317 40 4.9225 4.8835 35.8524 0.0390 -0.5961 0.3223 0.6788 0.3247 35.2563 -24.7826 -24.4603 37 6.5691 32.0138 -37.4573 6.5083 32.9525 -37.2013 0.0608 -0.9387 0.2560 0.9749 -0.9387 Υ **B-PILLAR** 3 38 4 1818 33 6409 -33.2671 4 1495 34 4374 -32 9321 0.0323 -0.79650.3350 0.8647 -0.7965ateral-39 7.5861 34.6773 -29.3280 7.5224 35.4340 -28.9888 0.0637 -0.7567 0.3392 0.8317 -0.7567 40 4.9225 35.2563 -24.7826 4.8835 35.8524 -24.4603 0.0390 -0.5961 0.3223 0.6788

Figure E-4. Occupant Compartment Deformation Data – Set 2, Test No. FLAGT-2

A Positive values denote deformation as inward toward the occupant compartment, negative values denote deformations outward away from the occupant compartment.

<sup>&</sup>lt;sup>B</sup> Crush calculations that use multiple directional components will disregard components that are negative and only include positive values where the component is deforming inward toward the occupant compartment.

<sup>&</sup>lt;sup>C</sup> Direction for Crush column denotes which directions are included in the crush calculations. If "NA" then no intrusion is recorded, and Crush will be 0.

			Test Name:	FLAGT-2	VIN:	1C6RR6GG	1FS646171
Model Year:	<del>-</del>	Make:	Dodge	Model:	Ram 1500		
		Pa	assenger Side Ma	ximum Deformation			
	Reference Se				Reference Se	t 2	
Location	Maximum Deformation <sup>A,B</sup> (in.)	MASH Allowable Deformation (in.)	Directions of Deformation <sup>C</sup>	Location	Maximum Deformation <sup>A,B</sup> (in.)	MASH Allowable Deformation (in.)	Directions of Deformation <sup>C</sup>
Roof	-1.0	≤ 4	Z	Roof	0.3	≤ 4	Z
Windshield <sup>D</sup>	2.5	≤ 3	X, Z	Windshield <sup>D</sup>	NA	≤ 3	X, Z
A-Pillar Maximum	0.1	≤ 5	X	A-Pillar Maximum	0.1	≤ 5	X
A-Pillar Lateral	-0.8	≤ 3	Υ	A-Pillar Lateral	-1.6	≤ 3	Υ
B-Pillar Maximum	0.0	≤ 5	Χ	B-Pillar Maximum	0.3	≤ 5	X, Z
B-Pillar Lateral	-0.1	≤ 3	Υ	B-Pillar Lateral	-0.9	≤ 3	Υ
Toe Pan - Wheel Well	9.9	≤ 9	X, Z	Toe Pan - Wheel Well	9.5	≤ 9	X, Z
Side Front Panel	8.1	≤ 12	Υ	Side Front Panel	7.8	≤ 12	Υ
Side Door (above seat)	-5.0	≤ 9	Υ	Side Door (above seat)	-5.4	≤ 9	Υ
Side Door (below seat)	1.2	≤ 12	Υ	Side Door (below seat)	1.0	≤ 12	Υ
Floor Pan	6.5	≤ 12	Z	Floor Pan	5.9	≤ 12	Z
Dash - no MASH requirement	4.3	NA	X, Y, Z	Dash - no MASH requirement	3.6	NA	X, Y, Z
<sup>C</sup> For Toe Pan - Wheel Well the c and Z directions. The direction c intruding into the occupant comp	ation as inward tovalirection of defromof deformation for contraction for the contraction for the contraction for the contraction for the contraction at the contraction and the contraction are the contraction at the contraction are the contraction at the contraction are the contraction as the contraction are the contraction as the contraction are the contraction as the contraction are the contraction ar	ward the occupant o ation may include > Toe Pan -Wheel We on of deformation is	compartment, negati Kand Z direction. Fo ell, A-Pillar Maximum s "NA" then no intrus	ve values denote deformations out r A-Pillar Maximum and B-Pillar Max , and B-Pillar Maximum only include ion is recorded and deformation wi sured posttest with an examplar veh	kimum the direction components who ll be 0.	n of deformation matere the deformation	ay include X, Y, is positive and
Notes on vehicle interior cr	ush:						

Figure E-5. Maximum Occupant Compartment Deformations by Location, Test No. FLAGT-2

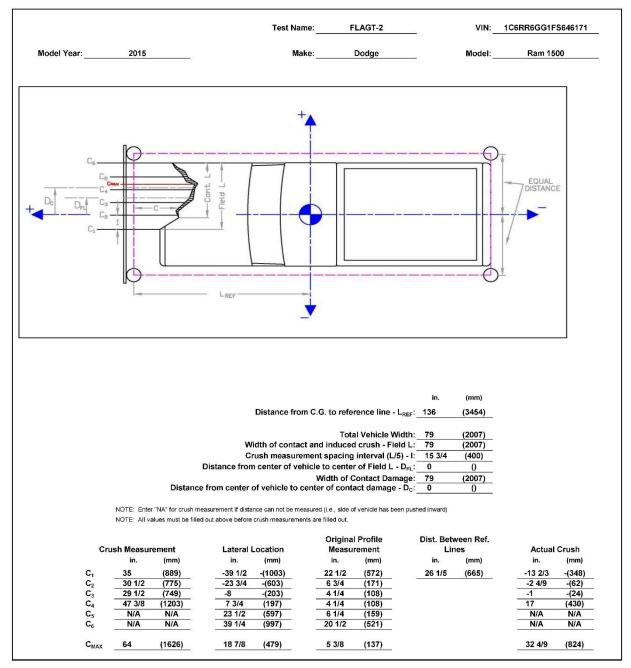


Figure E-6. Exterior Vehicle Crush (NASS) - Front, Test No. FLAGT-2

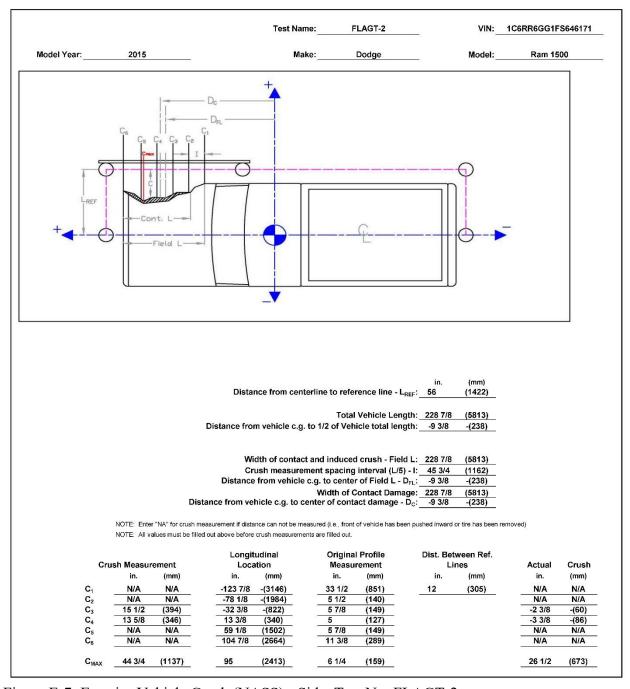


Figure E-7. Exterior Vehicle Crush (NASS) - Side, Test No. FLAGT-2

### Appendix F. Accelerometer and Rate Transducer Data Plots, Test No. FLAGT-2

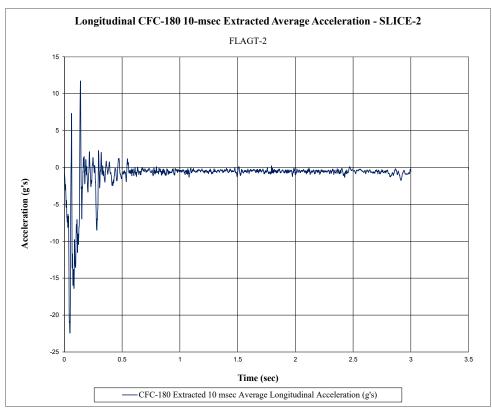


Figure F-1. 10-ms Average Longitudinal Deceleration (SLICE-2), Test No. FLAGT-2

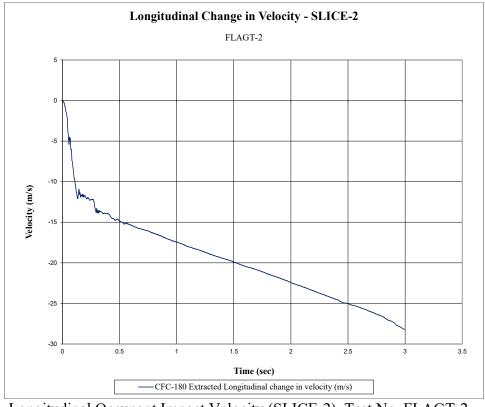


Figure F-2. Longitudinal Occupant Impact Velocity (SLICE-2), Test No. FLAGT-2

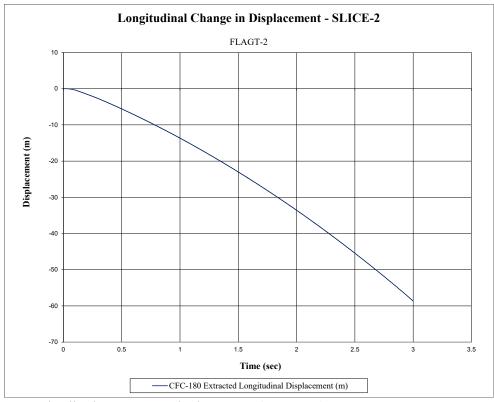


Figure F-3. Longitudinal Occupant Displacement (SLICE-2), Test No. FLAGT-2

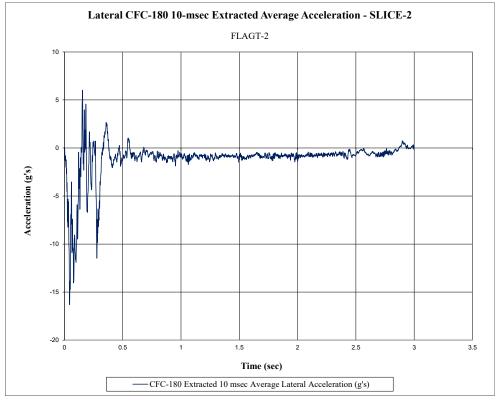


Figure F-4. 10-ms Average Lateral Deceleration (SLICE-2), Test No. FLAGT-2

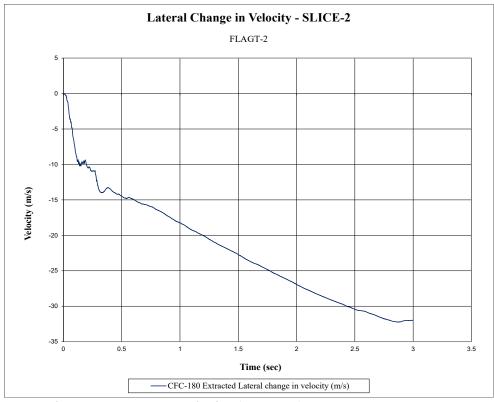


Figure F-5. Lateral Occupant Impact Velocity (SLICE-2), Test No. FLAGT-2

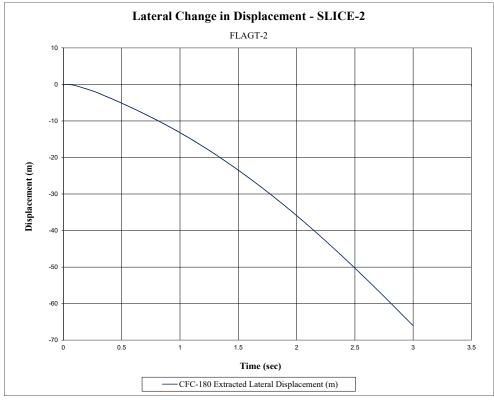


Figure F-6. Lateral Occupant Displacement (SLICE-2), Test No. FLAGT-2

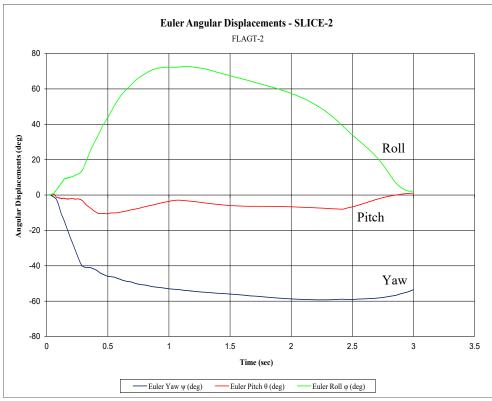


Figure F-7. Vehicle Angular Displacements (SLICE-2), Test No. FLAGT-2

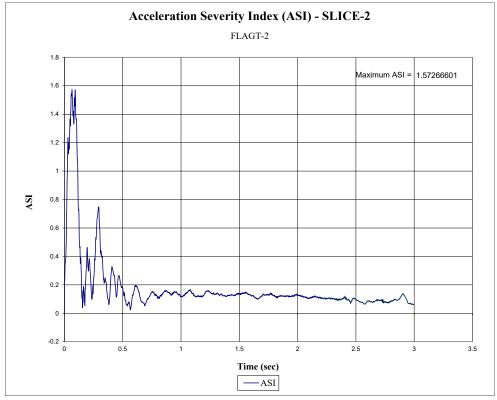


Figure F-8. Acceleration Severity Index (SLICE-2), Test No. FLAGT-2

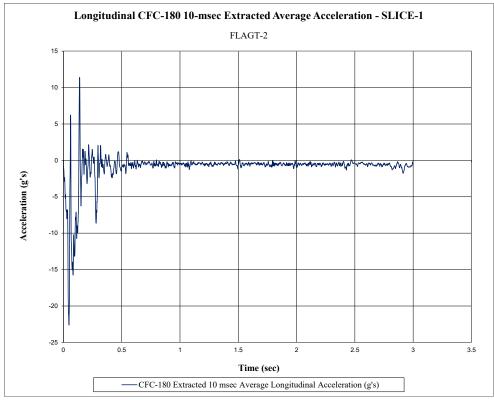


Figure F-9. 10-ms Average Longitudinal Deceleration (SLICE-1), Test No. FLAGT-2

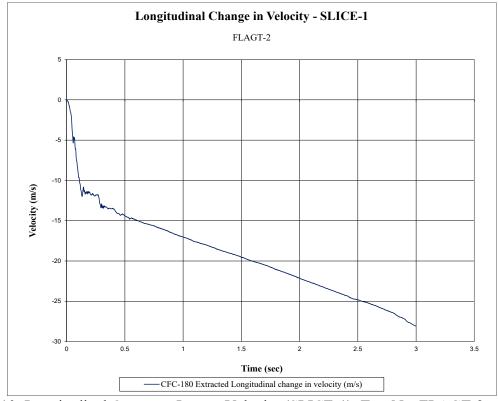


Figure F-10. Longitudinal Occupant Impact Velocity (SLICE-1), Test No. FLAGT-2

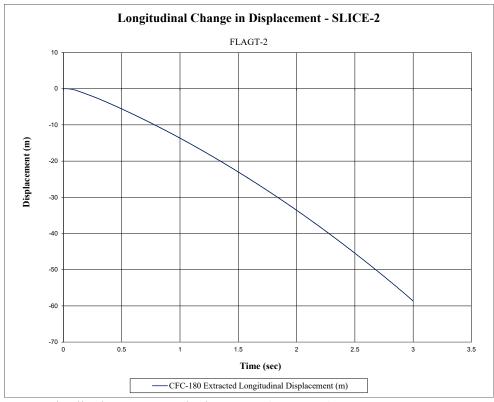


Figure F-11. Longitudinal Occupant Displacement (SLICE-2), Test No. FLAGT-2

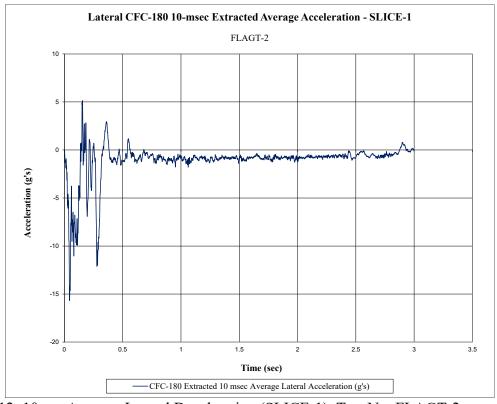


Figure F-12. 10-ms Average Lateral Deceleration (SLICE-1), Test No. FLAGT-2

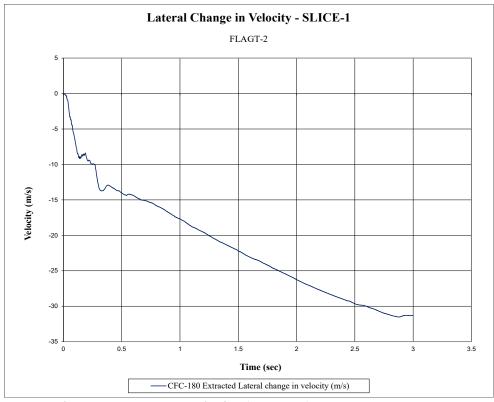


Figure F-13. Lateral Occupant Impact Velocity (SLICE-1), Test No. FLAGT-2

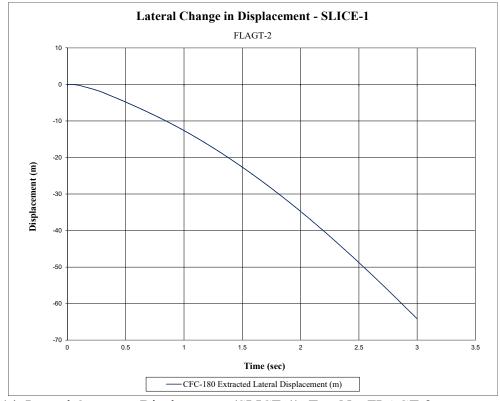


Figure F-14. Lateral Occupant Displacement (SLICE-1), Test No. FLAGT-2

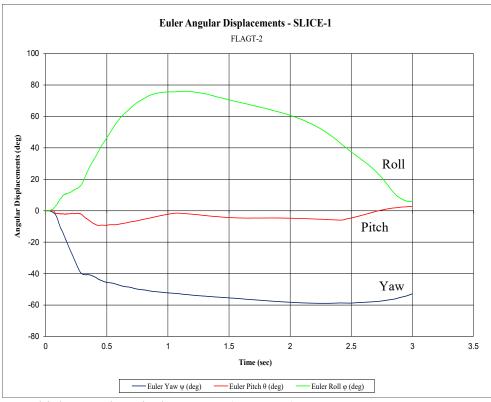


Figure F-15. Vehicle Angular Displacements (SLICE-1), Test No. FLAGT-2

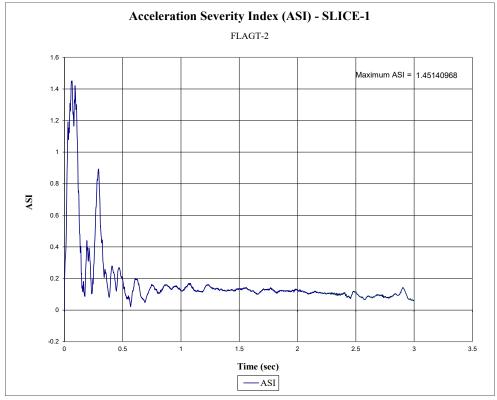


Figure F-16. Acceleration Severity Index (SLICE-1), Test No. FLAGT-2

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