





DYNAMIC EVALUATION OF A PINNED ANCHORING SYSTEM FOR NEW YORK STATE'S TEMPORARY CONCRETE BARRIERS – PHASE II

Submitted by

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16. Abstract (Limit: 200 words)

Temporary concrete barrier (TCB) systems are utilized in many situations, including placement adjacent to vertical drop-offs. Free-standing TCB systems are known to have relatively large deflections when impacted, which may be undesirable when dealing with limited space behind the barrier, such as on a bridge deck or with limited lane width in front of the barrier system. In order to allow TCB systems to be used in space-restricted locations, a variety of TCB stiffening options have been tested, including beam stiffening and pinning the barriers to the pavement. These pavement-pinning procedures have been considered time-consuming and may pose undue risk to work-zone personnel who are anchoring the barrier on the traffic-side face. Thus, a means of reducing TCB deflections while reducing risk to workers was deemed necessary.

The primary research objectives were to evaluate the potential for reducing barrier deflections through the use of pinning every barrier section on the back-side toe of the New York State's New Jersey-shape TCBs and evaluate the barrier system according to the Test Level 3 (TL-3) criteria set forth in the *Manual for Assessing Safety Hardware* (MASH). The research study included one full-scale vehicle crash test with a Dodge Quad Cab pickup truck. Four 15½-in. (394-mm) long, vertical steel pins were placed through holes on the back-side toe of each barrier section and inserted into drilled holes within the rigid concrete surface. Following the successful redirection of the pickup truck, the safety performance of the pinned anchoring system was determined to be acceptable according to the TL-3 evaluation criteria specified in MASH using the 2270P vehicle.

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UNCERTAINTY OF MEASUREMENT STATEMENT

The Midwest Roadside Safety Facility (MwRSF) has determined the uncertainty of measurements for several parameters involved in standard full-scale crash testing and non-standard testing of roadside safety features. Information regarding the uncertainty of measurements for critical parameters is available upon request by the sponsor and the Federal Highway Administration.

The Independent Approving Authority (IAA) for the data contained herein was Dr. Ronald K. Faller, Research Assistant Professor.

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(1) the New York State Department of Transportation for sponsoring this project; and (2) MwRSF personnel for constructing the barrier and conducting the crash test.

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1 INTRODUCTION

1.1 Problem Statement

Over the years, temporary concrete barrier (TCB) systems have been utilized to serve several functions. These include preventing motorists from encroaching into the work space within work zones; providing positive protection for construction and maintenance workers as well as the associated operations; separating two-way or opposing traffic; shielding vehicles from roadside and median hazards; and separating pedestrians and bicyclists from vehicle traffic.

In general, these temporary concrete barrier (TCB) systems are used in free-standing configurations where the base of each barrier segment is placed on a paved surface and without physical attachment to that surface. Under high-energy impact scenarios, these free-standing, linked concrete barrier segments can be displaced laterally for several feet, thus allowing the barrier system to encroach into the work space. For narrow work areas, this barrier displacement could potentially result in workers being crushed between the barrier system and objects located within the workspace. Free-standing barrier systems are also often used to shield vertical dropoffs, such as excavations adjacent to the roadway or at the exposed edge of a bridge deck. If TCBs are placed too close to a vertical drop-off and are displaced laterally, there exists a potential for the barrier system to fall onto workers in an excavation or below a bridge, or even onto traffic under the bridge.

Various anchorage and stiffening systems have been incorporated into selected TCB systems in order to reduce barrier deflections and allow their use in restricted work zones with confined space behind the barrier system and/or limited lane width in front of the barrier system. Some of these systems have included the use of stiffening beams placed on the back side of the barriers and across the joints, the placement of vertical pins or rods through either the front toe or

both toes of the barrier and into the pavement or bridge deck surface, as well as the use of an anchorage system that connects the joint hardware to the deck surface. Although various vertical pinning methods have been successfully developed for anchoring TCBs to paved surfaces, including concrete bridge decks, the installation process is often considered to be time-consuming, costly, and ultimately may result in damage to the bridge deck.

Over the last decade, the New York State Department of Transportation (NYSDOT) sponsored considerable research for the crash testing and evaluation of a New Jersey-shape temporary concrete barrier system used in both free-standing and stiffened configurations and with the barrier ends both pinned and unpinned to the pavement surface [1-4]. These crash testing programs were conducted according to the Test Level 3 (TL-3) safety performance criteria found in either the National Cooperative Highway Research Program (NCHRP) Report No. 350 [5] or the Manual for Assessing Safety Hardware (MASH) [6].

For TCBs located adjacent to vertical drop-offs, the NYSDOT has deemed it desirable to utilize vertical pins through the back-side toe in order to reduce barrier deflections as well as to reduce the need for workers to be positioned on the traffic-side face of the TCBs while anchoring the barrier segments. One of the aforementioned research programs explored the idea of the pinned, temporary concrete barrier system with <u>alternating</u> segments anchored to the rigid concrete surface with vertical steel pins placed through the back-side toe of New York State's New Jersey-shape TCB and set into drilled holes in the concrete surface [4]. However, significant barrier deflections were observed during the crash test and may be greater than desired for work areas with restricted space.

As a result, there still remained a need for determining whether the NYSDOT's New Jersey-shape, temporary concrete barrier system could be pinned throughout its length and only on the back-side face of the barrier. In order to reduce construction costs and damage to bridge decks, an investigation was undertaken to evaluate whether barrier deflections would be maintained to reasonable levels with vertical pins placed in every barrier segment.

1.2 Research Objective

The objectives of this research project were to evaluate the deflection performance and safety performance of a pinned option of NYSDOT's New Jersey-shape, temporary concrete barrier system. The NYSDOT officials were confident that the barrier system would meet all of the impact safety standards. However, the NYSDOT wanted to evaluate the TCB system using the TL-3 safety performance criteria set forth in MASH, which results in a 13.5 percent increase in impact severity over that provided in NCHRP Report No. 350.

1.3 Scope

The research objective was achieved through the completion of several tasks. First, a full-scale vehicle crash test was performed on the pinned temporary concrete barrier system with every segment anchored to the pavement surface. The crash test utilized a pickup truck, weighing approximately 5,000 lb (2,268 kg), as recommended in MASH guidelines [6]. The target impact conditions for the test were an impact speed of 62 mph (100 km/h) and an impact angle of 25 degrees. Next, the test results were analyzed, evaluated, and documented. Finally, conclusions and recommendations were made that pertain to the safety performance of the pinned temporary concrete barrier system.

2 LITERATURE REVIEW

Previous testing on the NYSDOT TCB system was conducted by the Texas Transportation Institute (TTI) and the Midwest Roadside Safety Facility (MwRSF) [1-4]. At TTI, the TCB system was evaluated according to the criteria provided in NCHRP Report No. 350 [5], while at MwRSF the TCB system was evaluated according to the criteria presented in MASH [6].

In 1999, TTI tested a free-standing version of the NYSDOT TCB with unpinned ends [1]. In test no. 473220-7, a 4,575-lb (2,075-kg) pickup truck impacted the ten barrier system 3 ft - 11 in. (1.2 m) upstream of the joint between barrier segment nos. 3 and 4 at a speed of 60.9 mph (98.0 km/h) and at an angle of 26.3 degrees. During the impact, three of the barrier joints failed, causing the barrier at the point of impact to overturn. Subsequently, the vehicle overrode the barrier and rolled over. Thus, the test was determined to be unacceptable according to the NCHRP Report No. 350 requirements, since the vehicle did not remain upright after collision with the system. The joint failure was subsequently attributed to substandard welding in the connection joints.

In 2001, TTI retested the properly fabricated unpinned NYSDOT TCB system [2]. It should be noted that the end barrier sections were unpinned as well. In test no. 473220-14, a 4,577-lb (2,076-kg) pickup truck impacted the ten barrier system 4 ft - 6 in. (1.4 m) upstream of the joint between barrier segment nos. 3 and 4 at a speed of 62.6 mph (100.8 km/h) and at an angle of 25.6 degrees. During the impact, the vehicle was redirected smoothly, and the test was determined to be acceptable according to the NCHRP Report No. 350 requirements. The barrier system experienced 50 in. (1,270 mm) of dynamic deflection and 50 in. (1,270 mm) of permanent set deflection. During the test, the upstream end was pulled 5 13/16 in. (148 mm)

longitudinally downstream, while the downstream end was displaced 3/16 in. (5 mm) longitudinally upstream, or toward the impact point. The noted lateral barrier deflections would be correlated to the unpinned section ends. It was NYSDOT's concern over this large barrier deflection that caused the state agency to contract with MwRSF to conduct the barrier stiffening research noted below.

In 2008, MwRSF crash tested three different versions of NYSDOT's TCB system [3]. The research study included three full-scale vehicle crash tests with 2270P pickup trucks conducted in accordance to the TL-3 evaluation criteria published in MASH. In all three tests, the first and last barrier sections were anchored to the concrete surface.

The first test, test no. NYTCB-1, consisted of stiffening three joints between barrier nos. 4 and 7 with 6-in. x 6-in. x 3/16-in. (152-mm x 152-mm x 4.8-mm) box beam sections. In this test, a 5,016-lb (2,275-kg) pickup truck impacted the ten barrier system essentially at the target location, which was 4 ft - 3 3/16 in. (1.3 m) upstream from the downstream end of barrier no. 4 at a speed of 61.8 mph (99.5 km/h) and at an angle of 24.6 degrees. During the impact, the vehicle was redirected smoothly, and the test was determined to be acceptable according to MASH requirements. The barrier system with anchored ends experienced 27 5/8 in. (700 mm) of dynamic deflection and 26 in. (660 mm) of permanent set deflection.

The second test, test no. NYTCB-2, consisted of an unstiffened version of the NYSDOT TCB system with anchored ends. In this test, a 5,024-lb (2,279-kg) pickup truck impacted the ten barrier system essentially at the target location, which was 4 ft - 3 3/16 in. (1.3 m) upstream from the downstream end of barrier no. 4 at a speed of 61.2 mph (98.5 km/h) and at an angle of 25.8 degrees. During the impact, the vehicle was redirected smoothly, and the test was determined to be acceptable according to MASH requirements. The barrier system with anchored ends

experienced 40 5/16 in. (1,023 mm) of dynamic deflection and 39½ in. (1,003 mm) of permanent set deflection.

The third test, test no. NYTCB-3, consisted of stiffening six joints between barrier nos. 2 and 8 with 6-in. x 8-in. x ¼-in. (152-mm x 203-mm x 6.4-mm) box beam sections. In addition, this system was installed with the back side of the barrier sections placed 12 in. (305 mm) away from a simulated bridge deck edge. In this test, a 5,001-lb (2,268-kg) pickup truck impacted the ten barrier system essentially at the target location, which was 4 ft - 3 3/16 in. (1.3 m) upstream from the downstream end of barrier no. 4, at a speed of 63.5 mph (102.2 km/h) and at an angle of 24.4 degrees. During the test, the vehicle was redirected smoothly, and the test was determined to be acceptable according to MASH requirements. The barrier system with anchored ends experienced 30% in. (784 mm) of dynamic deflection and 26 in. (660 mm) of permanent set deflection.

In 2009, MwRSF crash tested a pinned anchoring system for NYSDOT's TCB system [4]. The barrier system consisted of NYSDOT's TCB system with every other barrier pinned to the concrete surface with steel vertical rods placed through the back-side toe of the barrier section and set into drilled holes in the rigid concrete surface. In test no. NYTCB-4, a 5,172-lb (2,346-kg) pickup truck impacted the ten barrier system essentially at the target location, which was 4 ft - 3 3/16 in. (1.3 m) upstream from the joint between barrier nos. 4 and 5, at a speed of 62.3 mph (100.3 km/h) and at an angle of 24.3 degrees. During the impact and due to the vehicle's tail slap against the barrier, the joint between barrier nos. 4 and 5 completely separated at approximately the same time that the vehicle exited the barrier system. The barrier system experienced 64.8 in. (1,646 mm) of dynamic deflection and 53½ in. (1,359 mm) of permanent set deflection. The additional dynamic deflection was the result of the joint separation. However, the

vehicle was contained and smoothly redirected. Although complete joint separation occurred and is generally undesirable, the test was determined to be acceptable according to MASH requirements.

3 DESIGN DETAILS

The test installation was 200 ft (61.0 m) long and comprised of ten temporary concrete barrier sections in an anchored configuration, as shown in Figures 1 through 9. The 20-ft (6.1-m) long, temporary concrete barrier sections were placed on the rigid concrete surface with all sections attached to the concrete surface. Photographs of the test installation are shown in Figures 10 through 12. Material specifications, mill certifications, and/or certificates of conformity for the system materials are shown in Appendix A.

The concrete used for the barrier sections consisted of a concrete mix with a minimum 28-day compressive strength of 3,000 psi (21.0 MPa). A minimum concrete cover of 1½ in. (38 mm) was used along all rebar in the barrier. All of the steel reinforcement in the barrier was ASTM A615 Grade 60 rebar and consisted of four No. 6 longitudinal bars, eight No. 4 bars for the vertical stirrups, four No. 6 lateral bars, and nine No. 4 bars for the anchor hole reinforcement loops. The section reinforcement details are shown in Figures 3 and 8.

The barrier sections used a connection key, as shown in Figures 4 through 7, 10, and 11. The connection key assembly consisted of ½-in. (13-mm) thick, ASTM A36 steel plates welded together to form the key shape. Two stiffeners were welded to the top plate with their interior faces in contact with the I-beam shape and located 5/16 in. (8 mm) up from the ends of the top plate, as shown in Figures 4 and 5.

A connector key was configured at each end of the barrier section, as shown in Figure 3, 10, and 11. The connector key consisted of one ASTM A500 steel tube and three ASTM A36 steel plates. Three U-shaped plates were welded on the sides of the tube, as shown in Figure 6. A connection key was inserted into the steel tubes of two adjoining sections to form the connection, as shown in Figure 7. The connector key provides a stout connection that is particularly effective

at distributing the system's torsional twist about the longitudinal axis to the adjoining sections of TCB. This combined resistance is essential in order to minimize the barrier rotation sustained by the impacted section.

All barrier sections were pinned to the rigid concrete pavement with four 1-in. (25-mm) diameter by 15½-in. (394-mm) long, ASTM A36 (hot rolled) steel rods. The steel rods or pins were passed through the precast holes on the back-side toe of the barrier sections, as shown in Figures 1, 2, and 12. Each anchor rod was then inserted into a 1½-in. (29-mm) diameter, drilled hole in the rigid concrete surface using an embedment depth of 5 in. (127 mm), as shown in Figure 1. In addition, the system was installed with the back side of the sections placed 12 in. (305 mm) away from the edge of a simulated bridge deck, as shown in Figure 1.

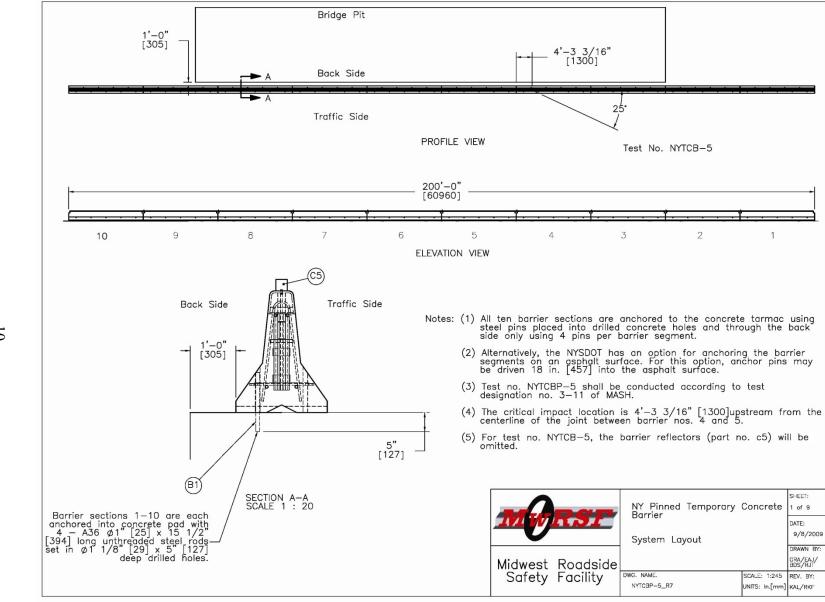


Figure 1. Test Installation Layout, Test No. NYTCB-5

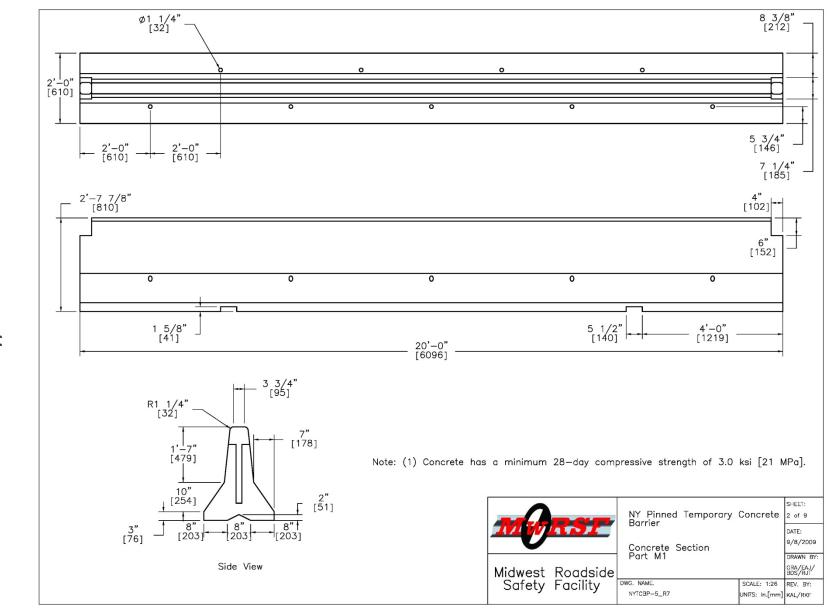


Figure 2. Temporary Concrete Barrier Details, Test No. NYTCB-5

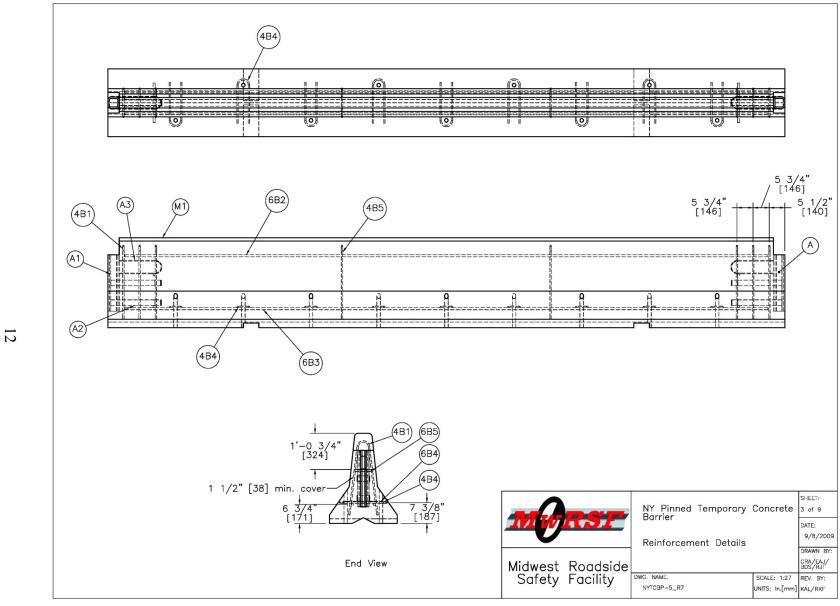


Figure 3. Temporary Concrete Barrier Reinforcement Details, Test No. NYTCB-5

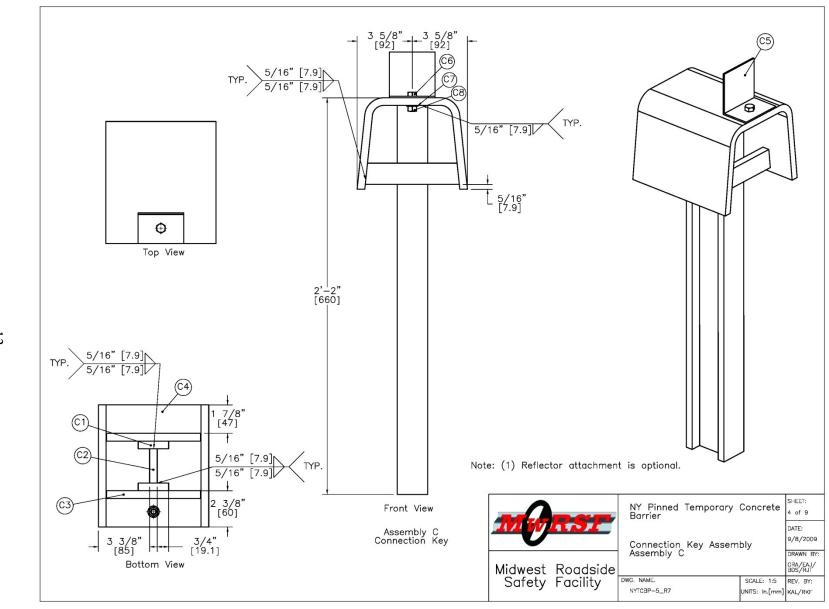


Figure 4. Connection Key Assembly Details, Test No. NYTCB-5

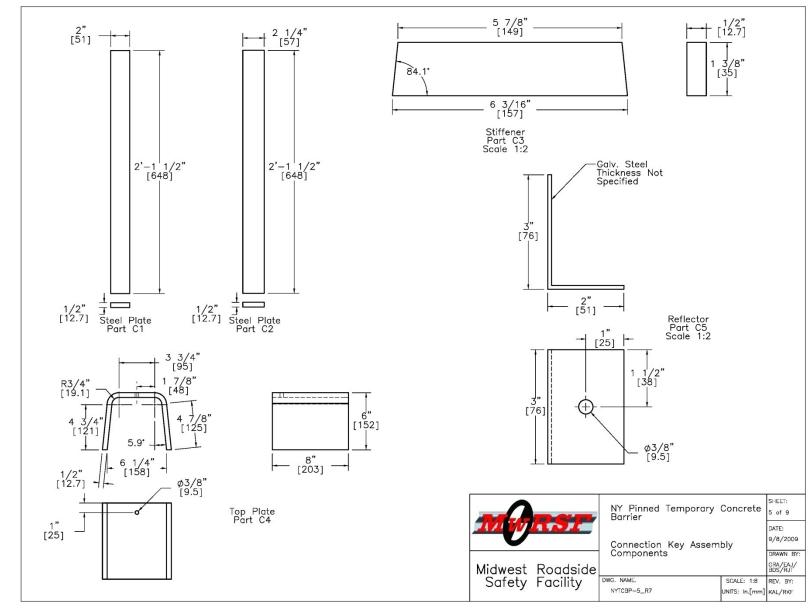


Figure 5. Connection Key Assembly Details, Test No. NYTCB-5

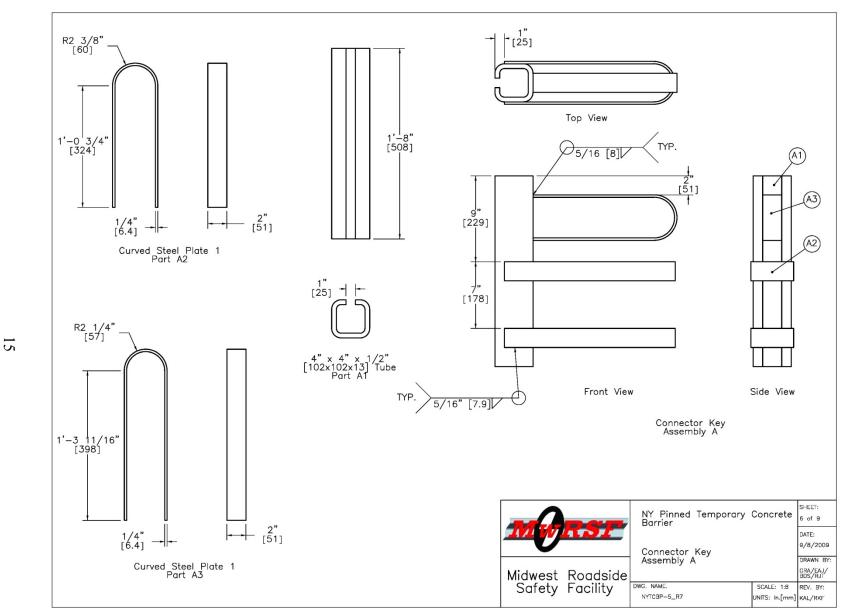


Figure 6. Temporary Concrete Barrier Connector Assembly Detatils, Test No. NYTCB-5

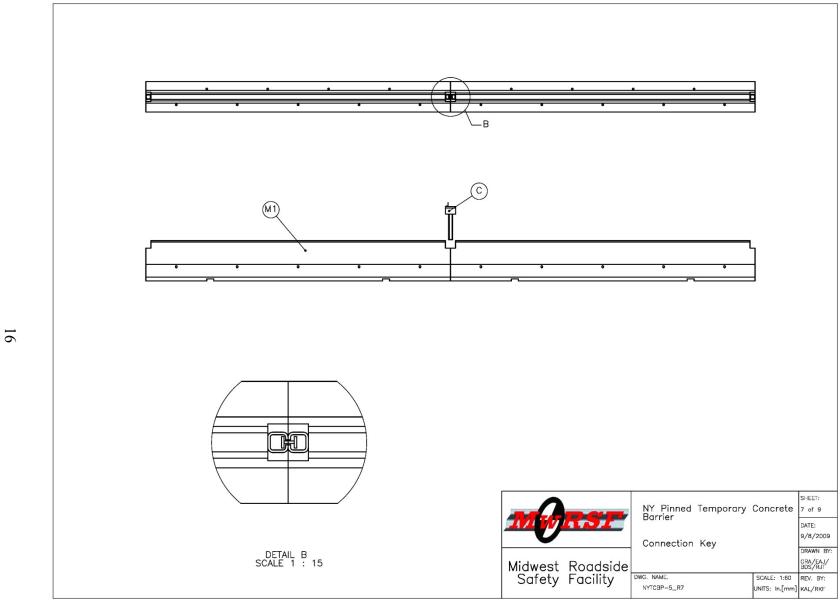


Figure 7. Temporary Concrete Barrier Connection Details, Test No. NYTCB-5

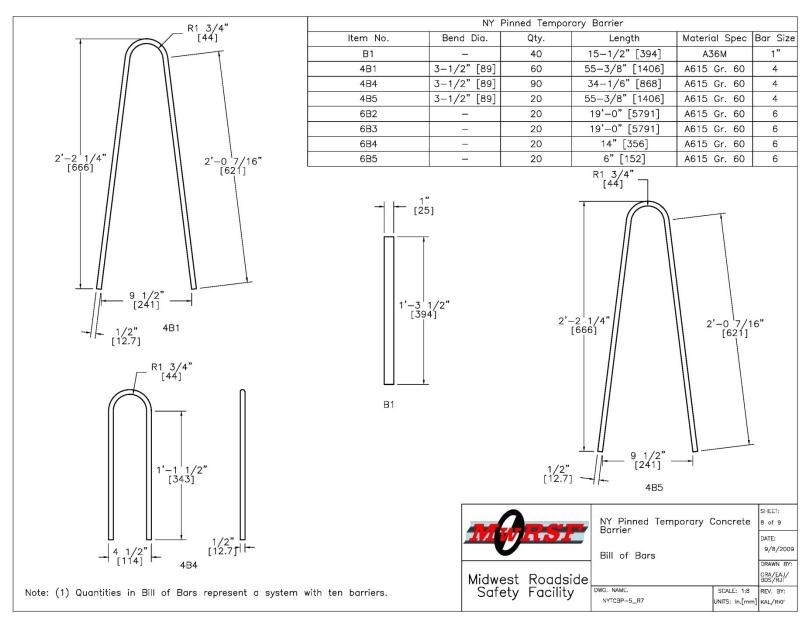


Figure 8. Bill of Bars, Test No. NYTCB-5

Item No.	QTY.	Description	Material Specification	
М1	10	Concrete Section	f'c = 3,000 psi [21 MPa]	
4B1	60	Rebar 4B1	ASTM A615 Gr. 60	
4B4	90	Rebar 4B4	ASTM A615 Gr. 60	
4B5	20	Rebar 4B5	ASTM A615 Gr. 60	
6B2	20	Rebar 6B2	ASTM A615 Gr. 60	
6B3	20	Rebar 6B3	ASTM A615 Gr. 60	
6B4	20	Rebar 6B4	ASTM A615 Gr. 60	
6B5	20	Rebar 6B5	ASTM A615 Gr. 60	
A1	20	Steel Tube - 4"x4"x1/2" [102x102x12.7] thick x 20" [508] long	ASTM A500 Gr. B or C	
A2	40	Steel Plate	ASTM A36	
A3	20	Steel Plate	ASTM A36	
B1	40	Anchor Rod	ASTM A36M	
C1	18	Steel Plate	ASTM A36	
C2	9	Steel Plate	ASTM A36	
С3	18	Steel Plate-1/2" [12.7]	ASTM A36	
C4	9	Steel Plate- 1/2" [12.7]	ASTM A36	
C5	9	Steel Plate	ASTM A36	
C6	9	Ø7/16"-17 x 1" [M10 x 25] long bolt	ASTM A325	
C7	9	7/16" [11.1] Dia. Lock Washer	ASTM F436 Gr. 1	
C8	9	7/16" [11.1] Dia. Hex Nut	ASTM A563	
		Midwest Roadside Safety Facility	NY Pinned Temporary Concrete Barrier Bill of Materials DWG. NAME. NYTCBP-5_R7 SHEET: 9 of 9 DATE: 9/8/2 DRAWN GRA/BA SCALE: None REV. B	

Figure 9. Bill of Materials, Test No. NYTCB-5







Figure 10. Pinned Temporary Concrete Barrier Test Installation, Test No. NYTCB-5









Figure 11. Temporary Concrete Barrier Sections and Connection Key, Test No. NYTCB-5





Figure 12. Temporary Concrete Barrier Section Anchor, Test No. NYTCB-5

4 TEST REQUIREMENTS AND EVALUATION CRITERIA

4.1 Test Requirements

Longitudinal barriers, such as temporary concrete barriers, must satisfy impact safety standards in order to be accepted by the Federal Highway Administration (FHWA) for use on National Highway System (NHS) new construction projects or as a replacement for existing designs not meeting current safety standards. In recent years, these safety standards have consisted of the guidelines and procedures published in NCHRP Report No. 350 [1]. However, NCHRP Project 22-14(2) generated revised testing procedures and guidelines for use in the evaluation of roadside safety appurtenances and are provided in MASH [6]. According to TL-3 of MASH, longitudinal barrier systems must be subjected to two full-scale vehicle crash tests. The two full-scale crash tests are as follows:

- 1. Test Designation 3-10 consisting of a 2,425-lb (1,100-kg) passenger car impacting the system at a nominal speed and angle of 62 mph (100 km/h) and 25 degrees, respectively.
- 2. Test Designation 3-11 consisting of a 5,000-lb (2,268-kg) pickup truck impacting the system at a nominal speed and angle of 62 mph (100 km/h) and 25 degrees, respectively.

A rigid, F-shape bridge rail was successfully impacted by a small car weighing 1,800 lb (893 kg) at 60.1 mph (96.7 km/h) and 21.4 degrees according to the American Association of State Highway and Transportation Officials (AASHTO) *Guide Specifications for Bridge Railings* [7-8]. In the same manner, rigid New Jersey safety shape barriers struck by small cars have also been shown to meet safety performance standards [9-10]. In addition, a New Jersey safety shape barrier was impacted by a passenger car weighing 2,579 lb (1,170 kg) at 60.8 mph (97.9 km/h) and 26.1 degrees according to the TL-3 standards set forth in MASH [11]. Furthermore, temporary New Jersey safety shape concrete median barriers have experienced only slight barrier

deflections when impacted by small cars and behave similar to rigid barriers [12]. Thus, if the NYSDOT's pinned TCB system does not exhibit significant roll when subjected to the pickup truck impact condition, then it may not be necessary to conduct the 2,425-lb (1,100-kg) passenger car test due to expectations for only minor barrier rotations. However, if the pickup truck impact into the barrier system induces significant barrier rotations, then it also may be necessary to conduct the passenger car test in order to evaluate the propensity for vehicular instabilities upon redirection. The test conditions of TL-3 longitudinal barriers are summarized in Table 1.

For this crash testing program, the NYSDOT's primary objective was to evaluate the deflection performance of a pinned version of the NYSDOT's TCB system when subjected to high-speed, high-energy, pickup truck impacts.

Table 1. MASH TL-3 Crash Test Conditions

		Test Vehicle	Impact Conditions			
Test Article	Test Designation		Speed		Angle	Evaluation Criteria ¹
THUICIC	Designation ,		mph	km/h	(deg)	
Longitudinal Barrier	3-10	1100C	62	100	25	A,D,F,H,I
	3-11	2270P	62	100	25	A,D,F,H,I

¹ Evaluation criteria explained in Table 2.

4.2 Evaluation Criteria

Evaluation criteria for full-scale vehicle crash testing are based on three appraisal areas:

(1) structural adequacy; (2) occupant risk; and (3) vehicle trajectory after collision. Criteria for structural adequacy are intended to evaluate the ability of the barrier to contain and redirect impacting vehicles. Occupant risk evaluates the degree of hazard to occupants in the impacting

vehicle. Vehicle trajectory after collision is a measure of the potential for the post-impact trajectory of the vehicle to result in secondary collisions with other vehicles and/or fixed objects, thereby increasing the risk of injury to the occupants of the impacting vehicle and other vehicles. These evaluation criteria are summarized in Table 2 and defined in greater detail in MASH. The full-scale vehicle crash test was conducted and reported in accordance with the procedures provided in MASH.

In addition to the standard occupant risk measures, the Post-Impact Head Deceleration (PHD), the Theoretical Head Impact Velocity (THIV), and the Acceleration Severity Index (ASI) were determined and reported on the test summary sheet. Additional discussion on PHD, THIV and ASI is provided in Reference 6.

Table 2. MASH Evaluation Criteria for Longitudinal Barrier

Structural Adequacy	A.	Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.					
	D.	Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH.					
	F.	The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.					
Occupant Risk	Н.	Occupant Impact Velocities (OIV) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits:					
		Occupant 1	Impact Velocity Lim	its			
	Component Preferred Maximum						
		Longitudinal and Lateral	30 ft/s (9.1 m/s)	40 ft/s (12.2 m/s)			
	I.	The Occupant Ridedown Accelerations (ORA) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits:					
	Occupant Ridedown Acceleration Limits						
		Component	Preferred	Maximum			
		Longitudinal and Lateral 15.0 g's 20.49 g's					

5 TEST CONDITIONS

5.1 Test Facility

The testing facility is located at the Lincoln Air Park on the northwest side of the Lincoln Municipal Airport and is approximately 5 miles (8.0 km) northwest of the University of Nebraska-Lincoln.

5.2 Vehicle Tow and Guidance System

A reverse cable tow system with a 1:2 mechanical advantage was used to propel the test vehicle. The distance traveled and the speed of the tow vehicle were one-half that of the test vehicle. The test vehicle was released from the tow cable before impact with the barrier system. A digital speedometer on the tow vehicle increased the accuracy of the test vehicle impact speed.

A vehicle guidance system developed by Hinch [13] was used to steer the test vehicle. A guide-flag, attached to the right-front wheel and the guide cable, was sheared off before impact with the barrier system. The 3/8-in. (9.5-mm) diameter guide cable was tensioned to approximately 3,500 lb (15.6 kN) and supported both laterally and vertically every 100 ft (30.48 m) by hinged stanchions. The hinged stanchions stood upright while holding up the guide cable, but as the vehicle was towed down the line, the guide-flag struck and knocked each stanchion to the ground.

5.3 Test Vehicles

For test no. NYTCB-5, a 2003 Dodge Ram 1500 Quad Cab pickup truck was used as the test vehicle. The curb, test inertial, and gross static vehicle weights were 5,109 lb (2,317 kg), 4,953 lb (2,247 kg), and 5,124 lb (2,324 kg), respectively. The test vehicle is shown in Figure 13, and vehicle dimensions are shown in Figure 14.







Figure 13. Test Vehicle, Test No. NYTCB-5

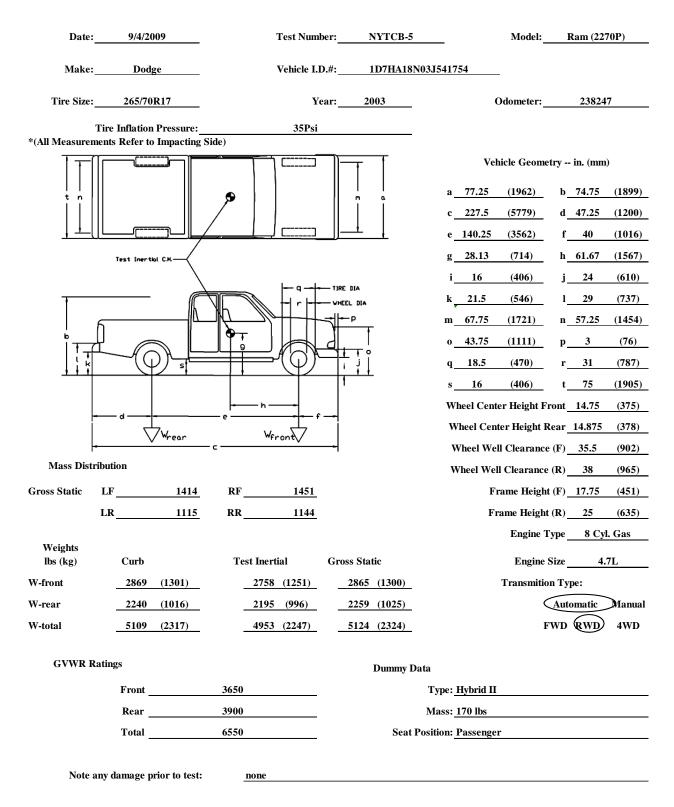


Figure 14. Vehicle Dimensions, Test No. NYTCB-5

The Suspension Method [14] was used to determine the vertical component of the center of gravity (c.g.) for the pickup truck. This method is based on the principle that the c.g. of any freely suspended body is in the vertical plane through the point of suspension. The vehicle was suspended successively in three positions, and the respective planes containing the c.g. were established. The intersection of these planes pinpointed the c.g. location. The longitudinal component of the c.g. was determined using the measured axle weights. The location of the final c.g. is shown in Figures 14 and 15. The data used for the c.g. calculations and ballast information is shown in Appendix B.

Square, black- and white-checkered targets were placed on the vehicle to aid in the analysis of the high-speed videos, as shown in Figure 15. Round, checkered targets were placed on the center of gravity on the left-side door, the right-side door, and the roof of the vehicle. The remaining targets were located for reference so that they could be viewed from the high-speed cameras for video analysis.

The front wheels of the test vehicle were aligned for camber, caster, and toe-in values of zero so that the vehicles would track properly along the guide cable. A 5B flash bulb was mounted on the left side of the vehicle's dash to pinpoint the time of impact with the barrier system on the high-speed videos. The flash bulb was fired by a pressure tape switch mounted at the impact corner of the bumper. A remote controlled brake system was installed in the test vehicle so the vehicle could be brought safely to a stop after the test.

5.4 Simulated Occupant

For test no. NYTCB-5, A Hybrid II 50th Percentile Adult Male Dummy, was placed in the right-front seat of the test vehicle with the seat belt fastened. The dummy was equipped with clothing and footwear and had a final weight of 170 lb (77 kg). The dummy was manufactured

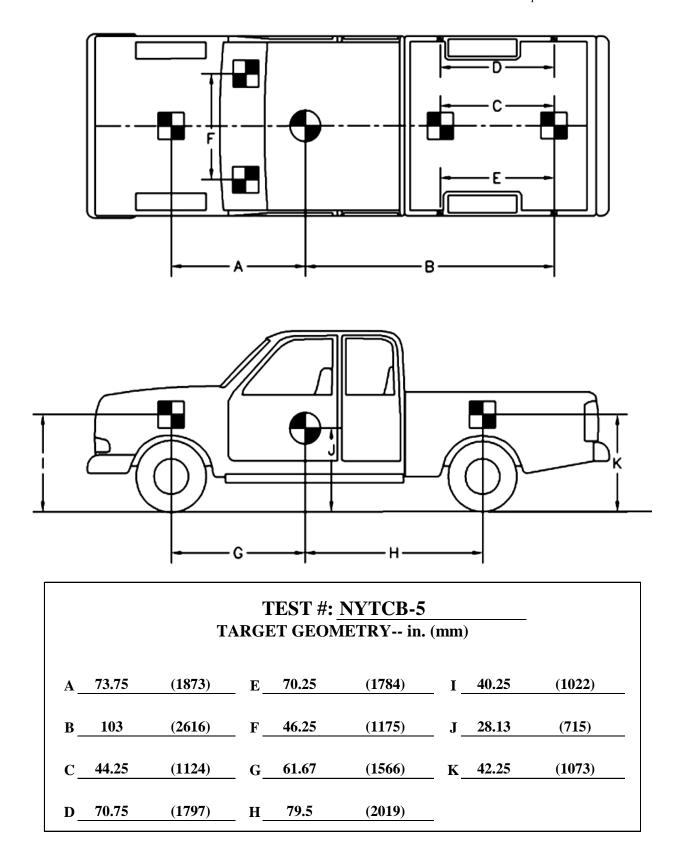


Figure 15. Target Geometry, Test No. NYTCB-5

by Android Systems of Carson, California under model no. 572 and serial no. 451. As recommended by MASH, the dummy was not included in calculating the c.g location.

5.5 Data Acquisition Systems

5.5.1 Accelerometers

Three environmental shock and vibration sensor/recorder systems were used to measure the accelerations in the longitudinal, lateral, and vertical directions. All of the accelerometers were mounted near the center of gravity of the test vehicle.

One triaxial piezoresistive accelerometer system, Model EDR-4-6DOF-500/1200, was developed and manufactured by Instrumented Sensor Technology (IST) of Okemos, Michigan and includes three differential channels as well as three single-ended channels. The EDR-4-6DOF-500/1200 was configured with 24 MB of RAM memory, a range of ±500 g's, a sample rate of 10,000 Hz, and a 1,677 Hz anti-aliasing filter. "EDR4COM" and "DynaMax Suite" computer software programs and a customized Microsoft Excel worksheet were used to analyze and plot the accelerometer data.

The second system was a two-arm piezoresistive accelerometer system developed by Endevco of San Juan Capistrano, California. Three accelerometers were used to measure each of the longitudinal, lateral, and vertical accelerations independently at a sample rate of 10,000 Hz. The accelerometers were configured and controlled using a system developed and manufactured by Diversified Technical Systems, Inc. (DTS) of Seal Beach, California. More specifically, data was collected using a DTS Sensor Input Module (SIM), Model TDAS3-SIM-16M. The SIM was configured with 16 MB SRAM memory and 8 sensor input channels with 250 kB SRAM/channel. The SIM was mounted on a TDAS3-R4 module rack. The module rack was configured with isolated power/event/communications, 10BaseT Ethernet and RS232

communication, and an internal backup battery. Both the SIM and module rack were crashworthy. The computer software program "DTS TDAS Control" and a customized Microsoft Excel worksheet were used to analyze and plot the accelerometer data.

The third system, Model EDR-3, was a triaxial piezoresistive accelerometer system developed and manufactured by IST of Okemos, Michigan. The EDR-3 was configured with 256 kB of RAM memory, a range of ±200 g's, a sample rate of 3,200 Hz, and a 1,120 Hz low-pass filter. The computer software program "DynaMax 1 (DM-1)" and a customized Microsoft Excel worksheet were used to analyzed and plot the accelerometer data.

5.5.2 Rate Transducers

An Analog Systems 3-axis rate transducer with a range of 1,200 degrees/sec in each of the three directions (roll, pitch, and yaw) was used to measure the rates of motion of the test vehicle. The rate transducer was mounted inside the body of the EDR-4 6DOF-500/1200. Data was recorded at 10,000 Hz to a second data acquisition board inside the EDR-4 6DOF-500/1200 housing. The raw data measurements were then downloaded, converted to the appropriate Euler angles for analysis, and plotted. "EDR4COM" and "DynaMax Suite" computer software programs and a customized Microsoft Excel worksheet were used to analyze and plot the angular rate sensor data.

An additional angle rate sensor, the ARS-1500, with a range of 1,500 degrees/sec in each of the three directions (roll, pitch, and yaw) was used to measure the rates of rotation of the test vehicle. The angular rate sensor was mounted on an aluminum block inside the test vehicle near the center of gravity. Data was recorded at 10,000 Hz to the SIM unit. The raw data measurements were then downloaded, converted to the proper Euler angles for analysis, and

plotted. The computer software program "DTS TDAS Control" and a customized Microsoft Excel worksheet were used to analyze and plot the angular rate sensor data.

5.5.3 Pressure Tape Switches

For test no. NYTCB-5, five pressure-activated tape switches, spaced at approximately 6.6 ft (2 m) intervals, were used to determine the speed of the vehicle before impact. Each tape switch fired a strobe light which sent an electronic timing signal to the data acquisition system as the right-front tire of the test vehicle passed over it. Test vehicle speeds were determined from electronic timing mark data recorded using TestPoint and LabVIEW computer software programs. Strobe lights and high-speed video analysis provided a backup method of determining the vehicle speed in the event that it could not be determined from the electronic data.

5.5.4 High-Speed Photography

Two AOS VITcam high-speed digital video cameras, three AOS X-PRI high-speed digital video cameras four JVC digital video cameras, and two Canon digital video cameras were utilized to film test no. NYTCB-5. Camera details, camera operating speeds, lens information, and a schematic of the camera locations relative to the system are shown in Figure 16. The high-speed videos were analyzed using ImageExpress MotionPlus software. Actual camera speed and camera divergence factors were considered in the analysis of the high-speed videos.

_	No.	Туре	Operating Speed (frames/sec)	Lens	Lens Setting
7	2	AOS Vitcam CTM	500	Kowa 8mm fixed	
High-Speed Video	4	AOS Vitcam CTM	500	Sigma 24-135	28
	5	AOS X-PRI Gigabit	500	Sigma 24-70	50
	6	AOS X-PRI Gigabit	500	Tamron 100-300	135
	7	AOS X-PRI Gigabit	500	Fujinon 50 mm fixed	
Digital Video	1	JVC – GZ-MC500 (Everio)	29.97		
	2	JVC – GZ-MG27u (Everio)	29.97		
	3	JVC – GZ-MG27u (Everio)	29.97		
	4	JVC – GZ-MG27u (Everio)	29.97		
	1	Canon ZR90	29.97		
	2	Canon ZR10	29.97		

Figure 16. Camera Locations, Speeds, and Lens Settings, Test No. NYTCB-5

6 FULL-SCALE CRASH TEST NO. NYTCB-5

6.1 Test No. NYTCB-5

The 5,124-lb (2,324-kg) pickup truck impacted the pinned temporary concrete barrier system at a speed of 64.3 mph (103.4 km/h) and at an angle of 26.2 degrees. A summary of the test results and sequential photographs are shown in Figure 17. Additional sequential photographs are shown in Figures 18 through 20. Documentary photographs of the crash test are shown in Figures 21 and 22.

6.2 Weather Conditions

Test No. NYTCB-5 was conducted on September 4, 2009 at approximately 2:15 pm. The weather conditions as per the National Oceanic and Atmospheric Administration (station 14939/LNK) were reported as shown in Table 3.

Table 3. Weather Conditions, Test No. NYTCB-5

Temperature	75° F
Humidity	54 %
Wind Speed	10 mph
Wind Direction	70° from True North
Sky Conditions	Overcast
Visibility	10.00 Statute Miles
Pavement Surface	Dry
Previous 3-Day Precipitation	0.43 in.
Previous 7-Day Precipitation	0.43 in.

6.3 Test Description

Initial vehicle impact was to occur 4 ft - 3 3/16 in. (1.3 m) upstream from the centerline of the joint between barrier nos. 4 and 5, as shown in Figure 23. Actual vehicle impact occurred at the targeted impact location. A sequential description of the impact events is shown in Table 4. The vehicle came to rest 205 ft (62.5 m) downstream from impact and 18 ft - 8 in. (5.7 m)

laterally away from the traffic-side face of the barrier. The vehicle trajectory and final position are shown in Figures 17 and 24.

Table 4. Sequential Description of Impact Events, Test No. NYTCB-5

TIME (sec)	EVENT
0.000	Right-front corner of the vehicle impacted barrier no. 4 at the targeted impact location.
0.006	Right headlight contacted the top of barrier no. 4, and the right-front tire became airborne.
0.016	Top of barrier no. 4 deflected backward at the downstream end, the joint between barrier nos. 4 and 5 twisted upstream, and the right-front tire deflated.
0.024	Right side of grill contacted the front face of barrier no. 4.
0.036	Top of barrier no. 5 deflected backward at the upstream end, the front of the engine hood deformed inward, and the vehicle pitched upward and began to redirect.
0.040	Top of barrier no. 4 deflected backward at the upstream end, the joint between barrier nos. 3 and 4 twisted downstream, and the top of the right-front door became ajar.
0.052	Downstream end of barrier no. 3 experienced cracking on the back side.
0.058	Toe on the back side of barrier no. 5 experienced cracking near the barrier's middle.
0.066	Tops of barrier nos. 3 and 5 deflected backward at their downstream ends.
0.076	Toe on the back side of barrier no. 3 experienced cracking, a large piece of concrete separated from the back side of barrier no. 5 near the barrier's middle, and the vehicle continued to redirect.
0.086	Surrogate occupant's head contacted the right-side door window and caused the window to disengage from the top frame and deflect outward.
0.098	Top of barrier no. 3 deflected backward at the upstream end, and the left-front tire became airborne.
0.116	A large piece of concrete separated from the back side of barrier no. 3 at the downstream end.
0.146	Tops of barrier nos. 2 and 6 deflected backward at the downstream end and both ends, respectively.
0.160	Crack that originated in the back-side toe of barrier no. 5 extended to the top of the barrier, and the right-rear tire contacted the downstream end of the front toe of barrier no. 4.
0.166	Right-rear tire deflated.
0.174	Right-rear tire became airborne.
0.182	Top of barrier no. 7 deflected backward at the upstream end.
0.210	Right corner of the rear bumper contacted the front face of barrier no. 4 near the downstream end, and the vehicle was parallel to the system with a velocity of approximately 54 mph (87 km/h).

0.240	Toe on the back side of barrier no. 5 experienced cracking near the downstream end, and the right-rear tire disengaged from the axle and was crushed between the barrier and the vehicle.
0.252	Left-rear tire became airborne.
0.280	Vehicle experienced roll toward the left, and a large piece of concrete separated from the back-side toe of barrier no. 5 near the downstream end.
0.296	Left-front tire lost contact with the front face of barrier no. 5, and the top of barrier no. 4 deflected forward along its entire length.
0.312	Top of barrier no. 2 deflected forward along its entire length.
0.322	A large piece of concrete separated from the toe on the back side of barrier no. 3 near the upstream quarter point, and the top of barrier no. 3 deflected forward.
0.336	Right-rear tire became detached from the axle and lost contact with the top of barrier no. 5, and the vehicle exited the system at an angle of 7.7 degrees with a velocity of approximately 51 mph (82 km/h).
0.488	Front toe of barrier no. 2 contacted the ground.
0.518	Front toe of barrier no. 3 contacted the ground.
0.576	Front toe of barrier no. 5 and the left-front tire contacted the ground.
0.594	Front toe of barrier no. 4 contacted the ground.
0.610	Detached right-rear tire contacted the upper-back edge of barrier no. 4.
0.638	Vehicle experienced roll toward the right.
0.752	Vehicle pitched upward.
0.790	Right-front tire contacted the ground.
1.044	Right-rear quarter panel contacted the top of barrier no. 7.
1.134	Vehicle rolled toward its left side.
1.260	Right-rear quarter panel lost contact with the barrier system.
1.530	Right-rear quarter panel contacted the top of barrier no. 8.
1.828	Left-rear tire contacted the ground.
1.898	Right corner of the rear bumper lost contact with the top of barrier no. 10.
2.038	Tailpipe contacted the ground as the vehicle continued downstream.

6.4 Barrier Damage

Damage to the barrier was moderate, as shown in Figures 25 through 32. Barrier damage consisted of contact and gouge marks, spalling of concrete, and concrete cracking and failure. The length of vehicle contact along the barrier was approximately 22 ft -8 in. (6.9 m) which spanned from 6 ft -4 in. (1.9 m) upstream from the center of the joint between barrier nos. 4 and 5 to 16 ft -4 in. (5.0 m) downstream from the center of the joint between barrier nos. 4 and 5.

The vehicle contacted the system again for approximately 60 ft (18.3 m) which spanned from the joint between barrier nos. 7 and 8 through the downstream end of barrier no. 10.

Tire marks were visible on the front fact of barrier nos. 4 and 5 and started 76 in. (1,930 mm) upstream from the downstream end of barrier no. 4 and continued through 44 in. (1,118 mm) upstream from the downstream end of barrier no. 5. Contact marks were also found on the top faces of barrier nos. 8 through 10 as well as the connection keys between barrier nos. 4 and 7.

A 28-in. (711-mm) long gouge was found on the front face of barrier no. 4 and began 51 in. (1,295 mm) upstream from the downstream end. A 19-in. (483-mm) long gouge was found on the front face of barrier no. 4 and began 39 in. (991 mm) upstream from the downstream end. A 7-in. (178-mm) and a 32-in. (813-mm) long gouge was found on the front face of barrier no. 5 near the upstream end. Gouges, 7 in. (178 mm) and 6 in. (152 mm) long, were found on the front- and back-top edges, respectively, beginning 65 in. (1,651 mm) upstream from the downstream end of barrier no. 5. Gouges, 4 in. (102 mm) and 3 in. (76 mm) long, were found on the front- and back-top edges, respectively, near the upstream quarter point of barrier no. 5. A 4-in. (102-mm) gouge was found on the front and back-top edges near the upstream quarter-point of barrier no. 8.

Minor cracking was found on barrier nos. 2 and 6. A 7½-in. (191-mm) long vertical crack was found at the corner of the upstream connection key socket in barrier no. 3. Barrier no. 4 experienced a vertical crack that extended the height of the barrier near the center anchor hole on the front side as well as cracking around the downstream-most anchor hole on the back side. Vertical cracks were found in the front and back face of barrier no. 5 at 19 in., 50 in., and 79 in. (483 mm, 1,270 mm, and 2,007 mm) downstream from the upstream end and at the middle of the

barrier. A 15-in. (381-mm) long, vertical crack was found at the downstream anchor hole on the front face of barrier no. 5.

Concrete spalling occurred on barrier nos. 3 through 7. The back side of barrier no. 3 experienced concrete spalling at the lower-upstream corner, the upper-downstream corner, and near the middle of the barrier. Concrete spalling also occurred at the lower-downstream corner at the front of barrier no. 3. An 8-in. x 14-in. x 3-in. (203-mm x 356-mm x 76-mm) piece of concrete was removed from the bottom-upstream corner on the back side of barrier no. 4. A 5-in x 12-in. (127-mm x 305-mm) piece of concrete was fractured near the connection key, but it remained attached to the barrier. A 4-in. x 15-in. (102-mm x 381-mm) piece of concrete was removed from the upstream toe on the front side of barrier no. 5. A 28-in. (711-mm) long piece of concrete was removed from the back side of barrier no. 5, slightly upstream of the middle of the barrier. Concrete spalling, measuring 15 in. x 11 in. (381 mm x 279 mm), was found at the bottom-downstream corner on the back side of barrier no. 5. Concrete spalling occurred on barrier no. 6 at the front upstream-most anchor hole. Concrete spalling, measuring 33 in. x 7 in. (838 mm x 178 mm), occurred near the upstream quarter-point of the toe on the back side of barrier no. 7.

The maximum permanent set of the barrier system was 9 in. (229 mm) at the downstream end of barrier no. 4, as measured in the field. The maximum lateral dynamic barrier deflection, including tipping of the barrier along the top surface, was 20.5 in. (521 mm) at the upstream end of barrier no. 5, as determined from high-speed video analysis. The working width of the system was found to be 32.5 in. (826 mm).

6.5 Vehicle Damage

The damage to the vehicle was moderate, as shown in Figures 33 through 35. The maximum occupant compartment deformations are listed in Table 5 with the deformation limits established in MASH for various areas of the occupant compartment. It should be noted that none of the MASH established deformation limits were violated. Complete occupant compartment and vehicle deformations and the corresponding locations are provided in Appendix C.

Table 5. Maximum Occupant Compartment Deformations by Location

LOCATION	MAXIMUM DEFORMATION in. (mm)	MASH ALLOWABLE DEFORMATION in. (mm)
Wheel Well & Toe Pan	71/4 (184)	≤ 9 (229)
Floor Pan & Transmission Tunnel	11/4 (32)	≤ 12 (305)
Side Front Panel (in Front of A-Pillar)	³ / ₄ (19)	≤ 12 (305)
Side Door (Above Seat)	2½ (64)	≤ 9 (229)
Side Door (Below Seat)	1 (25)	≤ 12 (305)
Roof	NA	≤ 4 (102)
Windshield	NA	≤ 3 (76)

The majority of the damage was concentrated on the right-front corner and right side of the vehicle where the impact occurred. The right side of the bumper was crushed inward and back and the plastic portion was fractured. The left side of the front bumper was deformed downward 2 in. (51 mm). Denting and scraping were observed on the entire right side. The right-front and right-rear quarter-panels were deformed inward toward the engine compartment and truck box, respectively. The top of the right-front door, the left side of the hood, and the tailgate were ajar. The grill was fractured around the bottom of the right-side headlight assembly. The

right-side headlight and left-side foglight were fractured, and the right-side foglight was removed from the vehicle. The right-front wheel and tire disengaged from the vehicle and were located under the rest of the wheel assembly. The right upper control arm and tie rod disengaged, while the right upper control arm fractured. The right-rear axle sheared, and the brake assembly disengaged from the vehicle. The right taillight was removed. The right side of the rear bumper was deformed outward, while a kink was found in the left side. The left-rear wheel assembly was deformed outward. The right side of the floorpan encountered significant deformation. The roof and all window glass remained undamaged.

6.6 Occupant Risk

The calculated occupant impact velocities (OIVs) and maximum 0.010-sec occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 6. It is noted that the OIVs and ORAs were within the suggested limits provided in MASH. The calculated THIV, PHD, and ASI values are also shown in Table 6. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 17. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix D. Due to technical difficulties, the DTS did not collect acceleration data in the longitudinal direction, but it did collect acceleration data in the lateral direction and angular data from the rate sensors.

6.7 Discussion

The analysis of the test results for test no. NYTCB-5 showed that the pinned temporary concrete barrier system adequately contained and redirected the 2270P vehicle with controlled lateral displacements of the barrier. There were no detached elements nor fragments which showed the potential for penetrating the occupant compartment nor presented undue hazard to

Table 6. Summary of OIV, ORA, THIV, and PHD Values, Test No. NYTCB-5

Evaluation Criteria		Transducer			
		EDR-3	EDR-4	DTS	
OIV	Longitudinal	-16.41 (-5.00)	-12.40 (-3.78)	NA	
ft/s (m/s)	Lateral	-20.14 (-6.14)	-18.51 (-3.78)	-19.93 (-6.07)	
ORA	Longitudinal	-4.79	-5.31	NA	
g's	Lateral	-6.92	-6.65	-6.92	
	THIV ft/s (m/s)		21.21 (6.46)	20.94 (6.38)	
PHD g's		NA	7.11	7.59	
ASI		1.50	1.36	1.40	

other traffic. Deformations of, or intrusions into, the occupant compartment that could have caused serious injury did not occur. The test vehicle did not penetrate nor ride over the barrier and remained upright during and after the collision. Vehicle roll, pitch, and yaw angular displacements were noted, as shown in Appendix D, and were deemed acceptable because they did not adversely influence occupant risk safety criteria nor cause rollover. After impact, the vehicle exited the barrier at an angle of 7.7 degrees and its trajectory did not violate the bounds of the exit box. Therefore, test no. NYTCB-5 conducted on the pinned temporary concrete barrier system was determined to be acceptable according to the MASH safety performance criteria for test designation no. 3-11.

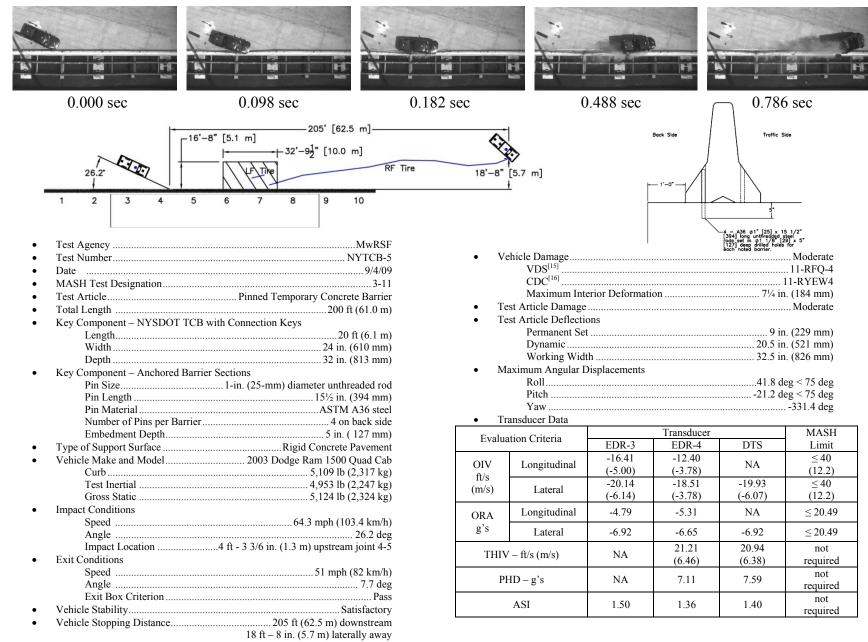


Figure 17. Summary of Test Results and Sequential Photographs, Test No. NYTCB-5

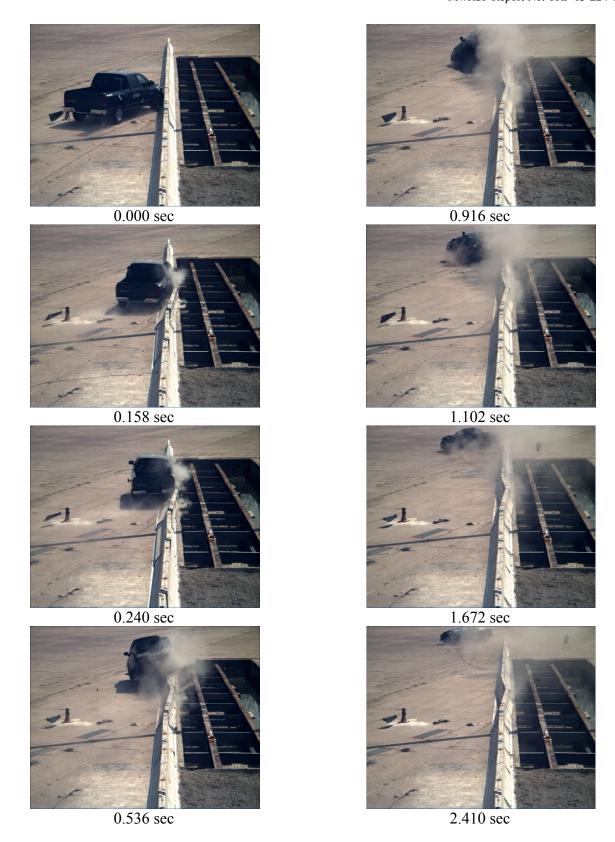


Figure 18. Additional Sequential Photographs, Test No. NYTCB-5

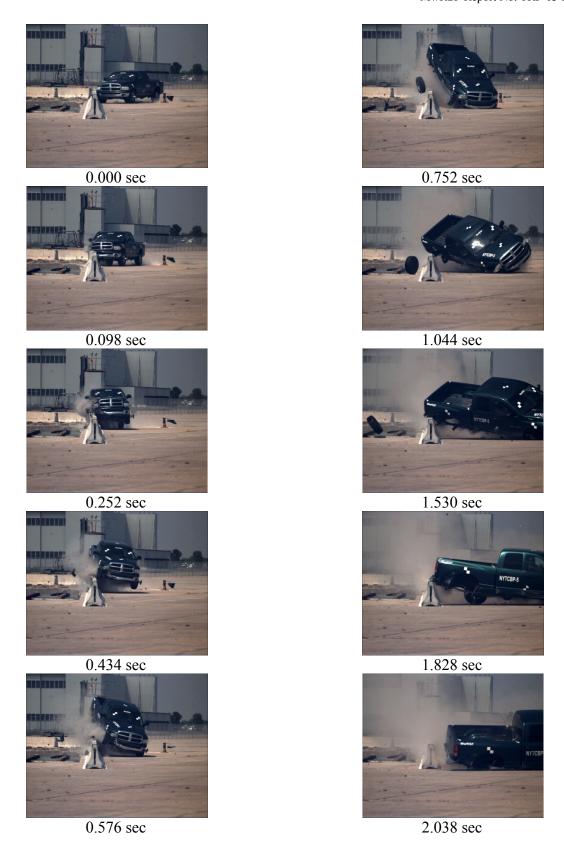


Figure 19. Additional Sequential Photographs, Test No. NYTCB-5

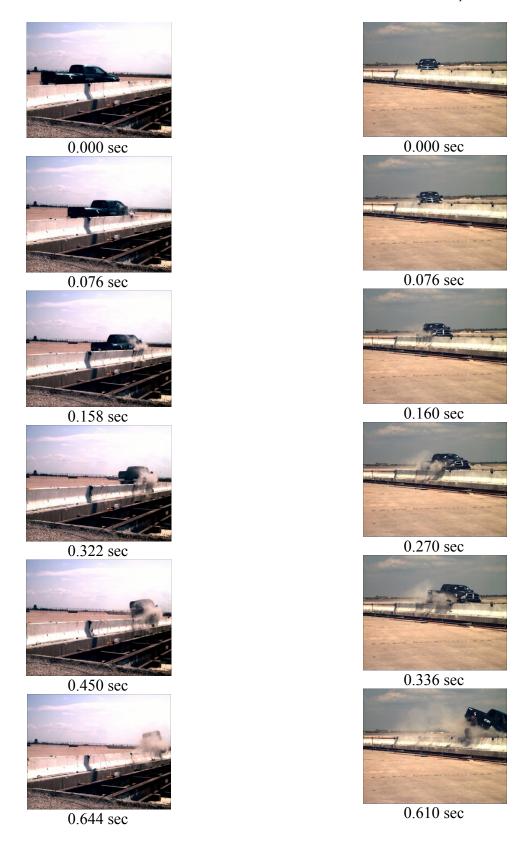


Figure 20. Additional Sequential Photographs, Test No. NYTCB-5

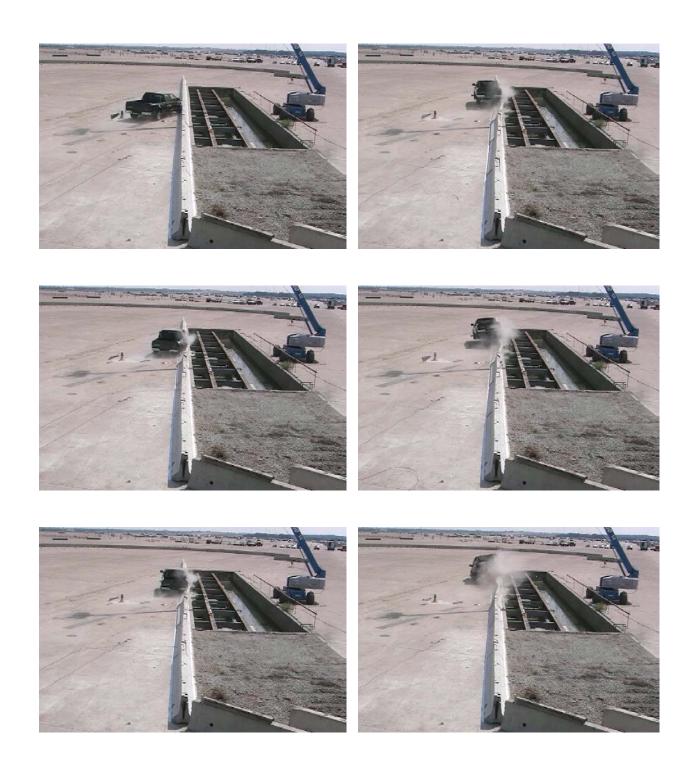


Figure 21. Documentary Photographs, Test No. NYTCB-5

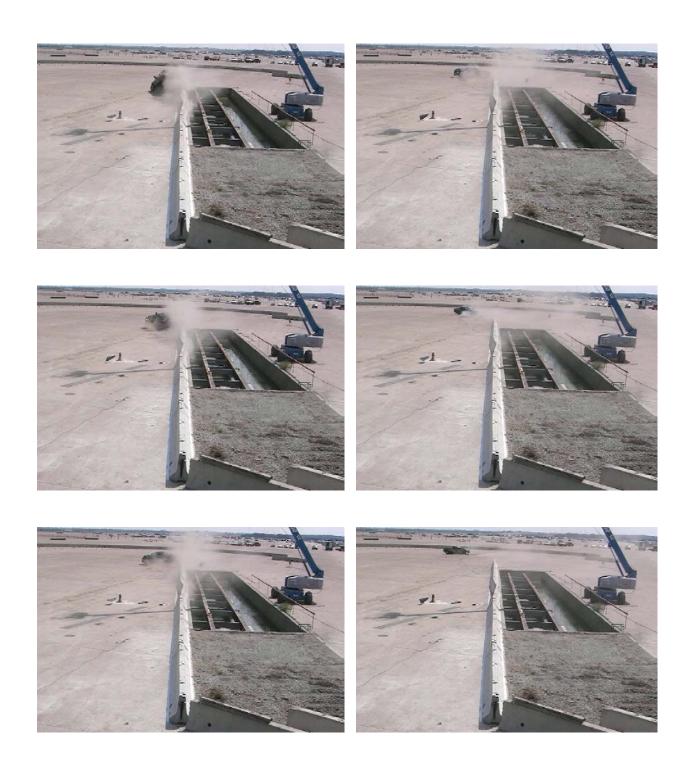


Figure 22. Documentary Photographs, Test No. NYTCB-5

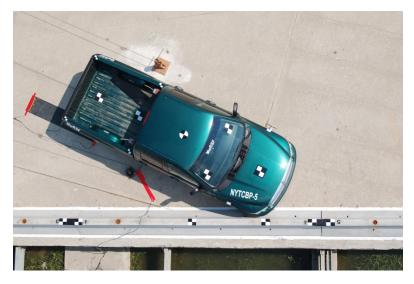






Figure 23. Impact Location, Test No. NYTCB-5





Figure 24. Vehicle Final Position and Trajectory Marks, Test No. NYTCB-5





Figure 25. System Damage, Test No. NYTCB-5





Figure 26. System Damage, Test No. NYTCB-5





Figure 27. Barrier Nos. 2 and 3 Damage, Test No. NYTCB-5





Barrier No. 3

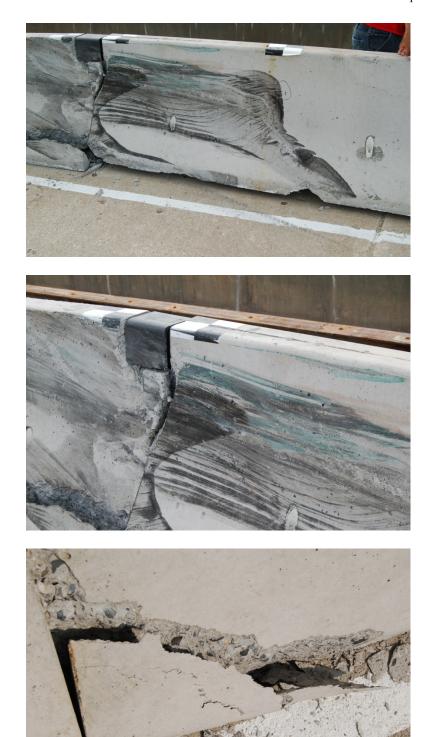


Figure 28. Barrier No. 4 Damage, Test No. NYTCB-5



Figure 29. Barrier No. 5 Damage, Test No. NYTCB-5







Joint between Barrier Nos. 6 and 7



Barrier No. 7



Barrier No. 7
Figure 30. Barrier Nos. 6 and 7 Damage, Test No. NYTCB-5

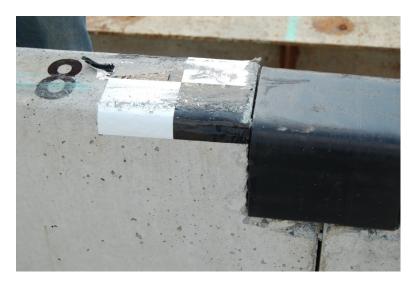






Figure 31. Barrier No. 8 Damage, Test No. NYTCB-5





Figure 32. Barrier Nos. 9 and 10 Damage, Test No. NYTCB-5



Joint between Barrier Nos. 9 and 10











Figure 33. Vehicle Damage, Test No. NYTCB-5







Figure 34. Vehicle Damage, Test No. NYTCB-5





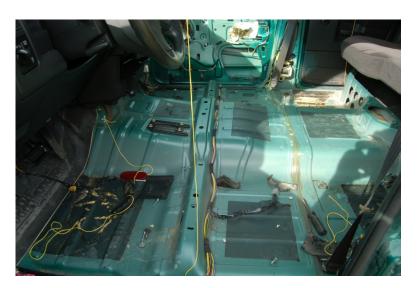


Figure 35. Occupant Compartment Deformation, Test No. NYTCB-5

7 SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS

The goal of this study was to investigate the potential for reducing barrier deflections through the use of vertical steel pins placed through the back-side toe of the NYSDOT's New Jersey-shape TCB sections. A longitudinal barrier system was constructed with ten, 20-ft (6.1-m) long, temporary concrete barrier sections attached end-to-end utilizing a connection at the joints. All sections were pinned to the concrete surface with four 1-in. (25-mm) diameter by 15½-in. (394-mm) long steel rods placed through the back-side toe of the barrier sections and inserted into drilled holes within the rigid concrete surface.

One full-scale vehicle crash test was conducted, evaluated, and reported according to the TL-3 safety performance criteria found in MASH. A summary of the safety performance evaluation for the test is provided in Table 7. Test no. NYTCB-5 (test designation 3-11) consisted of a 5,124-lb (2,324-kg) pickup truck impacting the pinned temporary concrete barrier system at a speed of 64.3 mph (103.4 km/h) and at an angle of 26.2 degrees, thus resulting in an impact severity of 138.0 kip-ft (187.2 kJ). The target impact location was 4 ft - 3 3/16 in. (1.3 m) upstream from the centerline of the joint between barrier nos. 4 and 5. The maximum permanent set and dynamic deflections were 9 in. (229 mm) and 20.5 in. (521 mm), respectively. Following an evaluation of the test results, the pinned, New Jersey-shape, temporary concrete barrier system was found to meet the MASH TL-3 safety requirements for the 2270P pickup truck.

Two full-scale crash tests were performed on the NYSDOT's New Jersey TCB sections when pinned to a rigid concrete surface. Test no. NYTCB-4 utilized four pins placed on the back-side toe of every other barrier segment, while test no. NYTCB-5 used four pins placed on the back-side toe of each section. For test no. NYTCB-5 and as a result of the continuous pinning, the maximum permanent set deflection, maximum dynamic deflection, and working

width were reduced by 83.2, 68.4, and 66.0 percent, respectively, from that observed during test no. NYTCB-4. This result was significant when considering the impact severity for test no. NYTCB-5 was approximately 21 percent greater than that observed in test no. NYTCB-4.

Table 7. Summary of Safety Performance Evaluation Results

Evaluation Factors		Evaluation	Test No. NYTCB-5			
Structural Adequacy	A.	Test article should contain and re to a controlled stop; the vehicle override the installation although test article is acceptable.	S			
	D.	Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH.				
	F.	The vehicle should remain upri maximum roll and pitch angles ar	S			
	Н.	Occupant Impact Velocities (OIV MASH for calculation procedure)	S			
Occupant		Occupant Imp				
Risk	Component Longitudinal and Lateral	Component	Preferred	Maximum		
		Longitudinal and Lateral	30 ft/s (9.1 m/s)	40 ft/s (12.2 m/s)		
	I.	The Occupant Ridedown Acce Section A5.3 of MASH for calc following limits:				
	Occupant Ridedown Acceleration Limits				S	
		Component	Preferred	Maximum		
Longitudinal and Lateral 15.0 g's			20.49 g's			

S – Satisfactory

U – Unsatisfactory

NA - Not Applicable

For 1100C passenger car impacts into the NYSDOT's New Jersey TCB sections that are pinned to a rigid concrete surface, the impact severity would be reduced by at least 50 percent, thus resulting in reduced barrier deflections from those observed in test no. NYTCB-5. Further, the 1100C passenger car of MASH has been shown to be more stable than the NCHRP Report No. 350 820C passenger car. Thus, it is the researchers' opinion that the New Jersey-shape TCB sections that are pinned to a rigid concrete surface would also meet the TL-3 safety performance criteria specified in MASH for the 1100C passenger car impacts. However, this opinion can only be verified through the use of full-scale vehicle crash testing.

The pinned, TCB system described herein was designed for use with the NYSDOT New Jersey-shape temporary concrete barrier segments. These sections utilize a connection key which provides sufficient torsional resistance and load distribution about the longitudinal axis to the adjoining TCB sections. Therefore, the vertical pinning of the NYDOT's TCB sections to rigid surfaces should not be used with other TCB sections or joint connections without further study. Although it is very likely that this pinned system can be adapted to other accepted TCB systems, it is necessary to utilize some criteria to aid in that determination. They are as follows:

- Joints between barrier segments must have comparable or greater torsional rigidity about the longitudinal barrier axis when compared to that of the as-tested configuration.
- 2. Alternative barrier segment lengths would be acceptable as long as they are at least 20 ft (6.1 m) long and utilize an equivalent or greater number of anchors per foot of barrier length. With shorter barrier lengths, it is believed that additional barrier rotation will occur due to the greater number of joints, thus resulting in the propensity for increased vehicle climb and rollover.

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- 3. Alternative barrier segments should have comparable mass per unit length.
- 4. Alternative barrier segments should have equal or greater reinforcement than that provided in the NYSDOT's New Jersey-shape barrier described herein. This reinforcement recommendation is to include the longitudinal steel, shear stirrups, and containment steel bars surrounding the holes used for placing the vertical pins.
- 5. The shape of alternative barrier segments may require further study. Past research has shown that the F-shape provides slightly improved results over those observed during tests performed on the New Jersey-shape barrier. However, further study may be necessary to assure safe performance when applying the design to other barrier shapes.

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9 APPENDICES

Appendix A. Material Specifications



JERSEY PRECAST CORP.

533 Nottingnam Way, Hamilton Township, NJ 08638
Phone: 609-689-3700 • Main Fax: 609-689-3797 • Accts. Fax: 609-689-9222
E-Mail: mail@jerseyprecast.com

www.jerseyprecast.com



August 20, 2009 Job #09-082

CERTIFICATION OF COMPLIANCE NYS DEPARTMENT OF TRANSPORTATION

University of Nebraska Lincoln Project: Midwest Roadside Safety 2. Contractor: 4800 NW 35th Street Lincoln, NE. 68524 Precast Concrete Barrier 24" x 32" in 20" Material Supplied: 3. Lengths 160 Lft. Typical Barrier (8 Pcs.) 4. Quantity of Material Supplied: Barrier Elements are dated per date of fabrication. Means of Identification: 8/19/09 on Flat Bed Trailers. 6. Date & Method of Shipment: The 160 Lft. of Precast Concrete Temporary 7. Material Found to Conform: Barrier as supplied to the job site has been manufactured using tested and approved materials and meets or exceeds applicable ASTM standards and NYSDOT, Specifications and requirements. Signature of Authorization For Jersey Precast Corp: General Manager Khwaja Abbas. State of New Jersey Notary Public Attestment: County of Middlesex Sworn and Subscribed Before Me The Do Day of August.

Commission Expires 2/3/20

Over 25 Years of Experience

Figure A-1. Temporary Concrete Barrier Certificate of Conformity, Test No. NYTCB-5

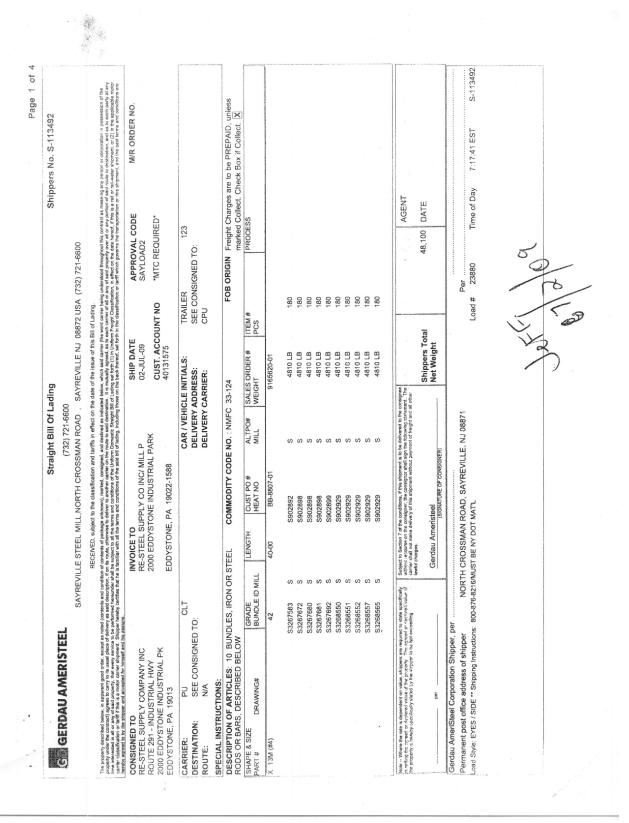


Figure A-2. No. 4 Rebar Material Specifications, Test No. NYTCB-5

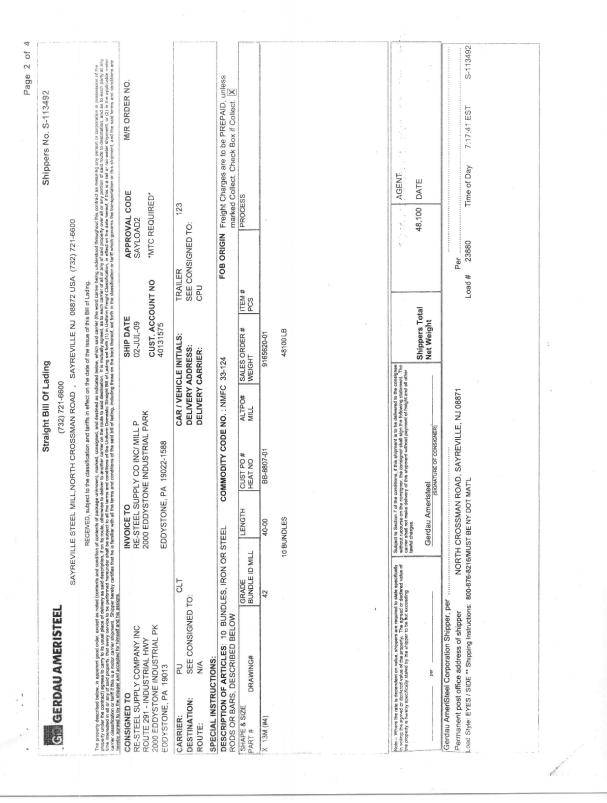


Figure A-3. No. 4 Rebar Material Specifications, Test No. NYTCB-5

NAT 10 10 10 10 10 10 10 1	INVOICE TO RE-STEEL SUPPLY CO INCIMIL P STHIP DATE COTROLOR COTROLO	NAVOICE TO NAV	AVREVILLE STEEL MILL ORTH CROSSMAN ROAD AYREVILLE NJ 08872 USA	Chemical and Physical Test Report		
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Part	P S S C M N C M N M B N S M T Z C C C C C C C C C	SPECIFICATION ASTM A615M GRAZO [GR60] P S SI Cu Ni Cr Mo V O19 O53 .17 .33 .09 .06 .023 .010 O19 O53 .17 .33 .09 O5 .00 O19 O53 .010 O19 O53 .010 O19 O53 .010 O19 O53 .010 O19 O19	IIP TO E-STEEL SUPPLY COMPANY INC SUTE 291 - INDUSTRIAL HWY	INVOICE TO RE-STEEL SUPPLY CO INC/ MILL P 2000 EDDYSTONE INDUSTRIAL PARK	3HIP DATE 77/02/09	
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466 67 MPA Tensile: 100650 PSI, 708.78 MPA %EI: 12.0/8n, 12.5/200MM Bend: OK Def HT: 0, 0MM %uh 4.9t. E SPECIFICATION ASTM A615M GRAZO (GR60) P S SI Cu Ni Cr Mo V Nb B N Sn A1 Ti Zr Ca CEQV O18 0.055 1.8 ZB 0.09 0.025 0.014 0.02 0.0000 0.032 0.000 0.0000 0.00 0.0	466 67 MPA Tensile: 102800 PSI, 708.78 MPA %EI: 12.5/81n, 12.0/200MM Bend: OK Def HT: 0, 0MM %/In 43L 465 88 MPA Tensile: 102800 PSI, 708.78 MPA %EI: 12.5/81n, 12.0/200MM Bend: OK Def HT: 0, 0MM %/In 4.9L SALES ORDER 9 ASTMAGISH GREGI P S S SI Cu Ni Cr Mo V Nb B N Sn AI TI Zr Ca Capr 10.8 .065 .18 .28 .09 .08 .026 .014 .002 .0000 .0000 .0000 .00	460 67 MPA Tensile: 102600 PSI, 708.78 MPA %EI: 12.08in. 465.88 MPA Tensile: 102600 PSI, 708.78 MPA %EI: 12.08in. 465.88 MPA Tensile: 102600 PSI, 708.78 MPA %EI: 12.08in. P S SI CIP NI Cr Mo V OI8 0.055 .18 28 0.9 08 0.26 .014 4.39 65 MPA Tensile: 986.35 PSI, 680.06 MPA %EI: 14.0/8in. 439 65 MPA Tensile: 986.35 PSI, 680.06 MPA %EI: 14.0/8in. Coulity Director Gerdau Ameristeel	C Mn P S Si 46 63 028 067 16	10 026 043 0000 0000 047 000 017	Ca	THE REAL PROPERTY AND ADDRESS OF THE PROPERTY ADDRESS OF THE PROPERTY AND ADDRESS OF THE PROPERTY ADDR
SPECIFICATION SALES ORDER	SPECIFICATION	SPECIFICATION ASTM A615M GR420 [GR60] P - S Si Cu Ni Cr Mo V	Yield 66815 PSI, 460.67 MPA Yield 67570 PSI, 465.88 MPA	%E: 12.0/8in, 12.0/200MM Bend: OK Def HT: 0, 0MM %EI: 12.0/8in, 12.0/200MM Bend: OK Def HT: 0, 0MM	1 4.7L	
GRADE SPECIFICATION SATIMABISM GRAZO [GR60] ASTM ABISM GRAZO [GR60] ASTM G	GRADE SPECIFICATION SATIMAGISM GRAZO [GR60] ASTM AGISM GRAZO [GR60] ASTM GRAZO [GR60]	GRADE SPECIFICATION		The second secon	76I/II -4.9L	
420 (60) ASTM A615M GR420 [GR60] C Mn P S Si Cu Ni Cr No V Nb B N Sn Ti Cr Cr C Eqv 471 018 28 056 118 28 08 09 08 026 0014 002 0000 0000 002 000 000 000 000 000	420 (60) ASTM A615M GR420 [GR60] C Mn P S Si Cu Ni Cr No V Nb B N Sn A1 Ti Zr Ca CEqv 471 0.18 0.56 1.8 28 0.90 0.8 0.02 0.014 0.02 0.000 0.000 0.032 0.000 0.000 0.000 0.000 0.000 0.000 Yield 62895 PSI, 439 69 MPA Tursile: 98635 PSI, 690.06 MPA %EI: 14.0/8in, 14.0/200MM Bend: OK Def HT: 0, 0MM %in 4.3L	420 (60) ASTM A615M GR420 (GR60) C Mn P S Si Cu Ni Cr Mo V 47	GRADE		SALES ORDER	CUST P.O. NUMBER
C Mn P S S Si Cu Ni Cr Mo V Nb B N Sn Ai Ti Zr Ca CEqv Vert 47 71 018 055 18 055 18 28 09 08 0.08 0.05 0.00 0.00 0.00 0.00 0.00 0	C Mn P > S Si Cu Ni Cr Mo V Nb B N Sn Ai Tr Zr Ca CEqv 447 771 018 055 18 28 09 0.8 0.26 014 002 0000 0000 032 000 0000 000 0000 00	C Mn P S Si Cu Ni Cr Mo V A7 71 .019 .056 .18 .28 .09 .08 .026 .014 Yield 62360 PSi, 439 63 MPA Tensile: 98635 PSi, 680.06 MPA %EI: 15.0/8in. Yield 62895 PSi, 439 56 MPA Tensile: 98635 PSi, 680.06 MPA %EI: 14.0/8in. A	420 (60)	R60]	9165620-01	BB-8807-01
Viola 69600 DEL 430 ES MARA TE CALLA OFFICE OF THE CALLA OFFICE OF THE CALLA OFFICE OF	Yield 62895 PSI, 439,69 MPA Tensile: 98635 PSI, 680.06 MPA "AEI: 14.0/8in, 14.0/200MM Bend: OK Def HT: 0, 0MM "AII" 4.3L Yield 62895 PSI, 439,65 MPA Tensile: 98635 PSI, 680.06 MPA "AEI: 14.0/8in, 14.0/200MM Bend: OK Def HT: 0, 0MM "AII" 4.3L	Yield 62500 P3l, 439 89 MPA Tensile: 97480 P3l, 672.1 MPA %EI: 15.0/8in, Yield 62895 PSl, 438 65 MPA Tensile: 98635 PSl, 680.06 MPA %EI: 14.0/8in, Yield 62895 PSl, 438 65 MPA Tensile: 98635 PSl, 680.06 MPA %EI: 14.0/8in, Iding the billets, was produced and manufactured in the United Bhaskar Yalamanchili Quality Director Gerdau Ameristeel	C Mn P S Si .47 .71 .018 .055 .18	OF Mo V Nb B N Sn Al Ti	O	
Yield 62895 PSI, 438.65 MPA Tensile: 98635 PSI, 680.06 MPA %EI: 13.0/6/n, 14.0/200MM Bend: OK Def HT: 0, 0MM %/In 4.2L:	*	rding the billets, was produced and manufactured in the United Bhaskar Yalamanchili Quality Director Gerdau Ameristeel	Yield 63500 PSI, 439.89 MPA Yield 62895 PSI, 439.65 MPA	%E: 15.0/8in, 15.0/200MM Bend: OK Def HT: 0, 0MM A %E: 14.0/8in, 14.0/200MM Bend: OK Def HT: 0, 0MM		
uding the billets, was produced and manufactured in the United			Mo	France B. & Bengench.	Mgr. Metallurg. Svcs. SAYREVILLE STEEL MILL	

Figure A-4. No. 4 Rebar Material Specifications, Test No. NYTCB-5

SHPC PRE-STEEL SUPPLY CO INC/ MILL P 2000 EDDYSTONE, PA 19022-1588 EDDYSTONE, PA 19022-1588 CUST.			
SPECIFICATION SPECIFICATION SPECIFICATION P S S Cu N Cr Mo V	SHIP TO RE-STEEL SUPPLY COMPANY INC ROUTE 291 - INDUSTRIAL HWY 2000 EDDYSTONE INDUSTRIAL PK EDDYSTONE: PA 19013 EDDYSTONE: PA 19013	OICE TO STEEL SUPPLY CO INC/ MILL P 0 EDDYSTONE INDUSTRIAL PARK SYSTONE. PA 19022-1588	SHIP DATE 07/02/09 CUST. ACCOUNT NO 40/13/575
SPECIFICATION ASTIN A615M GRA20 (GR60)			
C	E (6))ER
Yield 63805 PSI, 438 92 MPA Tensile: 97505 PSI, 672.27 MPA %EI: 14.0/8ln, Yield 64995 PSI, 448.12 MPA Tensile: 100185 PSI, 690.75 MPA %EI: 13.5/8ln yield 64995 PSI, 448.12 MPA Tensile: 100185 PSI, 690.75 MPA %EI: 13.5/8ln yield 64995 PSI, 448.12 MPA Tensile: 100185 PSI, 690.75 MPA %EI: 13.5/8ln yield 64995 PSI, 448.12 MPA %EI: 14.0/8ln yield 64995 PSI, 448.12 MPA %EI: 14.0/8ln yield 94995 PSI, 448.12 MPA %EI: 14.0/8ln yield 94995 PSI, 480.75 MPA	C Mn P .47 .64 .014	Mo V NB B N Sn .027 .010 .000 .0000 .0000 .027	C Eqv
uding the biliets, was produced and manufactured in the United	Yield 63805 PSI, 439.92 MPA Yield 64995 PSI, 448.12 MPA	%EI: 14.0/8in, 14.0/200MM Bend: OK I	
Quality Director	States of America Bhaskar Yalamanchili Quality Director	THE ABOVE FIGURES ARE CERTIFIED EXT AS CONTAINED IN THE PERMANENT RECO	RACTS FROM THE ORIGINAL CHEMICAL AND PHYSICAL TEST RE ORDS OF COMPANY. Mgr. Metallug. Svcs.

Figure A-5. No. 4 Rebar Material Specifications, Test No. NYTCB-5

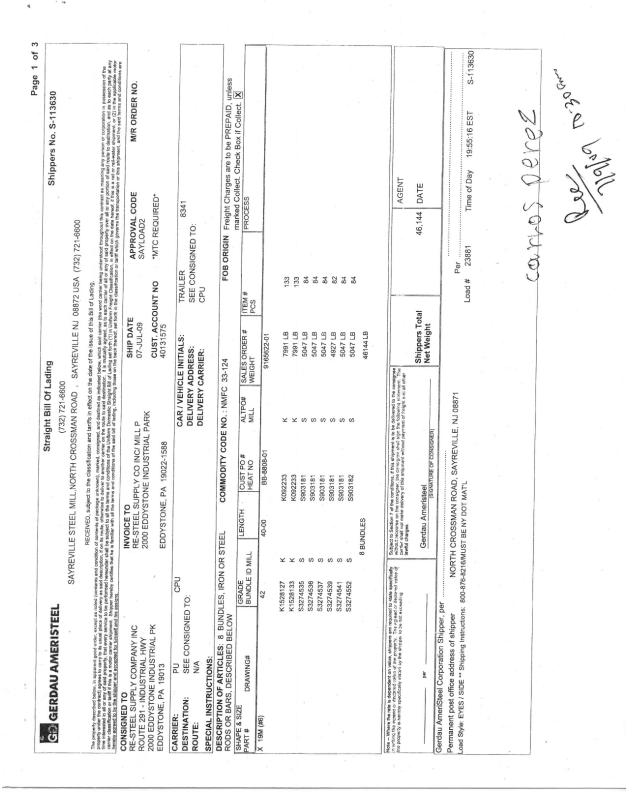


Figure A-6. No. 6 Rebar Material Specifications, Test No. NYTCB-5

THE ABOVE FIGURES ARE CERTIFIED EXTRACTS FROM THE ORIGINAL CHEMICAL AND PHYSICAL TEST RECORDS AS CONTAINED IN THE PERMANENT RECORDS OF COMPANY. W11% CUST P.O. NUMBER Def SP: .477, 12.12MM BB-8808-01 Seller warrants that all material furnished shall comply with specifications stubject to standard published manufacturing variations. NO OTHER WARRANTIES, EXPRESSED OR IMPLIED, ARE MADE BY THE SELLER, AND SPECIFICALLY EXCLUDED ARE WARRANTIES OF MERCHANTABILITY AND FITNESS FOR A PARTICULAR PURPOSE.
If no event shall seller be liable for indirect, consequential or punitive damages arising out of or related to the materials furnished by seller.
Any claim for damages for materials that do not conform to specifications must be made from buyer to seller rimmediately after delivery of same in order to allow the seller the opportunity to inspect the material is question. SALES ORDER Def Gap: .136, 3.45MM KNOXVILLE STEEL MILL 9165622-01 Mgr. Metallurg. Svcs. **CUST. ACCOUNT NO** 40131575 **SHIP DATE** 07/07/09 .05, 1.27MM Chemical and Physical Test Report Def HT: MADE IN UNITED STATES Los Churdst ŏ Bend: RE-STEEL SUPPLY CO INC/ MILL P 2000 EDDYSTONE INDUSTRIAL PARK Yield 79100 PSI, 545.38 MPA Tensile: 109800 PSI, 757.04 MPA %EI: 11.0/8in, 11.0/200MM P S Si Cu Ni Cr Mo V Sn CEqv 011 .069 .22 .44 .12 .08 .018 .031 .004 .632 EDDYSTONE, PA 19022-1588 This material, including the billets, was produced and manufactured in the United States of America ASTM A615/A615M-08B Bhaskar Yalamanchili Gerdau Ameristeel Customer Requirements Not suitable for hot forging applications Quality Director GE GERDAU AMERISTEEL ROUTE 291 - INDUSTRIAL HWY 2000 EDDYSTONE INDUSTRIAL PK EDDYSTONE, PA 19013 420 (60) Mn RE-STEEL SUPPLY COMPANY INC PRODUCED IN: KNOXVILLE
SHAPE + SIZE
GRADE SAYREVILLE STEEL MILL NORTH CROSSMAN ROAD SAYREVILLE NJ 08872 USA (732) 721-6600 42 SHAPE + SIZE X19MM REBAR (# 6) Mechanical Test: K092233

Figure A-7. No. 6 Rebar Material Specifications, Test No. NYTCB-5

Page 2 of 3

SAYREVILLE STEEL MILL NORTH CROSSMAN ROAD SAYREVILLE NJ 08872 USA (732) 721-6600	MADE IN UNITED STATES		S-113630
SA'REVILLE NJ 1887Z USA (732) 721-6600			
SHIP IO RE-STEEL SUPPLY COMPANY INC RE-STEEL S ROUTE 291 - INDUSTRIAL HWY 2000 EDDYSTONE INDUSTRIAL PK EDDYSTONE, PA 19013 EDDYSTONE, PA 19013	INVOICE TO RE-STEEL SUPPLY CO INC/ MILL P 2000 EDDYSTONE INDUSTRIAL PARK EDDYSTONE. PA 19022-1588	SHIP DATE 07/07/09 CUST. ACCOUNT NO	
PRODUCED IN: SAYREVILLE			
SHAPE + SIZE GRADE SPECIFICATION		CHOCK OFFICE	o day
X19MM REBAR (# 6) 420 (60) ASTM A615M GR420 [GR60]		SALES ONDER	CUST P.O. NOMBER
O Mn	Mo V Nb B N Sn Al	Zr Ca C Fov	BB-8808-01
S903181 .44 .67 .007 .055 .19 .43 .11 .06	.038 .024 .001 .0000 .0000 .026	00000	
Mechanical Test: Yield 69766 PSI, 481.02 MPA Tensile: 104445 PSI, 720.12 MPA Mechanical Test: Yield 70368 PSI, 485.17 MPA Tensile: 104114 PSI, 717.84 MPA	%EI: 11.0/8in, 11.0/200MM Bend: OK %EI: 10.0/8in, 10.0/200MM Bend: OK	%//h -3.4L	
PRODUCED IN: SAYREVILLE		1	
SHAPE + SIZE GRADE SPECIFICATION		SALES ODDED	C C HOI C
X19MM REBAR (# 6) 420 (60) ASTM A615M GR420 [GR60]		9165622-01	BD 8908 04
. C Mn P S	Mo V Nb B N Sn Al Ti	Zr Ca C Fov	0-0000-00
	.038 .023 .001 .0000 .0000 .022 .000 .0	00000	
Mechanical Test: Yield 69752 PSI, 480.92 MPA Tensile: 103130 PSI, 711.06 MPA Mechanical Test: Yield 70886 PSI, 488.74 MPA Tensile: 103466 PSI, 713.37 MPA	PA %EI: 11.0/8in, 11.0/200MM Bend: OK Def HT: 0, 0MM PA %EI: 10.0/8in 10.0/200MM Band: OK Def HT: 0, 0MM	%I/h 4.6L	

Page 3 of 3

THE ABOVE FIGURES ARE CERTIFIED EXTRACTS FROM THE ORIGINAL CHEMICAL AND PHYSICAL TEST RECORDS AS CONTAINED IN THE PERMANENT RECORDS OF COMPANY.

This material, including the billets, was produced and manufactured in the United States of America

Quality Director

James 8.8 Georgenest

SAYREVILLE STEEL MILL

Selier warrants that all material furnished shall comply with specifications subject to standard published manufacturing variations. NO OTHER WARRANTIES, EXPRESSED OR IMPLIED, ARE MADE BY THE SELLER, NO SPECIFICALLY EXCLUDED ARE WARRANTIES OF MARCH-MAY RABILITY AND FITNESS FOR A PARTICULAR PURPOSE. In the subject of the standard of or related to the materials standard solid. Seller and the subject of the specifications must be made from buyer to seller immediately after delivery of same in order to allow the seller the mondrinks to income the manufacturing.

Figure A-8. No. 6 Rebar Material Specifications, Test No. NYTCB-5

Atlas ABC Corp (Atlas Tube Chicago) 1855 East 122nd Street Chicago, iflinois, USA 60633 Tel: 773-646-4500 Fax: 773-646-6128



Ref.B/L: 80338055 Date: 04.13.2009 Customer: 98

Sold to

Metals USA-Northeast L.P. (Sto 50 Cabot Blvd East LANGHORNE PA 19047 USA

Shipped to

Jersey Precast Corp. 853 Nottingham Way HAMILTON TOWNSHIP NJ 08 USA

Material: 4.0x Sales order:		0'0"0(4x2).				l No: 4004 se Order:				USA Manufact erial #: 4		SA	
Heat No	С	Mn	P	S	Si	Al	Cu	Cb	Мо	Ni	Cr	v	TI
Г64898	0.22	0.770	0.010	0.009	0.011	0.048	0.030	0.001	0.004	0.010	0.030	0.001	0.001
Bundle No	١	'ield	Tensile	Eln.	.2in		c	ertification	on		C	E: 0.36	
M800102326	- 0	73670 Psi	080780	Psi 3	3 %			STM A50	0-07 GRAI	DE B&C			
Material Note Sales Or.Note										7- A			
Material: 4.0x)'0"0(4x2).				No: 4004			Melted &	USA Manufact erial #: 45	ured in US	SA	*
Heat No	С	Mn	P	S	Si	Al	Cu	Cb	Мо	Ni	Cr	CV _	TI
T64900	0.23	0.780	0.010	0.011	0.011	0.034	0.040	0.001	0.005	0.010	0.030	0.001	0.001
Bundle No	Y	ield	Tensile	Eln.	.2in		C	ertification	on	1	CI	E: 0.37	
M800102320	ō	63960 Psi	077680 I	Psi 35	5 %		A	STM A50	0-07 GRAD	DE B&C			
						4							
Sales Or.Note	4.0x500x40	'0"0(4x2).				No: 4004				USA Manufact		SA	
Sales Or.Note Material: 4.0x4 Sales order: 4	4.0x500x40	'0"0(4x2). M n	P	s				Cb	Melted &	Manufact		SA V	TI
Material: 4.0x4 Sales order: 4	4.0x500x40 464827	Mn	P 0.010	S	Purchas	se Order:	PHI 14472		Melted & Cust Mate	Manufacterial #: 45	600-40		TI 0.001
Material: 4.0x4 Sales order: 4 Heat No	4.0x500x40 464827 C	Mn			Purchas Si 0.011	se Order:	PHI 14472 Cu 0.030	Cb	Melted & Cust Mate	Manufactorial #: 45	Cr 0.030	V	
Sales Or.Note Material: 4.0x4 Sales order: 4 Heat No T64898 Bundle No	4.0x500x40 464827 C 0.220	Mn	0.010	0.009 Eln.:	Purchas Si 0.011	se Order:	O.030	Cb 0.001 ertification	Melted & Cust Mate	Manufactorial #: 45 Ni 0.010	Cr 0.030	V	
Material: 4.0x4 Sales order: 4 Heat No 164898 Bundle No M800102323 Material Note:	4.0x500x40 464827 C 0.220 Y	M n 0.770 ield	0.010 Tensile	0.009 Eln.:	Purchas Si 0.011 2in	se Order:	O.030	Cb 0.001 ertification	Melted & Cust Mate Mo 0.004	Manufactorial #: 45 Ni 0.010	Cr 0.030	V	
Material: 4.0x4 Sales order: 4 Heat No 164898 Bundle No 18800102323 Material Note: Sales Or.Note	4.0x500x40 464827 C 0.220 Y	Mn 0 0.770 ield 73670 Psi	0.010 Tensile 080780 F	0.009 Eln.: 33	Purchas Si 0.011 2in 3 %	Al 0.048	PHI 14472 Cu 0.030 C A	0.001 ertification	Melted & Cust Mate Mo 0.004 0.007 SHIFF NOTARY MY COM	Manufacticerial #: 45 Ni 0.010 DE B&C OFFICIAL RLEY L AU PUBLIC - S' MMISSION E	Cr 0.030 CE SEAL GUSTINIU TATE OF ILL XPIRES:07/70	V 0.001 E: 0.36	0.001
Material Note: Sales Or.Note Material: 4.0x4 Sales order: 4 Heat No 164898 Bundle No M800102323 Material Note: Sales Or.Note	4.0x500x40 464827 C 0.220 Y Quality A:	Mn 0.770 ield 73670 Psi ssurance: /r this report r	0.010 Tensile 080780 F	0.009 Eln.: 33	Purchas Si 0.011 2in 3 %	Al 0.048	PHI 14472 Cu 0.030 C A	0.001 ertification	Melted & Cust Mate Mo 0.004 0.007 SHIFF NOTARY MY COM	Manufacticerial #: 45 Ni 0.010 DE B&C OFFICIAL RLEY L AU PUBLIC - S' MMISSION E	Cr 0.030 CE SEAL GUSTINIU TATE OF ILL XPIRES:07/70	V 0.001 ≘: 0.36	0.001

Figure A-9. Flat Plate Material Specifications, Test No. NYTCB-5

Atlas ABC Corp (Atlas Tube Chicago) 1855 East 122nd Street Chicago, Illinois, USA 60633 Tel: 773-646-4500 Fax: 773-646-6128



Ref.B/L: 80338055 Date: 04.13.2009 Customer: 98

Sold to

Metals USA-Northeast L.P. (Sto 50 Cabot Blvd East LANGHORNE PA 19047 USA

Shipped to

Jersey Precast Corp. 853 Nottingham Way HAMILTON TOWNSHIP NJ 08 USA

Material: 4.0x4.0		"0(4x2).			Materia	l No: 4004	405004000		Made in: Melted &	USA Manufact	ured in U	SA	
Sales order: 46	64827				Purchas	se Order:	PHI 14472	2	Cust Mat	erial #: 45	500-40		
Heat No	С	Mn	P	S	Si	Al	Cu	Cb	Мо	Ni	Cr	V	TI
T64899	0.220	0.800	0.009	0.008	0.012	0.051	0.030	0.001	0.004	0.010	0.020	0.001	0.001
Bundle No	Yie	ld	Tensile	Eln.	2in		C	Certification	on		С	E: 0.36	
M800102331	071	140 Psi	078750 F	Psi 35	5 %		Ā	STM A50	0-07 GRAI	DE B&C			
Material Note: Sales Or.Note:										ear of	Total or		
Material: 4.0x4.0	0x500x40'0	"0(4x2).			Material	No: 4004	105004000			USA Manufact			
Sales order: 46	64827				Purchas	se Order:	PHI 14472		Cust Mat	erial #: 45	500-40		
Heat No	С	Mn	P	s	Si	Al	Cu	Cb	Мо	Ni	Cr	v 1	TI
T64898	0.220	0.770	0.010	0.009	0.011	0.048	0.030	0.001	0.004	0.010	0.030	0.001	0.001
Bundle No	Yie	ld	Tensile	Eln.2	2in		c	ertification	n	Barrie	С	E: 0.36	
M800102324	073	670 Psi	080780 F	rsi 33	%		Ā	STM A50	0-07 GRAI	DE B&C	5.5 1		
										laste	Stark to		
Sales Or.Note:		`x								x			
Sales Or.Note: Material: 4.0x4.0		'0(3x2).				No: 4004	05004800 PHI 14472	-				SA	
Sales Or.Note: Material: 4.0x4.0 Sales order: 46		'0(3x2). Mn	P	S				Сь	Melted &	USA Manufacti		SA V	ті
Sales Or.Note: Material: 4.0x4.0 Sales order: 46 Heat No	64827	, , , , , , ,	P 0.009	S	Purchas	e Order:	PHI 14472		Melted & Cust Mate	USA Manufacto erial #: 45	00-48		TI 0.001
Sales Or.Note: Material: 4.0x4.0 Sales order: 46- Heat No D62523	64827 C	Mn 0.750			Purchas Si 0.010	se Order:	PHI 14472 Cu 0.030	СЬ	Melted & Cust Mate	USA Manufacto erial #: 45	Cr 0.040	V	
Material Note: Sales Or.Note: Material: 4.0x4.0 Sales order: 46 Heat No D62523 Bundle No M800091360 Material Note: Sales Or.Note:	0.210 Yiel	Mn 0.750	0.009	0.009 Eln.2	Purchas Si 0.010	se Order:	PHI 14472 Cu 0.030	Cb 0.002 ertification	Melted & Cust Mate Mo 0.004 on 0-07 GRAD	USA Manufacti erial #: 45 Ni 0.010	Cr 0.040	V	
Sales Or.Note: Material: 4.0x4.0 Sales order: 46 Heat No D62523 Bundle No M800091360 Material Note:	0.210 Yiel 066	Mn 0.750 dd 840 Psi	0.009 Tensile 079220 F	0.009 Eln.2 Psi 33	Purchas Si 0.010 2in %	ee Order: Al 0.040	PHI 14472 Cu 0.030 CA A	0.002 sertification STM A500	Melted & Cust Mate Mo 0.004 n 0-07 GRAE OFFIC ARY PUBLICY CCOMMISSIC	USA Manufacti erial #: 45 Ni 0.010 DE B&C CIAL SEAL AUGUSTII CON EXPIRES	0.040 Cr 0.040 Cl	V 0.001 E: 0.35	0.001
Sales Or.Note: Material: 4.0x4.0 Sales order: 46 Heat No D62523 Bundle No M800091360 Material Note: Sales Or.Note:	0.210 Yiel 066	Mn 0.750 dd 840 Psi	0.009 Tensile 079220 F	0.009 Eln.2 Psi 33	Purchas Si 0.010 2in %	ee Order: Al 0.040	PHI 14472 Cu 0.030 CA A	0.002 sertification STM A500	Melted & Cust Mate Mo 0.004 in 0-07 GRAD OFFIC HIRLEY L ARY PUBLIC COMMISSION of Indicate	USA Manufacti erial #: 45 Ni 0.010 DE B&C CIAL SEAL AUGUSTII CON EXPIRES	0.040 CI O.040 CI O.0	v 0.001 E: 0.35	0.001

Figure A-10. Flat Plate Material Specifications, Test No. NYTCB-5

Atlas ABC Corp (Atlas Tube Chicago) 1855 East 122nd Street Chicago, Illinois, USA

60633 Tel: 773-646-4500 Fax: 773-646-6128



Ref.B/L: 80338055 Date: 04.13.2009 Customer: 98

Sold to

14.

Metals USA-Northeast L.P. (Sto 50 Cabot Blvd East LANGHORNE PA 19047 USA

Shipped to

Jersey Precast Corp. 853 Nottingham Way HAMILTON TOWNSHIP NJ 08 USA

Material: 4.0x	4.0x500x48'0	"0(3x2).							Made in: USA Melted & Manufactured in USA					
Sales order:	464827				Purchas	e Order:	PHI 14472	:	Cust Mat	erial #: 4	500-48			
Heat No	С	Mn	P	S	Si	Al	Cu	Cb	Мо	Ni	Cr	٧	TI	
D62523	0.210	0.750	0.009	0.009	0.010	0.040	0.030	0.002	0.004	0.010	0.040	0.001	0.001	
Bundle No	Yie		Tensile	Eln.	2in		c	ertification	on		CE	E: 0.35		
M800091359		840 Psi	079220 P		%		Ā		0-07 GRAI		E.			
Material Note	:													
Sales Or.Note):													

OFFICIAL SEAL SHIRLEY L AUGUSTINIUS NOTARY PUBLIC - STATE OF ILLINOIS MY COMMISSION EXPIRES:07/11/09

Authorized by Quality Assurance: M Whele The results reported on this report represent the actual attributes of the material furnished and indicate full compliance with all applicable specification and contract requirements.

CE calculated using the AWS D1.1 method.

Metals Service Center Institute

Figure A-11. Flat Plate Material Specifications, Test No. NYTCB-5

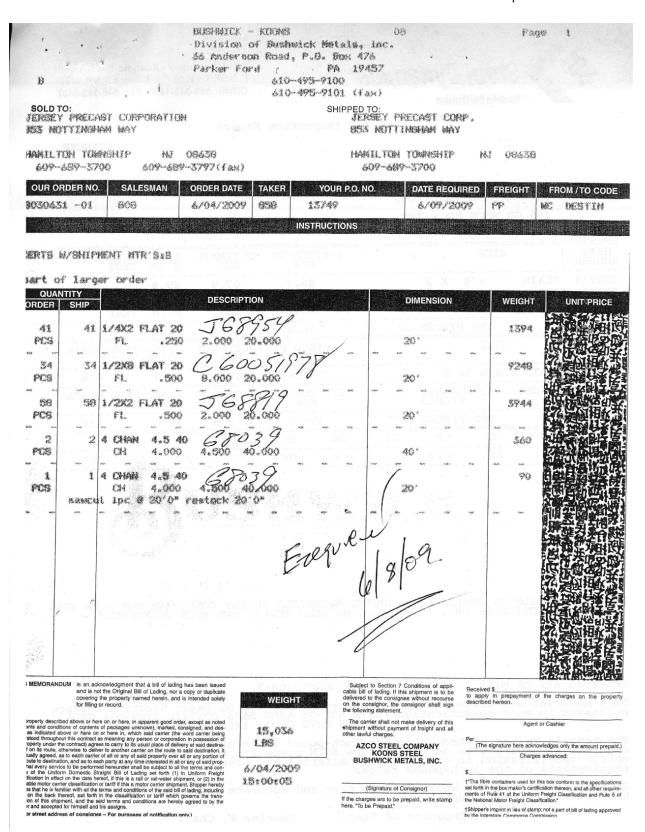


Figure A-12. Flat Plate Material Specifications, Test No. NYTCB-5



Test and Inspection Report

NO. 79781-1

ROANOKE

BUSHWICK METALS, INC.

185 GR NECK RD - 320 GREAT NECK NY 11021-0000

Date 6/04/0

======		=====	======	======	=====:		========	======		======	
HEAT NUMBER		SIZE			1-Y	IELD Pt. KSI	ULTIMATE KSI	E ELONG 8 IN.	BEND TEST	GRADE	
JG8954	FLATS	1/-	4 X 2			48.7	67.9	29.4		A36	
PURCHAS NUMB		}	NUMBER PIECES		2-Y	ELD PT.	ULTIMATE KSI	ELONG 8 IN.	BEND TEST	GRADE	
M0600R	======	150	PIECES	20'		46.4	66.4	30.0	1035	A36	-3
HEAT NUMBER		SIZE			1-Y	ELD Pt. MPA	ULTIMATE MPA	ELONG 203mm	BEND TEST	GRADE	====
JG8954	FLATS	6.4 X	8.00	.0	3	335.8	468.2	29.4		A36	
PURCHAS NUMB			NUMBER PIECES		2-Y]	ELD PT. MPA	ULTIMATE MPA	ELONG 203mm	BEND TEST	GRADE	0.2
M0600R		150	PIECES	20'	3	19.9	457.8	30.0		A36	
С	MN	S	P	SI	CR	NI	MO	CU	v	NB	====
.14	.72	.036	.017	.19	.06	.08	.02	.27 .0	02	001	

MERCURY, RADIUM OR OTHER ALPHA SOURCE MATERIALS IN ANY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL. NO WELD REPAIR HAS BEEN PERFORMED.

Approved ABS QA Mill. Certificate No. 00NN10108-X.

This material was melted and manufactured in the USA by basic Electric Furnace processes to meet specification: ASTM A36-05 ASME SA36 00S741D A709-06A GR36 AASHTO M270 GR 36 IMPACTS WAIVED

The tensile values stated in either inch-pound units or SI units are to be regarded as separate as defined in the ASTM scope for this material. Unless a metric specification is ordered, this material has been tested and meets the requirements of the inch-pound ranges.

This is to certify the above to be a true and accurate report as contained in the records of this company.

Engineer of Tests:

Charles R. Charlton

Figure A-13. Flat Plate Material Specifications, Test No. NYTCB-5

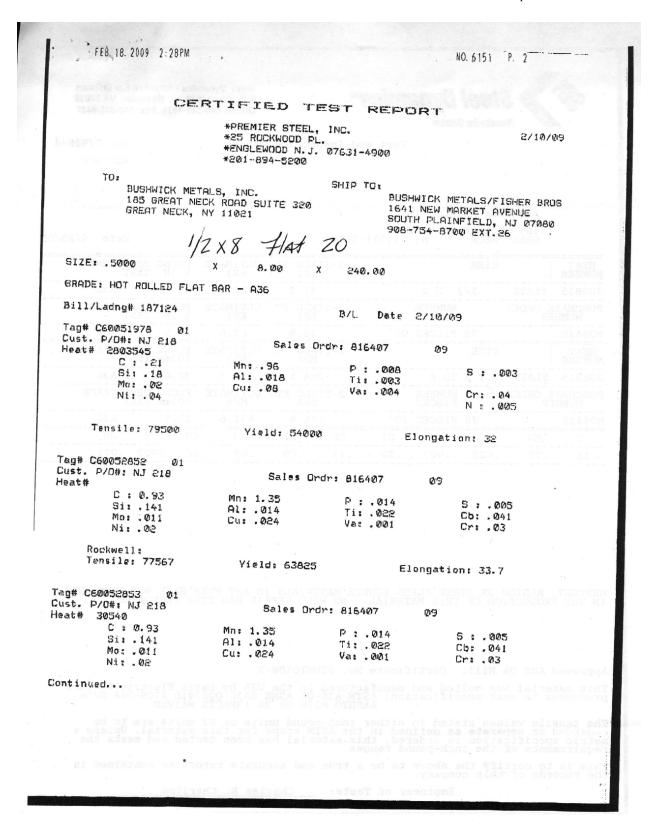


Figure A-14. Flat Plate Material Specifications, Test No. NYTCB-5



Test and Inspection Report

NO. 77926-4 ROANOKE

BUSHWICK METALS, INC.

185 GR NECK RD - 320 GREAT NECK NY 11021-0000

Date 4/20/09

HEAT NUMBER	SIZE			1-Y	IELD Pt.	ULTIMATE KSI	ELONG BEND 8 IN. TEST	GRADE
JG8815 FL	ATS 1/	2 X 2			41.2	62.7	30.0	A36
PURCHASE O NUMBER	RDER	NUMBER PIECES	9.34G	2-Y	IELD PT.	ULTIMATE KSI	ELONG BEND 8 IN. TEST	GRADE
MO442R	78	PIECES	20'		39.8	62.6	27.5	A36
HEAT NUMBER	SIZE	00		1-Y	IELD Pt. MPA	ULTIMATE MPA	ELONG BEND 203mm TEST	GRADE
JG8815 FL	ATS 12.7 X	50.8	799		284.1	432.3	30.0	A36
PURCHASE O NUMBER	RDER	NUMBER PIECES	#80 . s	2-Y	IELD PT.	ULTIMATE MPA	ELONG BEND 203mm TEST	GRADE
MO442R	78	PIECES	20'		274.4	431.6	27.5	A36
C M	N S	P	SI	CR	NI	MO	CU V	NB
.11 .7	5 .028	.007	.20	.11	.09	.02 .	26 .003	.001

MERCURY, RADIUM OR OTHER ALPHA SOURCE MATERIALS IN ANY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL. NO WELD REPAIR HAS BEEN PERFORMED.

Approved ABS QA Mill. Certificate No. 00NN10108-X.

This material was melted and manufactured in the USA by basic Electric Furnace processes to meet specification: ASTM A36-05 ASME SA36 QOS741D A709-06A GR36 AASHTO M270 GR 36 IMPACTS WAIVED

The tensile values stated in either inch-pound units or SI units are to be regarded as separate as defined in the ASTM scope for this material. Unless a metric specification is ordered, this material has been tested and meets the requirements of the inch-pound ranges.

This is to certify the above to be a true and accurate report as contained in the records of this company.

Engineer of Tests: Charles R. Charlton

Figure A-15. Flat Plate Material Specifications, Test No. NYTCB-5

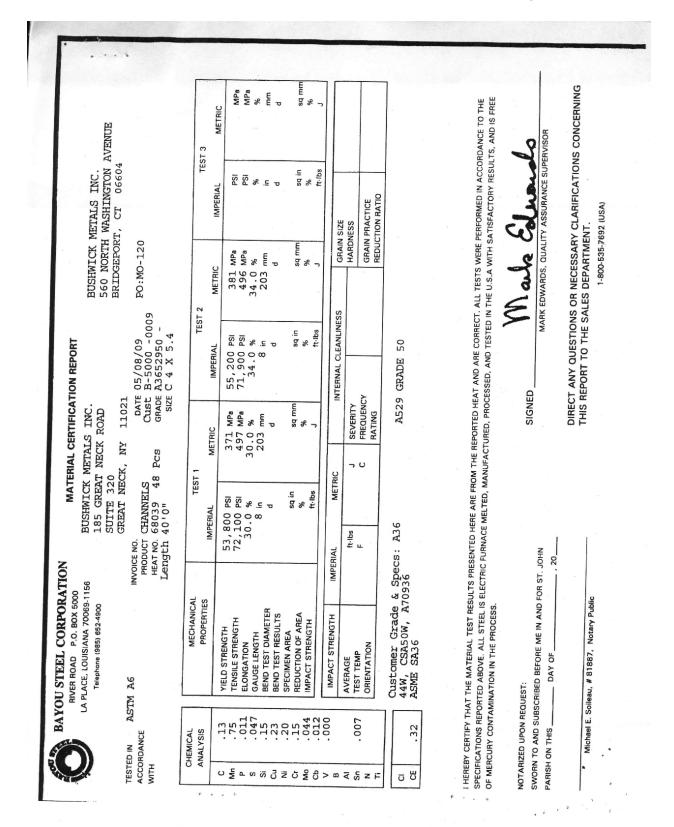


Figure A-16. Flat Plate Material Specifications, Test No. NYTCB-5

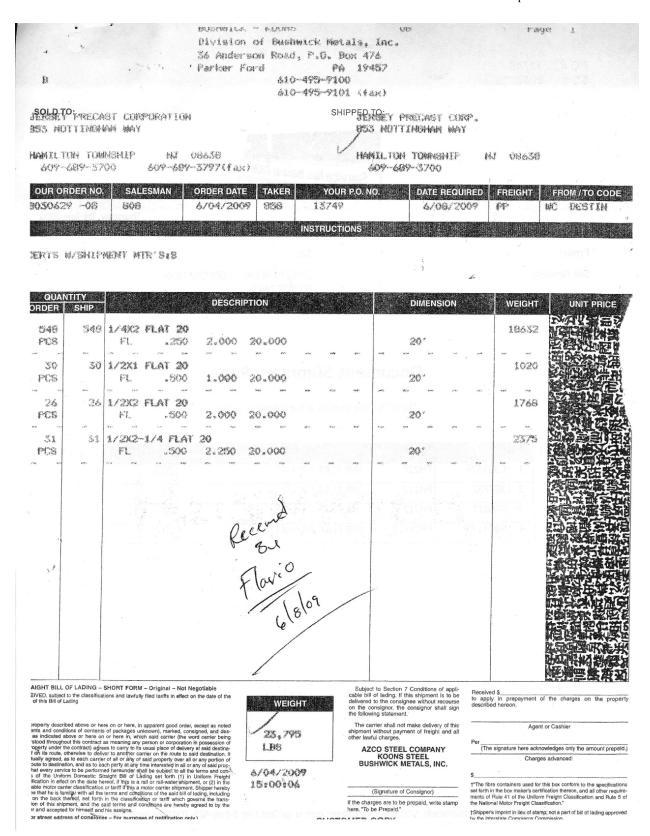


Figure A-17. Flat Plate Material Specifications, Test No. NYTCB-5

PO # 13749 SO# 8030629



Thursday, 04-Jun-2009

From:

Sue Kennedy

To:

JERSEY PRECAST CORPORATION PO BOX 7443 NORTH BRUNSWICK, NJ, USA 08902

Document Summary Page

The MTR's are printed in the following order:

#	Heat#	Item#	Description	Length	Width
1	C901479	FH214	HR FLAT 2 X 1/4	20	
2	JG7737	FH112	HR FLAT 1 X 1/2	20	
3	JG8817	FH212	HR FLAT 2 X 1/2	20	
4	N804173	FH21412	HR FLAT 2-1/4 X 1/2	20	

 $MTRs\ produced\ using\ MetalTrace \textcircled{@. Visit\ www.TraceApps.com\ or\ call\ toll-free\ 1-866-429-7007\ for\ more\ information.}$

Figure A-18. Flat Plate Material Specifications, Test No. NYTCB-5

Page 1 of 2	CUST P.O. NUMBER GRK248-03	CUST P.O. NUMBER GRY246-06 CUST P.O. NUMBER GRY246-04	LAND PHYSICAL TEST RECORDS E BY THE
	SHIP DATE 05/29/09 CUST. ACCOUNT NO 40/200735 SALES ORDER 9156383-03	SALES ORDER 9165383-06 8ALES ORDER 9165383-04	CTS FROM THE ORIGINAL CHEMICA OS OF COMPANY. Mar. Metallurg. Svcs. CHARLOTTE STEEL MILL EXPRESSED OR IMPLIED, ARE MADE
Chemical and Physical Test Report MADE IN UNITED STATES	NOICE TO BUSHWICK KOONS STEEL PO BOX 476 A SANE SA36-08A Lu Ni Cr No V Nb Sn CEqv 6.7239 PS!, 464.01 MPA 961-0200MM	A38-05 & ASME SA36-08A	THE ABOVE FIGURES ARE CERTIFIED EXTRACTS FROM THE ORIGINAL CHEMICAL AND PHYSICAL TEST RECORDS Bhaskar Yalamanchili Custity Director Gentlat was produced and manufactured in the United AS CONTAINED IN THE PERMANENT RECORDS OF COMPANY. County Director Gentlat was precised and manufactured of published manufacturing variations. NO OTHER WARRANTES STORES THE STORE STORE AND SPECIFICALLY EXCLUDED ARE WARRANTES. OF NEREX-WANTES FOR IMPLIED, ARE MADE BY THE claim for damages for materials that do not conform to specifications must be made from buyer to seller immediately after delivery of same in order to allow the seller the opportunity to inspect the material in
GENDAU AMERISTEEL HARIOTTE STEEL MILL 601 LAKEVIEW ROAD HARLOTTE NC 28269 USA 704) 596-0361 HIP TO	USHWICK KOONS STEEL 3 ANDERSON RD (OFF RT 724) 10-495-9162 (DAN) ARKER FORD, PA 19457 CODUCED IN: CHARLOTTE 14PE + SIZE 14PE + SI	SPECIFICATION STM A36-05	imaterial, including the billets, was produced and manufactured in the Bhaskar Yalamanchilis Bhaskar Yalamanchilis Ouality Director Gerdau Americator Gerdau Americator LER, AND SPECIFICALLY EXCLUDED ARE WARRANTES OF Macient shall seller be liable for indirect, consequential or puritive darranges for materials that do not conform to specifications sign.

Figure A-19. Flat Plate Material Specifications, Test No. NYTCB-5



Test and Inspection Report

NO. 73226-

ROANOKE

BUSHWICK-KOONS STEEL GLEN RAHME PO BOX 476 PARKER FORD PA

PA 19457-0476

Date 12/12,

HEAT NUMBER		SIZE			1-YIELD Pt.	ULTIMATE KSI	ELONG BEN 8 IN. TES	D GRADE
JG7737	FLATS	1/	2 X 1		43.3	64.7	30.0	A36
PURCHASI NUMBI	E ORDER ER		NUMBER PIECES		2-YIELD PT. KSI	ULTIMATE KSI	ELONG BEN 8 IN. TES	D GRADE
GRR111B		154	PIECES	20'	46.6	67.9	27.5	A36
HEAT NUMBER		SIZE			1-YIELD Pt. MPA	ULTIMATE MPA	ELONG BENI 203mm TES	GRADE
JG7737	FLATS 1	2.7 X	25.4		298.5	446.1	30.0	A36
PURCHASE	ORDER		NUMBER PIECES		2-YIELD PT. MPA	ULTIMATE MPA	ELONG BENI 203mm TEST	GRADE
GRR111B		154	PIECES	20'	321.3	468.2	27.5	A36
C	MN	S	P	SI	CR NI	MO	CU V	NB
.13	.71	.031	.009	.21	.07 .10	.02 .	31 .002	.001

MERCURY, RADIUM OR OTHER ALPHA SOURCE MATERIALS IN PARTY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL. NO WELD REPAIR, HAST BEEN PERFORMED.

My commission expires April 30, 2010

Approved ABS QA Mill. Certificate No. 00NN10108-X.

This material was melted and manufactured in the USA by basic Electric Furnace processes to meet specification: ASTM A36-05 ASME SA36 QQS741D A709-06A GR36 AASHTO M270 GR 36 IMPACTS WAIVED

The tensile values stated in either inch-pound units or SI units are to be regarded as separate as defined in the ASTM scope for this material. Unless a metric specification is ordered, this material has been tested and meets the requirements of the inch-pound ranges.

This is to certify the above to be a true and accurate the records of this company. report as contained in

Figure A-20. Flat Plate Material Specifications, Test No. NYTCB-5



Test and Inspection Report

NO. 79312-4

ROANOKE

BUSHWICK-KOONS STEEL GLEN RAHME PO BOX 476 PARKER FORD PA

PA 19457-0476

Date 5/22/

======	=====	======	===							
HEAT NUMBER	3	SIZE			1	-YIELD Pt. KSI	ULTIMATE KSI	ELONG 8 IN.	BEND TEST	GRADE
JG8817	FLATS	1/	2 X 2	1851	Ī	41.2	64.5	31.3		A36
PURCHAS NUMB		R	NUMBER PIECES		2	-YIELD PT. KSI	ULTIMATE KSI	ELONG 8 IN.		GRADE
GRR216B		78	PIECES	20'		43.2	64.4	28.8		A36
HEAT NUMBER		SIZE			1	-YIELD Pt. MPA	ULTIMATE MPA	ELONG 203mm		GRADE
JG8817	FLATS	12.7 X	50.8			284.1	444.7	31.3		A36
PURCHAS NUMB		?	NUMBER PIECES	% (3.5) 3 2	2	-YIELD PT. MPA	ULTIMATE MPA	ELONG 203mm		GRADE
GRR216B	5 9 3	78	PIECES	20'		297.9	444.0	28.8		A36
C	MN	S	P	sī	CI	R NI	MO	CU	-====	NB
.13	.75	.030	.006	.21	.0'	7 .08	.02	.27 .0	002 .	002

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and Street

MERCURY, RADIUM OR OTHER ALPHA SOURCE MATERIALS IN ANY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL. NO WELD REPAIR HAS BEEN PERFORMED.

My commission expires April 30, 2010

Approved ABS QA Mill. Certificate No. 00NN10108-X.

This material was melted and manufactured in the USA by basic Electric Furnace processes to meet specification: ASTM A36-05 ASME SA36 QQS741D A709-06A GR36 AASHTO M270 GR 36 IMPACTS WAIVED

The tensile values stated in either inch-pound units or SI units are to be regarded as separate as defined in the ASTM scope for this material. Unless a metric specification is ordered, this material has been tested and meets the requirements of the inch-pound ranges.

This is to certify the above to be a true and accurate poort as contained in the records of this company

Figure A-21. Flat Plate Material Specifications, Test No. NYTCB-5

GING GERDAU AMERISTEEL CAMBRIDGE STEEL MILL 160 ORION PLACE CAMBRIDGE ON NIT 1R9 CAN (519) 740-2488	Chemical and Physical Test Report MADE IN CANADA	N-070597
SHIP TO BUSHWICK FISHER BROS. 1641 NEW MARKET AVE. SOUTH PLAINFIELD, NJ 07080	INVOICE TO	
SHAPE + SIZE	SPECIFICATION	CUST P.O. NUMBER NJ-412R-03
material, including the billets, was produced and manufactured in Canada Bhaskar Yalemanchili Quality Director Gendau Ameristeel Gendau Ameristeel	This material, including the billets, was produced and manufactured in Canada AS CONTAINED IN THE PERMANENT RECORDS OF COMPANY. Bhaskar Yalamanchiii Quality Director Gerdau Ameristeel Gerdau Ameristeel Gerdau Ameristeel Configuration and additions. NO OTHER WARRANTIES EXPRESSED OR IMPLIED, ARE MADE BY THE	L AND PHYSICAL TEST RECORDS

Figure A-22. Flat Plate Material Specifications, Test No. NYTCB-5

	A Committee of the Comm			* *	4 M		458)6	e each
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				4 !		PAGE #	4		
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	JERSEY PRE 853 NOTTIA HAMILTON,	IGHAM WE	ťΥ				PRECAST INGHAM WA I TOWNSHIP.		
SHIP VIA	F. O. B.		-CUSTOME	R P/04	QUOTE	# TERMS	DUG	E DATE	ACK. DATE
TRUCK	DCL		11861		07150	1 .5% 10	NET 30 7/	/20/09	7/16/09
OTY SHIP	ORDERED	DESCRIP	TION				WEIGHT		L.00
	14 19	FLOT 1/	2 X 8 A36	X 2010"	7903	125	5158	e e e e e e e e e e e e e e e e e e e	THE PERSONAL PROPERTY AND THE PERSON OF THE
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Figure A-23. Flat Plate Material Specifications, Test No. NYTCB-5



Test and Inspection Report

NO. 80149-0 ROANOKE

INFRA METALS -VA

1900 BESSEMER ROAD PETERSBURG VA 23805-0000

Date 6/12/09

HEAT NUMBER		SIZE			1-	YIELD Pt. KSI	ULTIMATE KSI	ELONG 8 IN.	BEND TEST	GRADE
JG9370	FLATS	1/.	2 X 2			44.8	66.4	28.8		A36
PURCHAS NUMB	E ORDER ER		NUMBER PIECES		2-	YIELD PT. KSI	ULTIMATE KSI	ELONG 8 IN.		GRADE
V2161		78	PIECES	20'		45.1	67.2	27.5		A36
HEAT NUMBER		SIZE			1-1	YIELD Pt. MPA	ULTIMATE MPA	ELONG 203mm	BEND TEST	GRADE
JG9370	FLATS	12.7 X	50.8			308.9	457.8	28.8		A36
PURCHAS NUMB			NUMBER PIECES		2-3	TIELD PT.	ULTIMATE MPA	ELONG 203mm	BEND TEST	GRADE
V2161		78	PIECES	20'		311.0	463.3	27.5		A36
С	MN	S	P	SI	CR	NI	MO	CU	V	NB
.14	.71	.021	.011	.19	.15	.11	.03	. 29 . (002	.002

MERCURY, RADIUM OR OTHER ALPHA SOURCE MATERIALS IN ANY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL. NO WELD REPAIR HAS BEEN PERFORMED.

Approved ABS QA Mill. Certificate No. 00NN10108-X.

This material was melted and manufactured in the USA by basic Electric Furnace processes to meet specification: ASTM A36-05 ASME SA36 QQS741D A709-06A GR36 AASHTO M270 GR 36 IMPACTS WAIVED

The tensile values stated in either inch-pound units or SI units are to be regarded as separate as defined in the ASTM scope for this material. Unless a metric specification is ordered, this material has been tested and meets the requirements of the inch-pound ranges.

This is to certify the above to be a true and accurate report as contained in the records of this company.

Engineer of Tests: Charles R. Charlton

Figure A-24. Flat Plate Material Specifications, Test No. NYTCB-5

SOLD TO: INFRA-METALS 580 MIDDLETOWN BLVD SUITE D-100 I ANGHORNE DA			SHIP TO: INFRA-METALS 1900 BESSEMER ROAD	AD AD	Cal	Carrier Name or Initials & No. MITCHELL G Load No.	uitals & No.	Point No.	Freigr	Freight Prepaid	epaid carrier Code 4470	
	19047		PETERSBURG, VA		23805 Route	- 1	31712 SCRE EL ODE CENT		90	BL Printed	117 Inted	
1497070574 V9867 107193427 V9599	Size 31/2X 31/2X 2X 2X	Spec 1/2 A36 1/4 A36/36M	Length Pcs 40' 00" 23 40' 00" 81	Weight Heat # 10,212 776260	Tensil 72000 72000	Yield EL 48000 28 49000 27	C Mn	Si .17	P V.	ND CV.	12/17/07 21:22:34 Cr Ni Mo	Mo CE
107193428 V9599 307001909 V9867	2X 2X 1/2X 1	1/4 A36/36 A36	40' 00" 81	10,335 777592		52000 27 52000 25 51000 27 52000 25		.16	.010		.14 .11	.03 .37
307002374 V9867	х9 х9	3/8 A36	н	10,132 871251			.14 .79	.18	.040 .020 .004	3 .00 .37	.18 .10	.03 .33
Grade A36 meets all requirements for grades ASTM A36-05, A709-06a GR36(250), ASME SA36-04(250), and AASHTO M270-05 GR36(250) Grade A36/A36M meets all requirements for grades ASTM A36-05, A529-05 GR50(345), A709-06a GR36, (250) ASME SA36-04(250) and AASHTO M270-05 GR36(250) TARP!! INFRA TRK NO TARP. CALL CUST PRIOR TO LOADING FOR APPT. VERT/HOR BLK SHIP 20' TOGETHER. 40' TOGETHER FILL LOAD W/ 20'PF/RAIL\$1.19CWT THRU9/30/08	rements for grader grader for the form the TARP!	Grades ASTM A36-05, A709- 38 for grades ASTM A36-05, TARP!! INFRA TRK NO TARP. SHIP 20' TOGETHER. 40' TOG	Tors 22.887 Tors	342 45,774 Tons 22.887 -06a GR36(250), ASME SA36-04(250), and AASHTO M270 AS29-05 GR50(345), A709-06a GR36, (250) ASME SA36- CALL CUST PRIOR TO LOADING FOR APPT. VERT/HOR BLK BETHER FILL LOAD W/ 20'PP/RAIL\$1.19CWT THRU9/30/08	Tons SA36-04(2 A709-06a G LOADING FO	Tons 22.887 3 SA36-04(250), and AASHTO M270 A709-06a GR36, (250) ASME SA36-1 LOADING FOR APPT. VERT/HOR BLK 20'PP/RAIL\$1.19CWT THRU9/30/08	AASHTO M2 ASME SA31 ERT/HOR BI	70-05 GR36 6-04(250)a LK 18	(250) and AASHTO	M270-05 G	R36 (250)	
	GRADE	. A36/36M ME	GRADE A36/36M MEETS ASTM A36-05 AND ASTM A529-50 - A36 MEETS ASTM A36-05	ND ASTM A529-	-50 - A36	MEETS ASTA	M A36-05					
		:										
WELDING OR WELD REPAIR WAS NOT PERF MELTED AND MANUFACTURED IN THE U.S.A. MERCURY, RADIUM OR ALPHA SOURCE MAT	REPAIR WAS P FACTURED IN OR ALPHA SOL	OT PERFO THE U.S.A. JRCE MATE	IR WAS NOT PERFORMED ON THIS MATERIAL. JRED IN THE U.S.A. PHA SOURCE MATERIALS IN ANY FORM HAVE NOT BEEN USED IN THE PRODUCTION OF THIS MATERIAL.	IATERIAL. JRM HAVE NO	OT BEEN	USED IN 1	THE PRO	DUCTION	OF THIS N	MATERIAL		
NUCOR 300 STEEL MILL ROAD DARLINGTON, SC 29540 Permanent post office address of shipper SLS-FRAA08 REV DATE: 0771607 REVA APPROVALRU.								WE HEREBY C	WE HEREBY CERTIFY THAT THE ABOVE FIGURES ARE CORRECT AS CONTAINED IN THE RECORDS OF NUCOR STEEL. JAMES H. BLEW H. M.	IN THE RECORDS OF NUC. JAMES H. BLEW H. RECORDS OF NUC. JAMES H. BLEW	FIGURES ARE	CORRECT

Figure A-25. Flat Plate Material Specifications, Test No. NYTCB-5

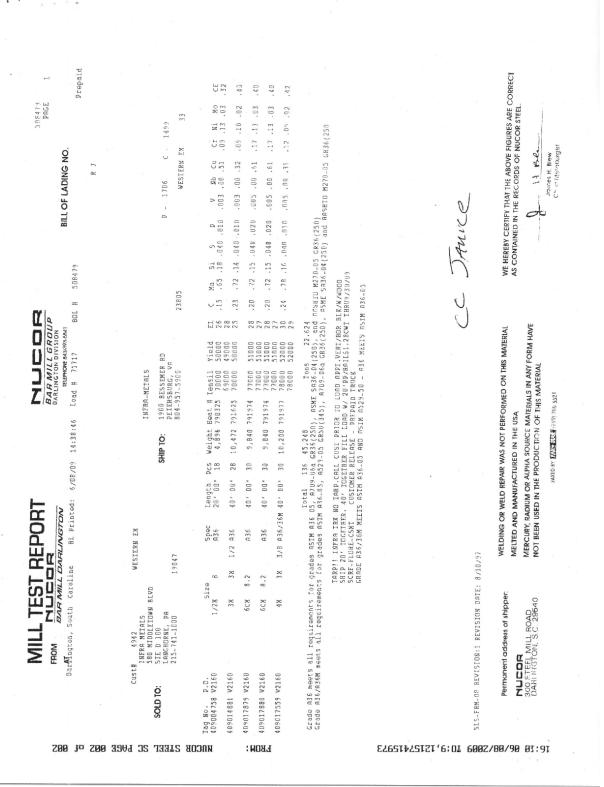


Figure A-26. Flat Plate Material Specifications, Test No. NYTCB-5

CHANGOTE TELL MADE IN UNITED STATES					
INVOICE TO STEEL AND PIPE SUPPL	GERDAU AMERISTEEL GOTARLOTTE STEELMILL SECTION STEELMILL SECTION STEELMILL	Chemical and Phyman MADE IN UNI	ysical Test Report TED STATES		C-524824
INVOICE TO STEEL (AND PIPE SUPPL STEEL (AND PIPE SUPPL PO BOX 1688 MANHATTAN, KS 66505- MANHATTAN, KS 6505- MANHATTAN, KS 66505- MANHATTAN, KS 66505	. USA	100 p. 100 p.	240	ū	4
SPECIFICATION	AND PIPE SUPPY CO INC V CENTURY PARKWAY 5185 SITURY, KS, 66031	INVOICE TO STEEL AND PIPE SUPPLY CO. INC. PO BOX 1888 MANHATTAN, KS 66505-1688	H. 80 D.	IP DATE 29/06 ST. ACCOUNT NO 30833	
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manufactured in the United TYalamandhiii Director Ameristeel	2 2 4	Mo V Nb Sn .020 .002 <.008 .013 %EI: 26.5/8in, 26.5/200MN	33	10-875(100	10-5920110054
and manufactured in the United basker Yalamanchiii usify Director erday Ameristeel	9 4				
manufactured in the United Yelemenchili Director Ameristeel	i	K .		ï	ţ
manufactured in the United TYalamandhili Director Ameristical					
Bhaskar Yalamandhiii Quality Director Gerdau Amerished Gerdau Amerished	sr Notes M A3G-00A & ASTM A709 GR36 Tail, Including the billicts, was produced and manufactured in the		GURES ARE CERTIPIED EXTRAC D IN THE PERMANENT RECORDS	TS FROM THE ORIGINAL CHEM OF COMPANY.	CAL AND PHYSICAL TEST RECORDS
	the	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	hy Capter	Mgr. Metallurg. Svcs. CHARLOTTE STEEL MILL	

Figure A-27. Steel Anchor Rod Material Specifications, Test No. NYTCB-5

Appendix B. Vehicle Center of Gravity Determination

	NYTCB-5		Vehicle:	Ram (2270)	P)	
			Vehicle C	G Determina	ation	
VEHICLE	Equipment	Weight	Long CG	Vert CG	HOR M	Vert M
+	Unbalasted Truck(Curb)	5109	61.56149	28.08136	314517.7	143467.7
+	Brake receivers/wires	5	110	52	550	260
+	Brake Frame	5	38	26	190	130
+	Brake Cylinder (Nitrogen)	28	74	27	2072	756
+	Strobe/Brake Battery	5	79	32	395	160
+	Hub	22	0	14.8125	0	325.875
+	CG Plate (EDRs)	6	55	32	330	192
-	Battery	-41	-7	40	287	-1640
-	Oil	-6	10	17	-60	-102
-	Interior	-73	66	23	-4818	-1679
-	Fuel	-167	114.5	18	-19121.5	-3006
-	Coolant	-13	-20	36	260	-468
-	Washer fluid	-5	-16	35	80	-175
BALLAST	Water	110	114.5	18	12595	1980
	Misc. DTS	20	70	29	1400	580
	Misc.				0	0
					308677.2	140781.6
	TOTAL WEIGHT	5005			61.67376	28.12818
wheel base	140.25	Calculated	Test Inertia	al Weight		
	MASH Targets	Targets		CURRENT	Difference	
	Test Inertial Weight	5000		5005	5.0	
	Long CG	62		61.67	-0.32624	
	Vert CG	28		28.13	0.12818	
	Note, Long. CG is measu	ured from fro	ont axle of	test vehicle		

Curb Weight			
	Left	Rig	jht
Front	1	523	1346
Rear	1	090	1150
FRONT	2	869	
REAR		240	
TOTAL		109	
-			

Actual test	t iner	tial we	ight
(from scales)			
	Left		Right
Front		1394	1364
Rear		1100	1095
		•	•
FRONT		2758	
REAR		2195	
TOTAL		4953	1

Figure B-1. Vehicle Mass Distribution, Test No. NYTCB-5

Appendix C. Vehicle Deformation Records

VEHICLE PRE/POST CRUSH FLOORPAN - SET 1

TEST:	NYTCB-5	Note: If impact is on driver side need to
VEHICLE:	Ram (2270P)	enter negative number for Y

POINT	Х	Υ	Z	Х	Y'	Z'	DEL X	DEL Y	DEL Z
1	26.25	11.75	0	26	11.5	1.5	-0.25	-0.25	1.5
2	26.5	16.75	-1.25	26.5	17.75	1.25	0	1	2.5
3	31.75	22.5	-1.5	25.25	22.25	4.25	-6.5	-0.25	5.75
4	30.5	28	0	26.75	28.75	2.75	-3.75	0.75	2.75
5	22.5	10.5	-1.25	22.5	10.25	-0.5	0	-0.25	0.75
6	25.25	17	-4	24.25	17	-2.25	-1	0	1.75
7	26.25	22.75	-6	21	22.75	-3.75	-5.25	0	2.25
8	27	30.25	-4	25	29.25	-2.5	-2	-1	1.5
9	15.5	3.5	-3	15.25	3.5	-2.75	-0.25	0	0.25
10	17.5	10.75	-4	17.5	10.75	-3.25	0	0	0.75
11	19.5	16	-8	19	15.75	-7.25	-0.5	-0.25	0.75
12	20.25	22.25	-9	19	22.25	-8.25	-1.25	0	0.75
13	20.5	28.75	-8.75	20	29	-8.75	-0.5	0.25	0
14	11	3	-3.5	10.75	3	-3	-0.25	0	0.5
15	14.75	10.75	-6.5	15	10	-6.25	0.25	-0.75	0.25
16	16.5	16.75	-9.25	16.5	16.5	-9	0	-0.25	0.25
17	16.75	24	-9	16.75	24	-9.25	0	0	-0.25
18	16.5	29.25	-8.75	16.5	29.25	-9	0	0	-0.25
19	7.5	4.5	-3.75	7.25	4.5	-3.75	-0.25	0	0
20	10	11.75	-9.25	10	11.5	-9.25	0	-0.25	0
21	10	18.5	-9	10.25	18	-9.25	0.25	-0.5	-0.25
22	10.5	24.5	-8.5	10	24.5	-9.25	-0.5	0	-0.75
23	10.25	29.5	-8.5	10.5	29	-8.75	0.25	-0.5	-0.25
24	1.25	4.75	-3.25	1.25	4.75	-3	0	0	0.25
25	0.75	11.75	-5.25	0.75	11.5	-5.25	0	-0.25	0
26	0.75	17.25	-5	0.75	17	-5	0	-0.25	0
27	0.75	22.25	-4.75	0.75	22	-4.75	0	-0.25	0
28	0.75	28.25	-4.5	0.75	28	-4.5	0	-0.25	0
29							0	0	0
30							0	0	0
31							0	0	0

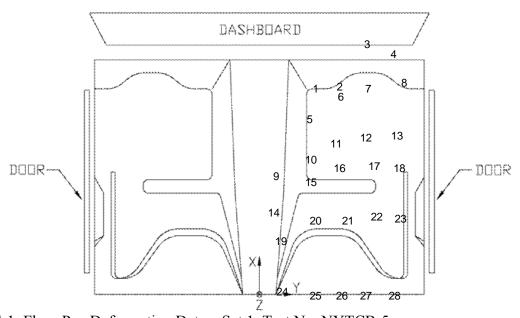


Figure C-1. Floor Pan Deformation Data – Set 1, Test No. NYTCB-5

VEHICLE PRE/POST CRUSH FLOORPAN - SET 2

TEST:	NYTCB-5	Note: If impact is on driver side need to
VEHICLE:	Ram (2270P)	enter negative number for Y

POINT	Х	Υ	Z	X	Υ'	Z'	DEL X	DEL Y	DEL Z
1	49	17.5	0	48.25	17.5	1.25	-0.75	0	1.25
2	52	22.5	-1.5	49	23.25	0.75	-3	0.75	2.25
3	54.5	29	-2.25	47.25	28.25	3.25	-7.25	-0.75	5.5
4	53	34.5	-1.5	49.25	35	1.5	-3.75	0.5	3
5	45.25	16.25	-1.25	45	16.5	-0.25	-0.25	0.25	1
6	48	23.5	-4.25	47	23	-2.75	-1	-0.5	1.5
7	49.25	28.75	-6.75	44	28.5	-4	-5.25	-0.25	2.75
8	50	36.5	-5.25	47.75	35.25	-4	-2.25	-1.25	1.25
9	38.25	9.75	-2.25	38.25	9.75	-2	0	0	0.25
10	40.75	17	-3.75	40.25	16.75	-3	-0.5	-0.25	0.75
11	42.5	22	-8	42	21.75	-7.5	-0.5	-0.25	0.5
12	43.25	28.75	-9.75	42	28.5	-9	-1.25	-0.25	0.75
13	43.5	35	-10	43.25	34.25	-10.25	-0.25	-0.75	-0.25
14	34	9.25	-2.75	33.75	9.25	-2.25	-0.25	0	0.5
15	38	17	-6.5	38	15.75	-6	0	-1.25	0.5
16	39.75	23	-9.75	39.5	22.75	-9.5	-0.25	-0.25	0.25
17	39.75	29.75	-9.75	39.75	29	-10	0	-0.75	-0.25
18	39.75	35	-10	39.75	35	-10.25	0	0	-0.25
19	30.5	11	-3	30.25	10.75	-3	-0.25	-0.25	0
20	33.5	18.25	-9.25	33.25	17.5	-9.25	-0.25	-0.75	0
21	33.75	24.25	-9.5	33.5	24.25	-9.75	-0.25	0	-0.25
22	33.75	30.75	-9.5	33.5	30.5	-10	-0.25	-0.25	-0.5
23	33.75	36	-9.75	33.75	35.5	-10	0	-0.5	-0.25
24	24.25	11	-2.5	24.25	11	-2.25	0	0	0.25
25	23.25	18.25	-5.25	23.75	17.75	-5	0.5	-0.5	0.25
26	24	23.5	-5.5	24	23	-5.25	0	-0.5	0.25
27	24	28.75	-5.5	24	28.5	-5.5	0	-0.25	0
28	24	34.5	- 5.75	24	34.5	-6	0	0	-0.25
29							0	0	0
30							0	0	0
31							0	0	0

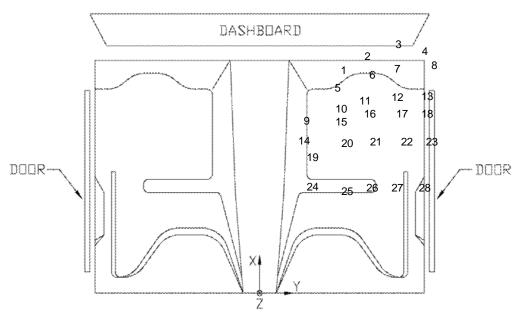
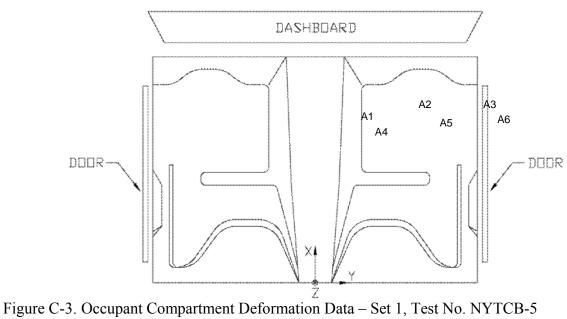


Figure C-2. Floor Pan Deformation Data – Set 2, Test No. NYTCB-5

VEHICLE PRE/POST CRUSH INTERIOR CRUSH - SET 1

TEST:	NYTCB-5	Note: If impact is on driver side need to
VEHICLE:	Ram (2270P)	enter negative number for Y

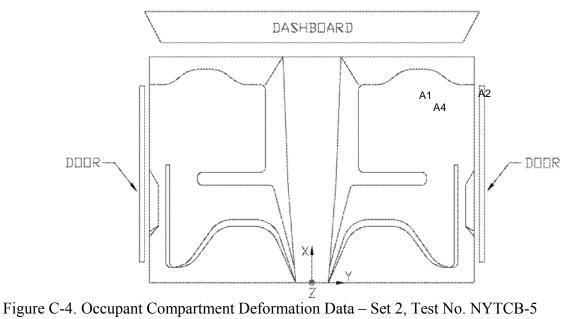
	POINT	Х	Υ	Z	Х	Y'	Z'	DEL X	DEL Y	DEL Z
	A1	28.5	11	23.5	29.5	11.25	23.75	1	0.25	0.25
DASH	A2	30.5	23.25	22.25	30	23.25	22.5	-0.5	0	0.25
	А3	30.5	37	22	30.25	37	22.5	-0.25	0	0.5
ĕ	A4	26	14	11.5	25.75	14	11.75	-0.25	0	0.25
	A5	27.5	27.75	15.5	27.5	27.25	15.75	0	-0.5	0.25
	A6	28	40	16.25	28	39.75	16.75	0	-0.25	0.5
	B1	40.5	43	-1.5	40.5	42.5	-1.25	0	-0.5	0.25
SIDE	B2	38	43	-5	38	42.5	-4.5	0	-0.5	0.5
0, 7	В3	35.5	43	-1	35.75	42.25	-0.5	0.25	-0.75	0.5
	C1	28	44	17.75	28.5	43	17.75	0.5	-1	0
IMPACT SIDE DOOR	C2	14.25	44	19.25	13.75	45.5	19.25	-0.5	1.5	0
OR OR	C3	2.25	44	20	1.5	46.5	19.5	-0.75	2.5	-0.5
8 8	C4	31.5	45.5	-0.25	30.75	44.5	-0.25	-0.75	-1	0
₽	C5	18	45	0.25	17.5	45	0.75	-0.5	0	0.5
_	C6	0.5	45.25	-1.75	0.25	45.75	-1	-0.25	0.5	0.75
	D1							0	0	0
	D2							0	0	0
	D3	This section	n is not ap	plicable				#VALUE!	0	0
	D4							0	0	0
	D5							0	0	0
	D6							0	0	0
டி	D7							0	0	0
ROOF	D8							0	0	0
ď.	D9							0	0	0
	D10							0	0	0
	D11							0	0	0
	D12							0	0	0
	D13							0	0	0
	D14							0	0	0
	D15							0	0	0



VEHICLE PRE/POST CRUSH INTERIOR CRUSH - SET 2

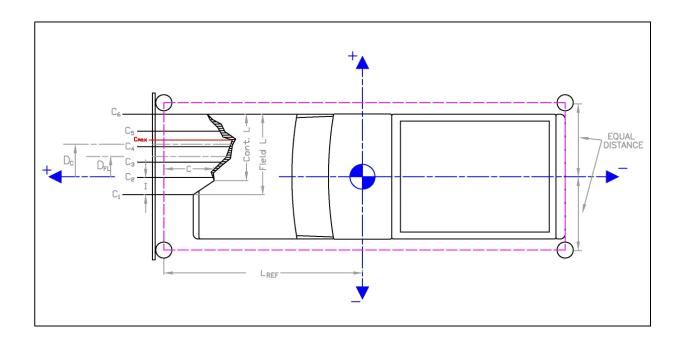
TEST:	NYTCB-5	Note: If impact is on driver side need to
VEHICLE:	Ram (2270P)	enter negative number for Y

	POINT	Х	Y	Z	X	Y'	Z	DEL X	DEL Y	DEL Z
	A1	49.75	24	24	49.75	24.25	24.25	0	0.25	0.25
DASH	A2	50.25	36.5	22	50.5	36.5	22.25	0.25	0	0.25
	А3	50.25	49.75	20.75	50.5	49.5	21.5	0.25	-0.25	0.75
ă	A4	46.75	27	12.25	47	27	12.5	0.25	0	0.25
	A5	47.25	40.5	14.75	47.5	41	15.25	0.25	0.5	0.5
	A6	48.25	52.75	14.75	48.5	52.75	15.5	0.25	0	0.75
	B1	61.5	54.75	-3	62	54.25	-2.75	0.5	-0.5	0.25
SIDE	B2	59.25	54.75	-6	60	54	-6	0.75	-0.75	0
0 4	B3	56.75	54.75	-2.25	57	54.5	-2.25	0.25	-0.25	0
ш	C1	49.25	57	16	49.75	57.25	16.25	0.5	0.25	0.25
	C2	34.5	57	17.5	35.25	58	17.5	0.75	1	0
IMPACT SIDE DOOR	C3	21.75	57	17.75	22.75	58.75	18	1	1.75	0.25
8 8	C4	52.5	56.25	-2	53	56	-2	0.5	-0.25	0
₽	C5	39	56.75	-1	39.5	57	-1	0.5	0.25	0
_	C6	21.25	56.25	-3.5	22.25	56.5	-3.25	1	0.25	0.25
	D1							0	0	0
	D2							0	0	0
	D3	This section	n is not ap	plicable				#VALUE!	0	0
	D4							0	0	0
	D5							0	0	0
	D6							0	0	0
டி	D7							0	0	0
ROOF	D8							0	0	0
ď.	D9							0	0	0
	D10							0	0	0
	D11							0	0	0
	D12							0	0	0
	D13							0	0	0
	D14							0	0	0
	D15							0	0	0



 Date:
 9/10/2009
 Test Number:
 NYTCB-5

 Make:
 Dodge
 Model:
 Ram (2270P)
 Year:
 2003



Distance from C.G. to reference line - L_{REF} : 119.5 (3035)

Width of contact and induced crush - Field L: 22.37 (568)

Crush measurement spacing interval (L/5) - I: 4.474 (114)

Distance from center of vehicle to center of Field L - D_{FI} : 27.435 (697)

Width of Contact Damage: 22.37 (568)

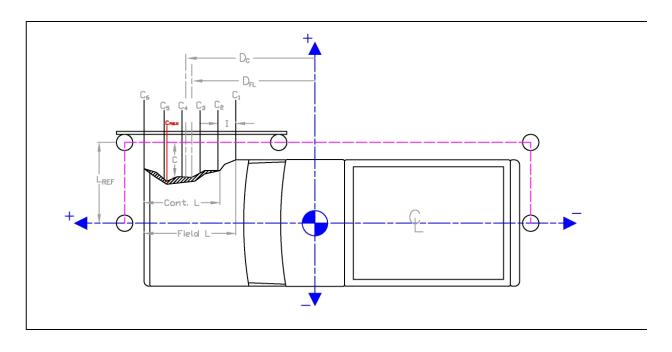
Distance from center of vehicle to center of contect damage - D_{C} : 27.43 (697)

	Crush Measurement		Late	eral	Origina	Profile	Dist. Be	etween	Act	Actual	
			Loca	Location		rement	Ref. I	ines	Cr	Crush	
	in.	(mm)	in.	(mm)	in.	(mm)	in.	(mm)	in.	(mm)	
C_1	21	(533)	16.25	(413)	11.797	(300)	8.0762	(205)	1.1269	(29)	
C_2	25.5	(648)	20.724	(526)	12.625	(321)			4.7988	(122)	
C_3	33	(838)	25.198	(640)	14.063	(357)			10.861	(276)	
C_4	NA	NA	29.672	(754)	15.906	(404)			NA	NA	
C_5	NA	NA	34.146	(867)	19.25	(489)			NA	NA	
C_6	NA	NA	38.62	(981)	29	(737)			NA	NA	
C_{MAX}	44	(1118)	29	(737)	15.688	(398)			20.236	(514)	

Figure C-5. Exterior Vehicle Crush (NASS) - Front, Test No. NYTCB-5

 Date:
 9/10/2009
 Test Number:
 NYTCB-5

 Make:
 Dodge
 Model:
 Ram (2270P)
 Year:
 2003



	Crush Measurement		Crush Longitudinal		Original	Profile	Dist. l	Between	Actual		
			Loc	Location		rement	Ref.	Lines	Crush		
	in.	(mm)	in.	(mm)	in.	(mm)	in.	(mm)	in.	(mm)	
C_1	8	(203)	-125.8	-(3196)	15.688	(398)	-4	-(102)	-3.688	-(94)	
C_2	NA	NA	-80.33	-(2040)	10.5	(267)		·	NA	NA	
C_3	6.5	(165)	-34.83	-(885)	11.604	(295)			-1.104	-(28)	
C_4	7.75	(197)	10.67	(271)	11.25	(286)			0.5	(13)	
C_5	NA	NA	56.17	(1427)	10.5	(267)			NA	NA	
C_6	NA	NA	101.67	(2582)	36.125	(918)			NA	NA	
C_{MAX}	18.5	(470)	79	(2007)	11.25	(286)			11.25	(286)	

Figure C-6. Exterior Vehicle Crush (NASS) - Side, Test No. NYTCB-5

Appendix D. Accelerometer and Rate Transducer Data Plots, Test No. NYTCB-5

Longitudinal CFC 180 10 msec Extracted Average Acceleration - EDR-4

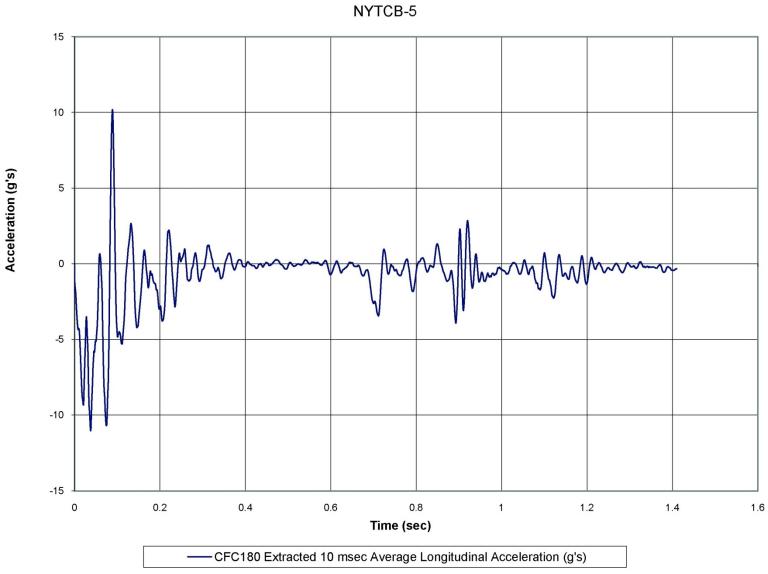


Figure D-1. 10-ms Average Longitudinal Deceleration (EDR-4), Test No. NYTCB-5

Figure D-2. Longitudinal Occupant Impact Velocity (EDR-4), Test No. NYTCB-5

108

Figure D-3. Longitudinal Occupant Displacement (EDR-4), Test No. NYTCB-5

109

Lateral CFC 180 10 msec Extracted Acceleration - EDR-4

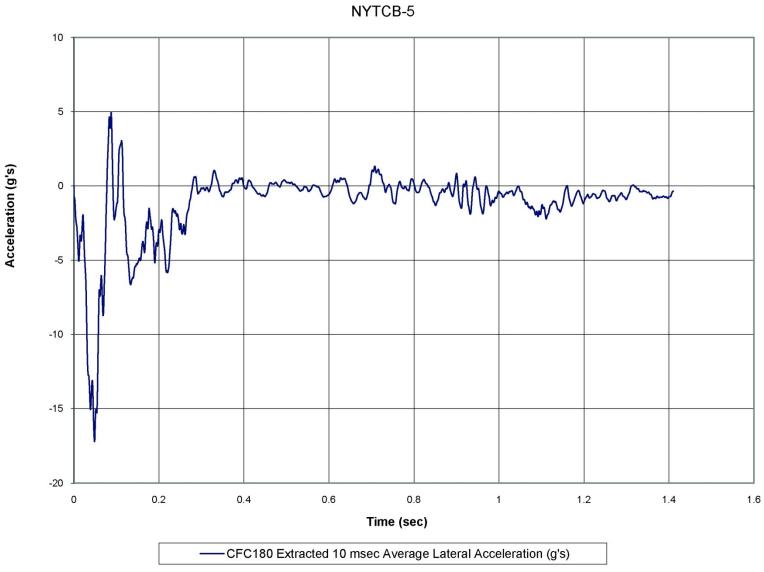


Figure D-4. 10-ms Average Lateral Deceleration (EDR-4), Test No. NYTCB-5

Figure D-5. Lateral Occupant Impact Velocity (EDR-4), Test No. NYTCB-5

Lateral Change in Displacement - EDR-4

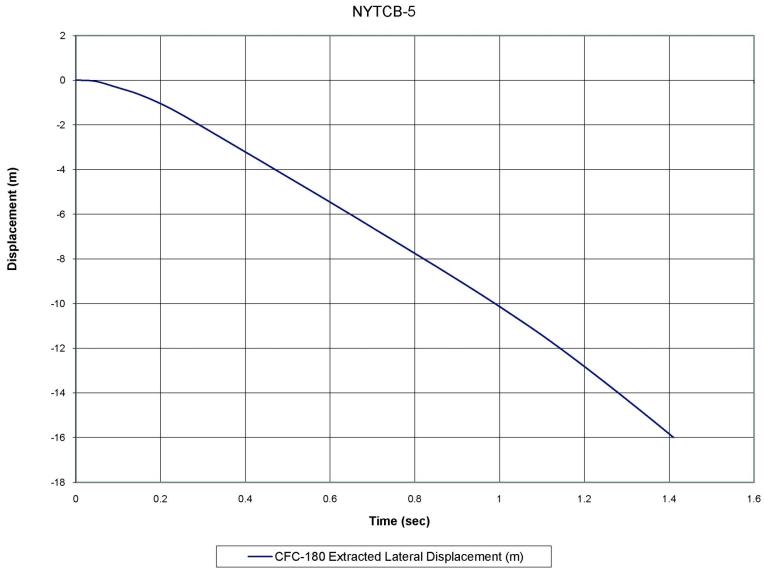


Figure D-6. Lateral Occupant Displacement (EDR-4), Test No. NYTCB-5



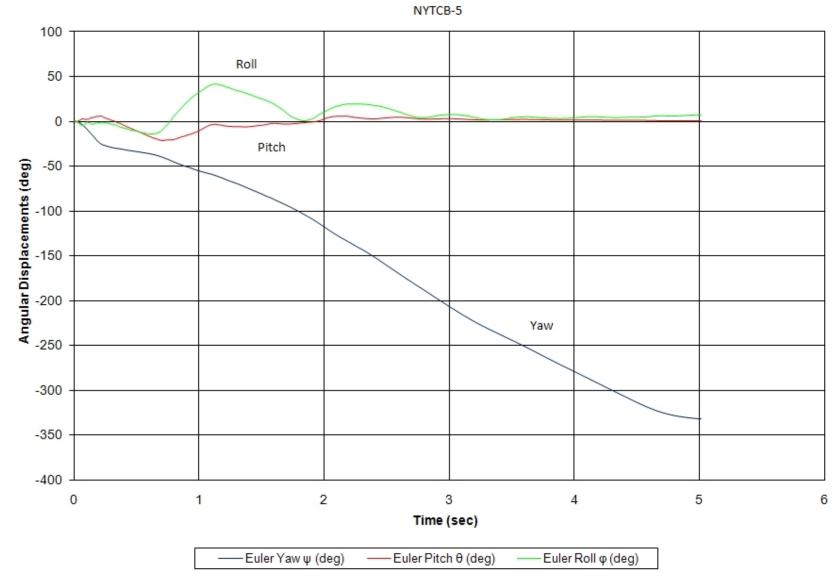


Figure D-7. Vehicle Angular Displacements (EDR-4), Test No. NYTCB-5

Acceleration Severity Index (ASI) - EDR-4



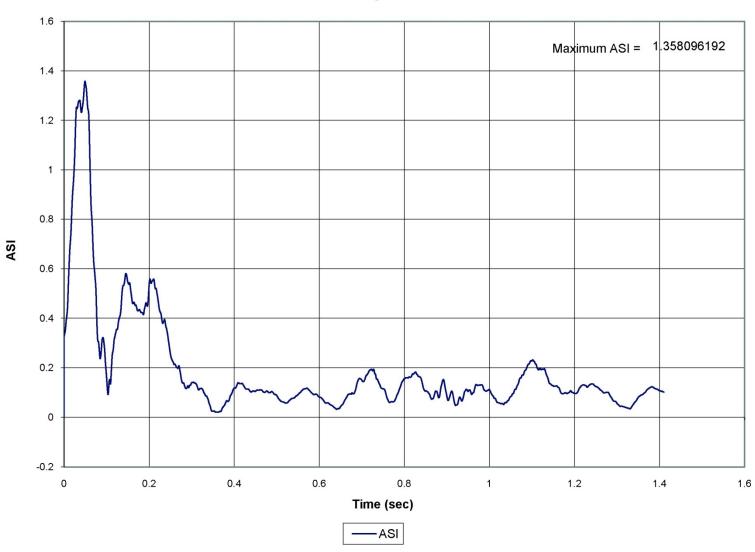


Figure D-8. Acceleration Severity Index (EDR-4), Test No. NYTCB-5

Lateral CFC 180 10 msec Extracted Acceleration - DTS

NYTCB-5

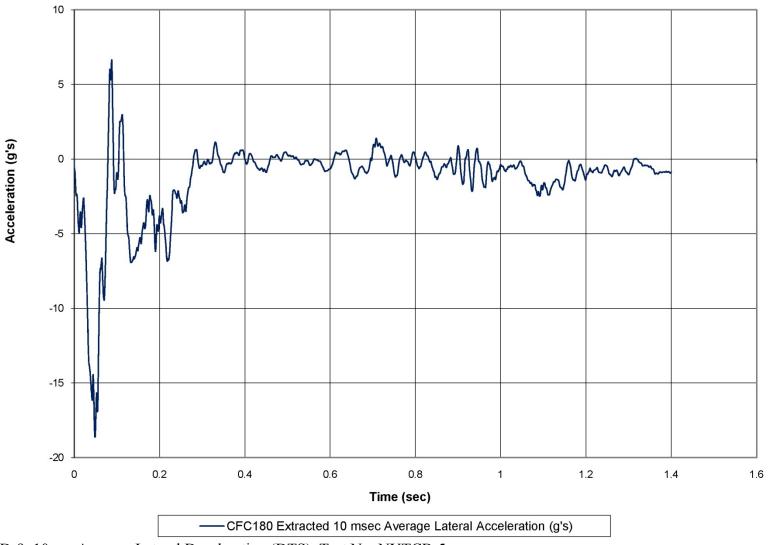


Figure D-9. 10-ms Average Lateral Deceleration (DTS), Test No. NYTCB-5

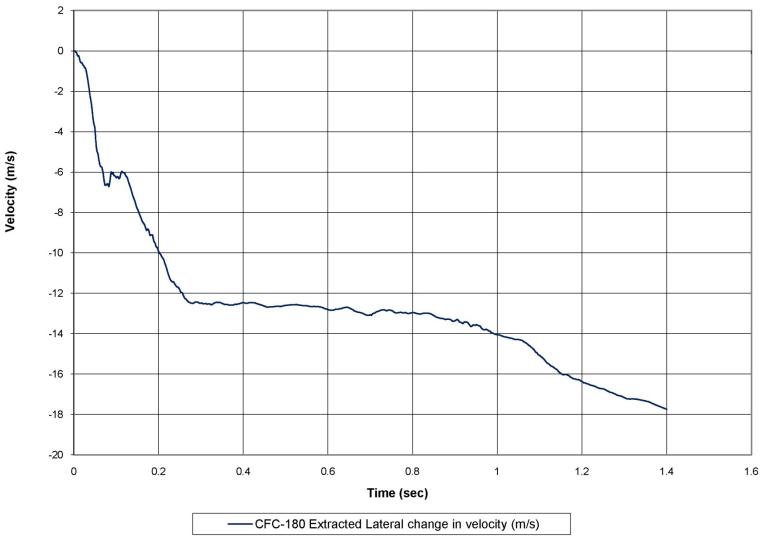


Figure D-10. Lateral Occupant Impact Velocity (DTS), Test No. NYTCB-5

Lateral Change in Displacement - DTS

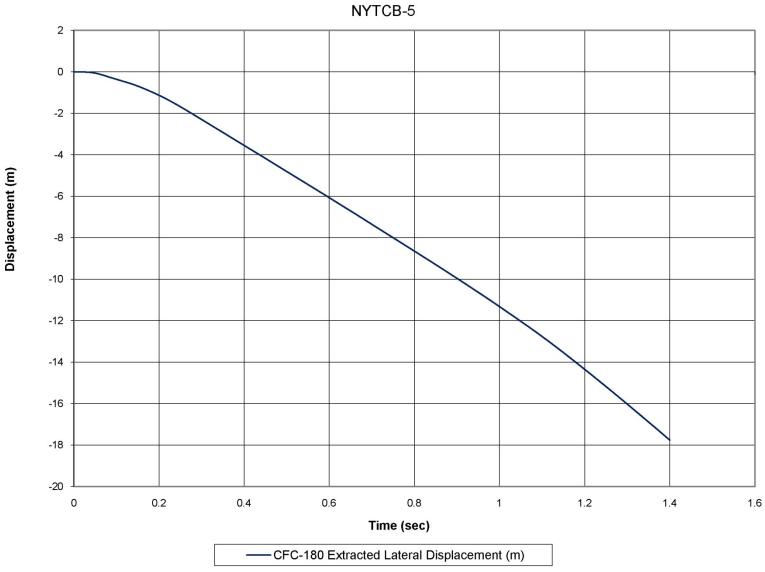
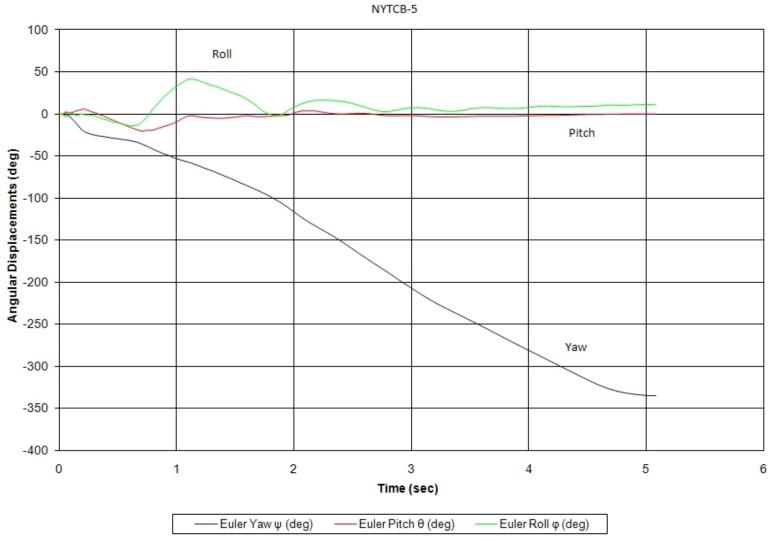


Figure D-11. Lateral Occupant Displacement (DTS), Test No. NYTCB-5



Euler Angular Displacements - DTS

Figure D-12. Vehicle Angular Displacements (DTS), Test No. NYTCB-5

Longitudinal CFC 180 10 msec Extracted Average Acceleration - EDR-3

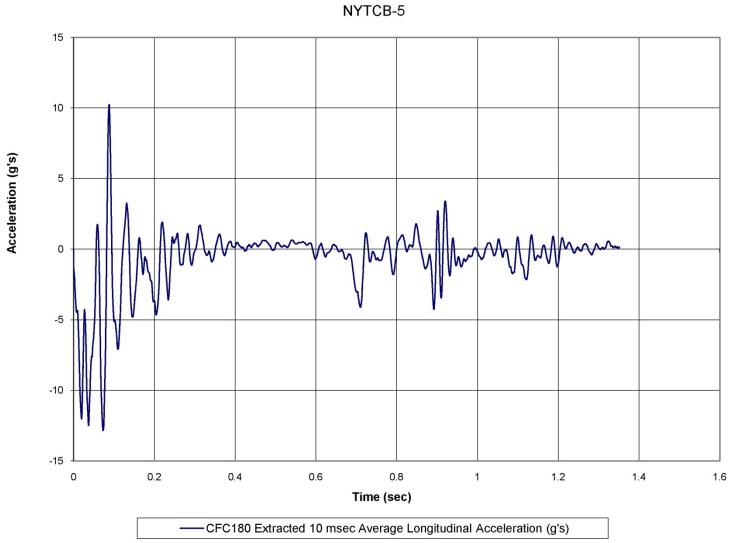


Figure D-13. 10-ms Average Longitudinal Deceleration (EDR-3), Test No. NYTCB-5

Longitudinal Change in Velocity - EDR-3

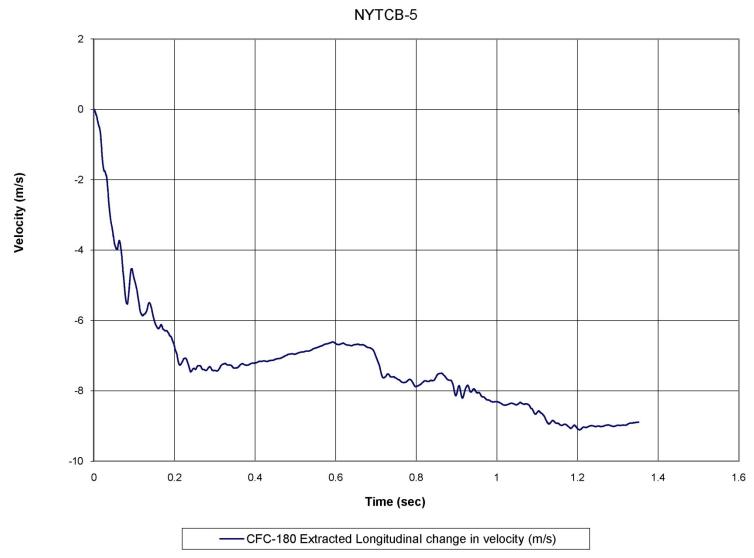


Figure D-14. Longitudinal Occupant Impact Velocity (EDR-3), Test No. NYTCB-5

Longitudinal Change in Displacement - EDR-3



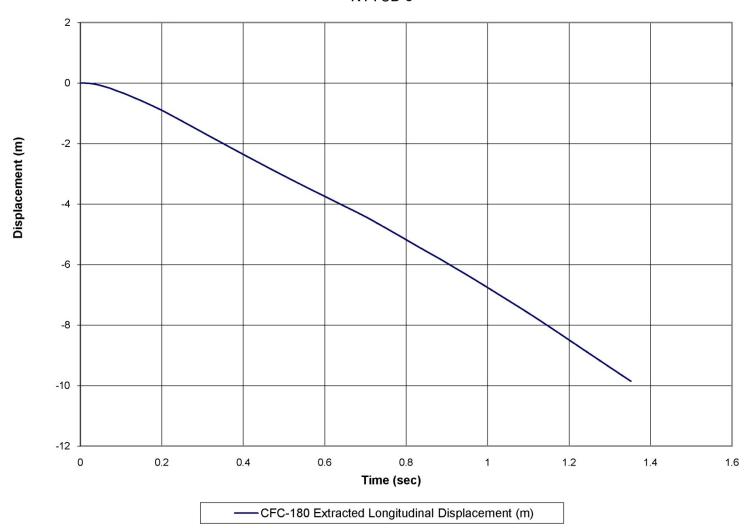


Figure D-15. Longitudinal Occupant Displacement (EDR-3), Test No. NYTCB-5

Lateral CFC 180 10 msec Extracted Acceleration - EDR-3

NYTCB-5

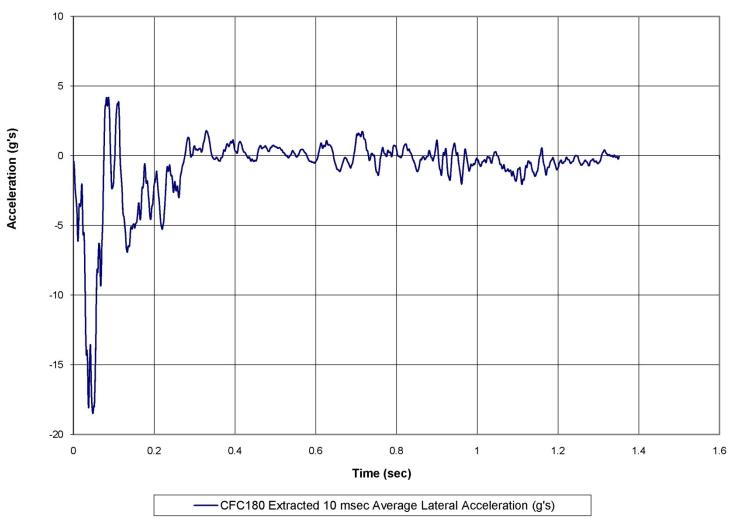


Figure D-16. 10-ms Average Lateral Deceleration (EDR-3), Test No. NYTCB-5

Lateral Change in Velocity - EDR-3



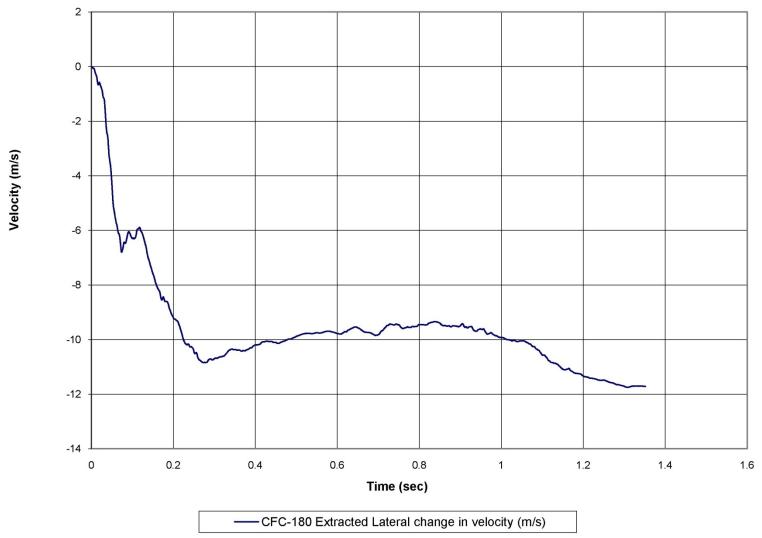


Figure D-17. Lateral Occupant Impact Velocity (EDR-3), Test No. NYTCB-5

Lateral Change in Displacement - EDR-3



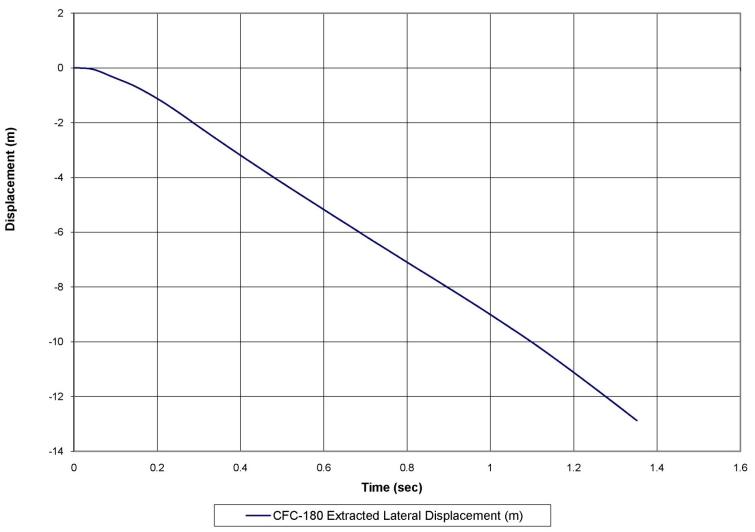


Figure D-18. Lateral Occupant Displacement (EDR-3), Test No. NYTCB-5

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